

## ROMA FLOOD LEVEE PROJECT FINAL DESIGN REPORT

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## **1** INTRODUCTION

#### 1.1 Background / Purpose

The Roma Township is located in the Bungil Creek floodplain and has an extensive history of flooding, with a number of major flood events in the past 100 years resulting in minor damage to private property, public infrastructure and utilities. Three significant large flood events have occurred between March 2010 and February 2012, with the February 2012 flood the largest event ever recorded in Roma and resulting in significant damage to over 500 premises.

As a result of the recent major flood events the Roma Flood Study and Mitigation Project was developed to minimise the flood risk to the Roma community and alleviate property inundation from the Bungil Creek catchment regional flooding.

#### **1.2 Previous Studies**

A number of previous studies have been undertaken by Maranoa Regional Council to identify the current flooding mechanisms and develop a number of mitigation options to alleviate the flooding in the Roma Community. The proposed levee alignment and levels are based on a number of these previous reports and public consultation the levee alignment seeks to optimise a reduction in flood risk, minimize environmental, social and cultural impacts and be economically efficient. Previous studies and referenced documents forming part of this design include:

- Roma Flood Study and Mitigation Project, ENGENY May, 2012;
- Roma Flood Mitigation Project Hydrology and Hydraulic Assessment Report, GHD April, 2013 and;
- Roma Flood Mitigation Project Principle Project Requirements, GHD April, 2013

The proposed Roma Flood Mitigation Levee selected to form part of this design is based on the above previous studies has an alignment that extends from the airport north of Roma, continuing south to finish north-east of Lovell Street as per Figure 1. The levee levels are based on the 1000 year ARI storm event and the historical 2012 flood event as stipulated in the Project Principal Requirements (PPR).



Figure 1 – Locality plan of the Levee



#### **1.3** Objectives and Design Criteria

The levee is to be designed and constructed within the alignment envelope and levels shown in the Principle Project Requirement (PPR) reference drawings and the following design criteria:

- The levee is to consider the historical 2012 flood event and the 1 in 1000 year flood event;
- Have a design life of 100 years;
- Will minimise the ongoing and future operational and maintenance costs;
- Will be designed to satisfy the requirements/standards of the NRM 2002 Dam Safety Management guidelines and ANCOLD guidelines;
- Will be modelled with larger than design floods to determine the point at which overtopping occurs;
- Designed to mitigate against possible failure mechanisms and appropriate design and provision of spillways;
- Ensure access is provided to existing roads, highways and private property owners in accordance with the requirements of any and all Legislative Requirements and the requirements of Council and the property owners;
- Designed so as not to detract from the visual amenity of the local area; and
- Will incorporate provision for local drainage and backflow prevention.

#### 1.4 Queensland Dam Management Guidelines

There is the potential for the levee to be considered referable under the Water Supply (Safety and Reliability) Act (2008) and as a result the levee has been designed with consideration of the requirements this Act.

SMEC have used their extensive dam design and safety monitoring experience to ensure that the proposed levee has been designed to ensure that all potential critical uncertainties are recognised, investigated and resolved to an acceptable risk level to meet all of the applicable design requirements under the Queensland Dam Safety management guidelines relevant to a levee which is planned to be modified and extended in the future. This design process will also ensure that Maranoa Regional Council has a levee which is safe, operable, and maintainable and will address any potential future referable conditions, within reason, if imposed, with minimal impacts to Council.

#### **1.5 Qualifications**

The Design team has defined the vertical height of the levee based on modelling of the 2012 flood event and the 1 in 1000 year ARI event with an alignment stipulated by Council. The Design team has endeavored to ensure that the design has been completed with the utmost care to provide the required amount of protection to the town however the levee in stage 1 offers very little protection to the Roma Community until stage 2 of the levee is completed. The second stage of the levee will require stage 1 outcomes to be reviewed as the characteristics of the levee and flood flows will change.

#### **1.6** Scope of this Package

This report has been compiled to outline the design development during the Final Design (100%) stage for the Levee.

#### **1.6.1** Design Documentation

This package consists of the following design documentation:

This report



- Appendix A Relevant Design Drawings
- Appendix B Construction Specifications
- Appendix C Geotechnical Factual Report
- Appendix D Geotechnical Interpretive Report
- Appendix E Environmental Requirements Checklist
- Appendix F Construction Comments
- Appendix G Third Party Client Comments
- Appendix H Safety In Design
- Appendix I Carnarvon Highway Report
- Appendix J Mechanistic Analysis
- Appendix K Scour Design Note
- Appendix L Flood Report
- Appendix M Operations and Maintenance Manual
- Appendix N Certification
- Appendix O Consequence Assessment
- Appendix P Catchment Plan



## **2 DETAILED DESIGN**

#### 2.1 General Civil

#### 2.1.1 Earthworks

The civil earthworks have been designed with consideration for the site conditions and geotechnical requirements. These requirements have been detailed in Appendix D – Geotechnical Interpretive Report.

#### 2.1.2 Services

A number of clashes with services have been identified and require treatment. A typical treatment detail is shown on the design drawings included in Appendix A. Underground conduits provide areas of weakness in the levee foundation which are susceptible to seepage of water along the conduit and the backfill material of the trench with the associated increased risk of piping causing failure of the levee. To ameliorate this risk removal and replacement of the trench material with flowable fill and a concrete cut-off wall extending into the existing ground each side of the trench has been detailed for each underground conduit underneath the levee.

This treatment will be required on all formal and informal service conduits crossing the levee.

For overhead power lines less than 33kv clearances of 3m will need to be maintained during construction and maintenance of the levee. Overhead power lines with clearances less than 3m have been identified and are to be raised by the service owner.

#### 2.1.3 Fencing

The levee crosses numerous properties and boundary and internal fencing will need to be removed for construction. The design details the replacement of the impacted fencing on a like for like basis. Gates will be required at the top of the levee to ensure that access along the top of the levee is maintained.

#### 2.1.4 Intra Property Access

Where landowners have requested them, the design has provided for access ramps to safely access across the levee and minimise the impact on the landowners. Details of these crossings have been developed in consultation with the relevant landowners.

#### 2.1.5 Driveways

The levee alignment requires changes to numerous driveways. The driveways being changed are:

Lot 5 on RP880094, Lot 1 on RP87687, Lot 1 on SP148633, Lot 6 on RP880094, Lot 4 on RP802116, Lot 2 on RP204592, Lot 73 on WV1650 and Lot 222 on R863.

#### 2.2 Carnarvon Highway

#### 2.2.1 Design

The design for the Levee crosses over the Carnarvon highway. This design has been developed and presented to TMR Roma. The detailed design report and discussion relating to this element of the design is included in Appendix I.



#### 2.3 Geotechnical Design

#### 2.3.1 Investigation

Geotechnical investigations have been carried out and the results of these investigations are included in Appendix C in the Geotechnical Factual Report.

#### 2.3.2 Interpretation

The geotechnical design in relation to the levee is documented in Appendix D.

#### 2.4 Levee Alignment

#### 2.4.1 Vertical Alignment

The vertical alignment has been designed to consider the 2012 ARI event and the 1 in 1000 year event.

- Average levee height 2.8m
- Max levee height 4.0 m

#### 2.4.2 Horizontal Alignment

The horizontal alignment for the levee is approximately 5,151m in length and is located to the northeast of the town and an average of 300 to 400 metres to the west and south of Bungil Creek. The levee starts (Chainage 0) to the north of the Airport building, heads east across the Carnarvon highway and generally follows the course of Bungil Creek terminating (Chainage 5151) at the bank of the creek approximately 500m south of Miscamble St.

For construction and community purposes the levee was decided into four sections:

- Section A Ch. 3750 to 5151
- Section B Ch. 2650 to 3750
- Section C Ch. 1450 to 2650
- Section D Ch. 0 to 1450

#### 2.4.3 Cross Section

The levee cross section consists of 1 on 4 batters, and a 3m crest width.

#### 2.5 Road Crossings

#### 2.5.1 McPhie Street

The levee alignment crosses McPhie Street approximately 500m to the east of the intersection with Edwardes Street. McPhie Street terminates approximately 200m east of the levee close to Bungil Creek. As there are no residential properties on the Creek side of the Levee at McPhie Street no road way has been provided at the location. An access track with 1 in 8 sloped ramp and a 3m formation width has been developed here. The road will now end with fence and gate provided at this location to prevent access to the creek side in a flood event. Signage has been provided on the town side of the road to indicate the road end.



#### 2.5.2 Miscamble Street

The levee alignment crosses Miscamble Street approximately 100m to the east of the intersection with Edwardes Street. Miscamble Street connects with End Short St to the east of the Bungil Creek and is used as an access to the Rubbish Tip.

Given the current usage and functionality of Miscamble St, the existing design existing design speed, assumed to be 70km/hr, needs to be maintained. With this design speed the new road embankment extends over 100m either side of the levee. The existing road blockage signs will be relocated to the dry side of the levee to allow the road to be closed during a flood event.

#### 2.5.3 Bassett Lane

The levee alignment also crosses Bassett lane but no access is required over the levee at this point.

#### 2.5.4 Carnarvon Highway

Refer to Appendix I for the Carnarvon Highway Design Report.

#### 2.6 Pavement Design Miscamble Street

The levee is to be constructed across Miscamble Street approximately 100m east of Edward Street. Miscamble Street is proposed to also be constructed over the levee to provide access to the town of Roma. Consideration for the new pavement over the levee bank has been based on ensuring that the pavement is designed in such a way as to minimise the pavements sensitivity to moisture and ensure that the road is able to function as a link to the township of Roma during periods of inundation and saturation.

#### 2.6.1 Geotechnical Investigation

As part of the geotechnical investigation component of this project, investigation was undertaken on along Miscamble Street to provide information on the existing pavement profile and geotechnical properties of the existing pavement and subgrade materials. Site investigations in the form of test pits were undertaken by SMEC and Ostwald Brothers, with samples taken for laboratory analysis. An overview of the test pit locations is shown on the drawing titled "Geotechnical Investigation and Borrow Pit Locations Sheet 4" as attached in Appendix A. Test pits along Miscamble Street were undertaken at the following locations:

Test Pit	Location	Chainage (Approx.)	Control Line	Easting (m)	Northing (m)	RL (m) AHD
TPX06	Crossing	236	MCA1	679179.382	7060810.796	297.863
TPX08	East End	331	MCA1	679275.378	7060809.143	298.195
TPX09	West End	96	MCA1	679040.312	7060807.014	297.833

There has been 3 test pits undertaken on Miscamble Street. They include TPX06, TPX08 and TPX09. The existing pavement profile from the test pits is summarised below:



lavor	Test Pits						
Layer	ТРХО6	ТРХ08	ТРХ09				
Surfacing	30mm – Asphalt	30mm – Asphalt	15mm – Bituminous Seal				
Base	150mm – Sandy Gravel	180mm – Silty Sandy GRAVEL	160mm – Sandy GRAVEL				
Natural Fill / Subgrade	Gravelly SILT	Sandy CLAY	Silty CLAY				

#### 2.6.2 Design Traffic Loading

The design traffic loading was calculated according to the Capricorn Municipal Development Guidelines - Pavement Design D2. These guidelines stipulate that for a rural street with less than 250 vehicles per day the Design Traffic Loading is  $5 \times 10^5$  Equivalent Standard Axles (ESA's).

#### 2.6.3 Subgrade Assessment

The new pavement for Miscamble Street is to be constructed over two varying subsurface materials. The new Miscamble Street pavement is to be constructed over both imported fill and the existing subsurface material. Material samples were taken from the test pits to determine the likely condition of the existing subgrade material. Laboratory test reports for the existing subgrade material are included in Appendix C.

The geotechnical testing undertaken on Test Pits TPX06, TPX08, TPX09 for Miscamble Street have indicated that the area is characterised by black reactive clay with possible organic content. The results of materials testing indicate that the existing subsurface conditions are generally reactive and have a low bearing strength when wet. Based on the geotechnical testing undertaken, in a soaked condition a design subgrade CBR of 3% is considered representative, with a swell of 3%.

The pavement for Miscamble Street is also to be constructed on imported fill with a Type B Main Roads specification. The imported fill material is considered to be of an equivalent standard to the existing subgrade conditions. Internal Geotechnical advice indicates that settlement of the fill embankment is not expected. It is considered appropriate, therefore, that the imported material is of an approximately the same structural capacity as the existing subsurface materials along Miscamble Street. Therefore a design subgrade CBR of 3% and swell of 3% is considered appropriate for the subgrade over the levee and the existing subgrade.

#### 2.6.4 Pavement Design

Our pavement design has been undertaken in accordance with TMR's Pavement Design Supplement 2013, and has also considered proven North Queensland pavement design practice using a 3 season design approach. The pavement designs produced for this project has considered the existing pavement profile indicated from site investigations, past performance, coupled with design intent to ensure the pavement gravels sensitivity to moisture is minimised.

The recommended pavement composition considers the swell potential of the clay subgrade and utilises two different cemented gravel layers to mitigate against degree of saturation issues of unbound materials and takes a managed risk approach to reflective cracking of the heavily bound cemented layers. The option to use a layer of CBR 35 select fill material has been provided to achieve cover requirements over the expected reactive subgrade.



Mechanistic pavement analysis has been utilised for this design, with a pre-crack and post-crack life for the heavily bound cement treated base material incorporated into the design (refer to Appendix J for further information) and a 3 season design approach has been adopted to accommodate for expected wide ranging design conditions. A managed risk approach has been adopted when considering management of reflective cracking.

This pavement design is considered significantly different to the existing pavement details along Miscamble Street. Project specific pavement design notes have been provided on Drawing No. 30031182-PD-0011, 30031182-DP-1001 and this report is to be read in conjunction with the design drawings and pavement notes provided.

#### 2.7 Pavement Specification for Miscamble Street

Given that this project involves works for the Carnarvon Highway (a TMR asset) as well as a local Maranoa Council roads a consistent specification has been adopted, that being TMR's, so that the contractor can achieve common work practices. Reference should be made to the Pavement Notes for the appropriate Main Roads Specification and Annexures.

#### 2.8 Roma Airport

Roma Airport is owned and operated by Maranoa Regional Council. The levee commences on airport land and travels 730m until it reaches the Carnarvon Highway. The levee does not conflict with the runway, runway clear zone or runway sight distances. The levee has been designed in accordance with the CASA Manual of Standards Part 139 - Aerodromes.

#### 2.9 Hydrological and Hydraulic Analysis

The hydrological and hydraulic analysis is completed and included in Appendix J.

#### 2.9.1 Catchment Description

The Bungil Creek catchment is a small catchment located on the southern foothills of the Carnarvon Range upstream of Roma covering approximately 1400 km2. Bungil Creek flows in southerly direction. Approximately 70 km further downstream from Roma, Bungil Creek joins with the Balonne River which is part of the greater Murray-Darling system.

Annual average rainfall throughout the Bungil Creek catchment is relatively constant. The mean annual rainfall is reported to be approximately 700 [mm]. There is a degree of variability in regard to monthly rainfall, and records show that the summer months dominate rainfall totals within the catchment.

#### 2.10 Local Drainage

The local drainage has been assessed in the vicinity of the levee using the aerial photography, supplied Digital Elevation Model and survey. The intent of the local drainage design is to mimic the existing scenario and use positive drainage paths / grades rather than a formal drainage channel and pipe network. The levee will require adjustments to ground levels and existing flow paths adjacent to the levee. At various locations there is a need to provide cross drainage structures. This has been determined by assessing the existing flow regime, identifying the potential to affect upstream properties, and the potential to damage existing infrastructure. Based on the stormwater assessment undertaken there are two types of stormwater culverts proposed.

One is a drainage culvert that conveys runoff from one side of the levee to the other. This culvert crossings requires the installation of a penstock or equivalent back flow device to prevent backwater



impacting the operation of the levee, a cut off wall, and a suitable backfill material to ensure the levee structural integrity is not comprised and piping failure will not occur.

The second type of culvert specified is a normal culvert configuration for drains, driveways, ramps and accesses not hydraulically impacted on by the levee.

Consultation with property owners has been undertaken to determine any impacts the drainage strategy may have on current farm dams or current water usage.

A hydrologic and hydraulic assessment was completed for the proposed culvert crossings located along the Roma Flood Levee alignment. A desktop study and physical site inspection were undertaken at project inception. The topography of the catchment areas in the vicinity of the Flood Levee alignment was very flat and available data typically did not accurately define waterway characteristics such as channel bed or banks in the vicinity of the Flood Levee. Catchments located on the western side of the Flood Levee alignment generally flowed in a south-westerly direction away from the Flood Levee. Catchments located on the eastern side of the Flood Levee alignment generally flowed in a southern direction adjacent to the Bungil Creek.

Culvert crossing locations were determined using the abovementioned topography assessment to provide drainage relief along the Flood Levee alignment as required. Landowners potentially impacted by the Flood Levee alignment were also consulted throughout the design process.

Hydrologic calculations were completed for the proposed transverse drainage culverts and results are shown in the following table. Catchments were derived using a combination of detailed ground survey and Lidar aerial survey and were delineated using the 12d design software. Flows were calculated using the Rational Method in accordance with the Queensland Urban Design Manual (QUDM). The Rational Method used did not consider rainfall initial and continuing losses which would significantly reduce predicted design flows for large rural / undeveloped catchments, consideration of farm dams that would further attenuate flows have also not been considered. The hydrologic calculations can therefore be considered conservative. Hydrographs were also not developed for the hydrologic assessment.

Hydraulic calculations were completed for the proposed transverse drainage culverts using Manning's Equation in accordance with QUDM and results are shown in the following table. Manning's Equation was used to calculate the capacities of stormwater culverts flowing full, but not under pressure. During significant flood events where the water head level would be above the culvert inlet, more water would be forced through the culvert, increasing the culvert capacity. The Manning's Equation calculations did not take this into account and can therefore be considered conservative. The culvert capacities reported in the following table would likely be higher accounting of pressurised flow.

A summary of the hydrologic and hydraulic results for the transverse culvert crossings is shown in the following table. A catchment plan is included in Appendix P.



Culuart ID	Culvert	Contributing	Catchment	Contribu (m	Culvert	
Culvert ID	Structure	Catchment(s)	Area (ha)	2 year AEP	10 year AEP	(m <sup>3</sup> /s)
WP01F	600RCP	А	1.4	0.08	0.14	0.04
WP01G	600RCP	В	2.6	0.15	0.28	0.06
WP01H	600RCP	А	1.4	0.08	0.14	0.04
WP02A	600RCP	С	1.2	0.08	0.15	0.13
WP02B	450RCP	D	0.3	0.02	0.04	0.02
WP04A	600RCP	E	2.6	0.14	0.25	0.05
WP05A	600RCP	F	13.9	0.66	1.21	0.04
WP08B	600RCP	G+H	31.5	1.21	2.20	0.03
WP08C	600RCP	Н	4.5	0.24	0.43	0.03
WP08D	600x225RCBC	I	0.1	0.01	0.02	0.14

The above table demonstrates that culverts WP01F, WP01G, WP01H, WP02A, WP02B, WP04A and WP08D will adequately convey flows in the vicinity of the 2 to 10 year Annual Exceedance Probability (AEP) flood events.

Culvert WP05A is located on the eastern side of the Flood Levee alignment and is expected to provide relief drainage for minor catchment flows. During regional flooding the culvert penstock is to be closed and zero flows will be conveyed by the culvert.

Culvert WP08B is located underneath Miscamble Street on the eastern side of the Flood Level at the southern end of the alignment. The proposed culvert is a 600mm diameter RCP and replaces an existing culvert approximately 375mm in diameter. The culvert provides relief drainage for flows converging at the intersection of the Flood Levee and Miscamble Street. Flows in major flood events will likely flow adjacent to Miscamble Street to the east and towards the Bungil Creek.

Culvert WP08C is located at the southern end of the Flood Levee alignment adjacent to Miscamble Street. The proposed culvert provides relief drainage for the local catchment drain area located to the west of the Flood Levee alignment. Any major flows in this location are expected to flow to the south and overtop Miscamble Street.

### 2.11 Environmental Controls and Permits

The Environment team has reviewed the proposed design for the levee and relevant desktop information to identify the potential major environmental impacts associated with the works and any likely permitting requirements.

The main environmental consideration for the proposed levee is associated with the disturbance footprint resulting from excavation and placement of fill. This may trigger clearing permits, permits



to operate an Environmentally Relevant Activity, and building works permits (to allow demolition of structures).

Considering the rural and previously disturbed nature of the site, ecological impacts are expected to be minimal. If works encroach into Bungil Creek, some permitting requirements may apply including permits for clearing regional ecosystem vegetation or construction of waterway barriers.

Permits under Maranoa Regional Council are likely to be required for interference with and alteration of local roads and road reserve, as the proposed levee impacts three local roads.

A summary of the potential environmental approvals for the proposed works, including the typical time frames and responsible party for each, is provided in Appendix E. The need for many of these permits and approvals is currently the responsibility of the contractor.

#### 2.12 Possible Failure Modes

This section sets out the major possible failure modes of an earth embankment levee and details what steps the design team has gone through to prevent those failures. It is not a full list of all cases considered but a high level look into steps taken by the design team to optimise the design to suit local conditions.

#### 2.12.1 Failures related to Culverts and Service Conduits

Penetrations such as culverts and service conduits are potential weak points through the levee. The failure of a levee around a culvert or service conduit through an earth embankment levee can be initiated by development of a piping type failure mechanism along the penetration. Piping failures can occur over time or develop quickly during a flood event.

Current design practice in earth embankment levees and dams includes a sand filter running along the levee to mitigate against the development of a piping failure. Due to concerns regarding material availability and in order to find a more economically viable solution for the project the design team investigated alternative options.

The design that was produced specifies a centrally placed cut-off wall and non-dispersive materials for all penetrations through the levee. The cut off wall includes a ring of hydrotite or similar product to prevent water passing through any shrinkage that may occur between the penetration and the cut off wall.

Culverts passing through the levee have been designed with flowable fill up to the mid-point of the culvert to minimise the possibility of voids. A less dispersive fill material Type C is then placed above this flowable fill. Type C material is to be placed at a 1 on 1 slope into the existing levee in in order to ensure that it integrates with the adjacent Type B material. For more detail on the design of culverts and drainage please see section 2.10.

Services, such as Telstra, passing under the levee are to be exposed, all previous trenching and unsuitable material to be removed and replaced with a non-dispersive flowable fill around the service. For more details on the services please see Section 2.1.2.

#### 2.12.2 Seepage-related Failures

An earth embankment levee is known to be potentially susceptible to failure due to seepage through the levee and underseepage in pervious foundations beneath the levee. There are a number of factors affecting failures due to seepage, including hydraulic gradient, soil plasticity, and particle size, soil dispersiveness, soil permeability and others. In order to evaluate the proposed levee in relation to safety against seepage and seepage control measures to mitigate the effects of seepage, it is important to understand the various modes of potential failure that can occur due to seepage through the proposed levee or its foundation.



The design team has therefore identified possible failure modes related to seepage and presented as follows:

- Tunnelling / Jugging
- Concentrated leak erosion through embankment and/or foundation
- Heave or boiling
- Piping due to high exit gradient

Section 2.12.1 details the design intent with regard to possible tunnelling, piping or boiling failures. The levee profile includes a 3m wide crest, with 1:4 batters minimising any risk of concentrated leak erosion through the embankment and foundation. The O&M manual in Appendix M discusses identification of these types of failures and how to mitigate against them in a flood event.

Please see Appendix D, the interpretive geotechnical report for more detail on how the design has considered these failure mechanisms.

#### 2.12.3 Stability-related Failures

Common failures of an earth embankment levee and relevant structures such as penstocks are also related to geotechnical instability under the various loading conditions to which the levee may be subjected in its life time and which should be accounted for the design e.g. end of construction, long-term, flood event, sudden drawdown and earthquakes. In particular for levees of significant height or where there are concerns regarding qualified embankment material availability or foundation conditions requires extensive stability analysis.

Regarding the stability of the levee and relevant structures, the design team has identified the following possible failure modes:

- Instability of levee under the various loading conditions
- Cracking due to differential settlement, desiccation or other causes
- Progressive failure due to aging of levee materials
- Instability of penstock in terms of sliding, overturning and global stability

Instability of the levee has been considered in Appendix D, the Geotechnical Interpretive Report. The design team featured numerous hold points and inspections to be undertaken by a Geotechnical Representative in order to ensure the suitability and minimise any possible differential settlement of materials. Where possible the use of locally sourced materials has been undertaken to prevent failure due to aging of levee materials. Penstocks have been analysed by both a geotechnical and structural engineer to ensure there is negligible risk of movement at the culvert.

Please see Appendix D, the interpretive geotechnical report for more detail on how the design has considered these failure mechanisms.

#### 2.12.4 External Erosion Related Failures

External erosion related failures include levee failures caused by overtopping, hydraulic actions such as currents and turbulence and human and animal damage. The design has taken a number of steps to ensure that the levee has the structural integrity to resist these failure mechanisms.

The levee has considered a number of flood events and modelled the flood levels in order to gain as accurately as possible the required crest level to prevent overtopping. The levee design level is to be constructed with Type B approved levee fill to prevent overtopping in certain flood events. For more information on this modelling please refer to section 2.9 and Appendix J.

The levee design has examined a number of options to mitigate against high velocity flows predicted by flood modelling in Appendix K – Scour Design Note. The detailed design drawings have prescribed treatments of placed rock and turf reinforced matting with suitable toe protection details to counter these forces.



The levee should not be grazed by animals where possible and should be repaired as soon as possible as described in Appendix M – Operations and Maintenance Manual. The levee is not recommended to be accessed by vehicles or animals at any stage.

In order to prevent the drying and desiccation of the Type B material the design has included the placement of 150mm of topsoil and grass seeding. The topsoil and grass seeding has been specified to maintain moisture content and reduce the risk of desiccation. The Topsoil and grass seeding is to be maintained as per the operations and maintenance manual in Appendix M.

#### 2.13 Design Life

The design of the levee has taken into account fit for purpose measures in relation to a levee and all elements have a 100 year design life where feasible. Erosion protection measures and replaceable elements do not meet the 100 year design life.

The design considers a number of flood events as discussed in the reporting and the design has been development to ensure that future operation and maintenance costs have been minimised.

#### 2.14 Design Changes

#### 2.14.1 Changes between Detailed Design and Final Design

The Final Design differs from the detailed design in the following key areas:

Location	Description	Reason
Section D	Alignment changed between Carnarvon Highway and end of Section D	Landowner Preferences
Section B	Alignment at McPhie Street changed.	Landowner Preferences
Section B to D	Culvert WP04A	As a result of independent internal review.

Table 1 – Key Differences between Detailed Design and Final Design

### 2.15 Items for Resolution

The following items are to be resolved in the next stage of this design package:

Table 2 – Items to be resolved in the next submission of this design package

Location	Description	Method of Resolution
Nil		

#### 2.16 Verification, Reviews and Certification

#### 2.16.1 Internal Design Verification

Internal Verification has been undertaken and all comments closed out.

#### 2.16.2 Other Party Review (Construction)

Third party comments from the construction team have been received and addressed comments are attached in Appendix F.



#### 2.16.3 Other Party Review (Clients Representative)

Third party comments from the client's representative have been received and are included in Appendix G.

#### 2.16.4 Certification

Certification of the design has been included in Appendix N. A consequence assessment has also been undertaken and included in Appendix O.

#### 2.17 Design Drawings

Refer to Appendix A for the list of design drawings applicable to this design lot.

#### 2.18 Construction Methodology

The construction methodology of the levee has been to build the levee in stages with large scale earth moving machinery. To ensure that the levee is constructed in accordance with the design intent SMEC has maintained an onsite presence during the construction of the works where required. The key in relation to the construction methodology is building the levee up in linear layers and ensuring the levee is filled and cut back. SMEC and Ostwalds Brothers have undertaken multiple workshops in relation to suitable construction methods and staging.

#### 2.19 Construction Specifications

Refer to Appendix B for the construction specifications.



## **3** SAFETY IN DESIGN AND RISK ASSESSMENT

#### 3.1 Safety in Design

SMEC and Ostwald Brothers acknowledge the importance of Safety in Design and the following measures are used during the design phase:

Identify the hazards presented by potential design solutions and consider the risks these hazards will generate for construction works and others who may be affected by the construction work

Include health and safety considerations amongst the design options to avoid the hazards, reduce their impact or introduce control measures to protect those at risk where it is reasonably practicable

Forewarn the construction team of the residual hazards that have been identified within the design and will need to be managed during the construction work

As well as the above, the following hierarchy of risk control has adopted:

- **1.** Eliminate the hazards. The job is redesigned so as to remove the hazard. However, the alternative method should not lead to a less acceptable product or less effective process
- **2.** Substituting the system of work or plant with something safer. Replace the material or process with a less hazardous one.
- **3.** Engineering controls Install or by using additional machinery such as local exhaust ventilation to control a risk. Separating the hazard from operators by methods such as enclosing or guarding dangerous items of machinery. For example, use guards on compression testing machines.
- **4.** Administrative controls Reduce the time the worker is exposed to the hazard. Prohibit eating, drinking and smoking in laboratory areas. Provide training. Perform risk assessments. Increase safety awareness signage.
- 5. Personal Protective Equipment (PPE) Only after all previous measures have been tried and found to be ineffective in controlling the risks should Personal Protective Equipment be considered. If chosen, PPE should be selected and fitted individually. Workers must be trained in the function and limitations of each item of PPE. For example, an operator should know how long the compressed supply in a self-contained breathing apparatus will last. PPE may be used as a temporary control measure until other alternatives are installed. In most cases a combination of engineering controls, administrative controls and PPE are chosen to effectively control the risks

#### 3.2 Construction Hazard Assessment and Implication Review (CHAIR) Purpose

Construction hazard assessment and implication review (CHAIR) is a systematic method of analysing the importance of hazardous situation associated with a system or activity. It uses a simple technique to look for unexpected deviations from the way things are designed to happen, and decides whether these deviations could damage people, property or the environment. It does not review the design of the system or activity, but seeks to challenge the design intent looking for unexpected was the design can be defeated or modified. The aim is to facilitate safer construction or future operation

One of the main purpose of CHAIR is to bring together designers, constructors, asset owners and operates, and key stakeholders to eliminate (or minimise) the construction, operation, maintenance and demolition risk of project/asset during the design phase

The CHAIR provides a rigorous framework to facilitate discussion through the use of guidewords. The risks are formally identified, along with the likely consequences for action by the appropriate stakeholders.



The CHAIR is conducted in three parts, **CHAIR 1** (global project risks) during the concept/ tender design providing the best opportunity to make fundamental change even though much of the detail is yet too determined.

**CHAIR 2** (construction and demolition risks) and **CHAIR 3** (operation and maintenance risks) assess the specific risks of those items during the detail design (85% Design) and reduce the CHAIR 2 risks while identifying the CHAIR 3 risks.

The CHAIR process and it's relation to the project and design phases has been shown below in Figure 2.



Figure 2 – The Chair Process

A safety in design workshop was undertaken on 15<sup>th</sup> October 2013, the outcomes from this assessment are included in Appendix H.



## **4 OPERATION, SAFETY AND MAINTENANCE**

The detailed design has considered the ongoing operation, safety and maintenance issues. For further details please refer to Appendix M – Operations and Maintenance Manual.

#### **4.1 Operation Considerations**

The design of the proposed levee embankment including addressing potential seepage and dam safety issues will be designed to consider current standards for a "Referable Dam". It is noted that over time standards change and there is no guarantee that the proposed levee will meet future changes to design standards and government requirements.

Council is to also note that a Regulated Dam is required to have in place a raft of documentation and maintenance practices. These include regular dam safety inspections (periodic, annual and comprehensive) together with their associated reporting requirements, Operations and Maintenance Manual, Standing Operation Procedures (SOPs), an established maintenance regime and an emergency action plan that includes levee failure scenarios. This documentation needs to be developed in close consultation with Council.

#### 4.2 Safety Considerations

Safety has been considered during the design, construction and operation of the levee and key considerations to date are:

- Suitable signage;
- Inclusion of fencing and gates to restrict public access:
- Providing safe access for the operation of penstocks / gates
- Maintain 1 on 4 batters; and

Proposing desirable road geometry solutions.

#### 4.3 Maintenance Considerations

Maintenance has been considered during the design, and construction and operation of the levee and key considerations to date are:

- Inclusion of access;
- Providing safe access for the operation of penstocks / gates;
- Maintain 1 on 4 batters; and

Temporary Alternative Access tracks.



**APPENDIX A – RELEVANT DESIGN DRAWINGS** 

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OWNTRUE LINE AND FENCING SETOUT SHEET 4         CL-1004         B           CONTROL LINE AND FENCING SETOUT TABLES         CL-1004         B           CONTROL LINE AND FENCING SETOUT TABLES         CL-1004         B           CONTROL LINE AND LONG SECTION CONTROL LINE MC10 SHEET 1         WP-1001         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 2         WP-1002         D           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 3         WP-1003         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 4         WP-1004         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 5         WP-1006         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 7         WP-1006         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 7         WP-1006         E           WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 7         WP-1008         F           GENERAL ARANGEMENT - LEVEE CROSSINGS MISCAMBLE STREET - PLAN AND SECTIONS         GA-0001         C           GENERAL ARANGEMENT - LEVEE CROSSINGS PROPERTY ACCESS SHEET 1         PA-0001         D           PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS SHEET 1         PA-0002         C           PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS SHEET 3         PA-0003         C           PROPER	CONTROL LINE AND FENCING SETOUT SHEET 2		CL-1002	D F				
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	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 13		XS-1012	c				

ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 14	XS-1014	С
ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 15	XS-1015	С
ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 16	XS-1016	С
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS OVERALL PLAN	GE-0101	С
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 1	GE-1001	D
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 2	GE-1002	D
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 3	GE-1003	D
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 4	GE-1004	В
GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 5	GE-1005	С
PUBLIC UTILITIES PLAN SHEET 1	PU-1001	С
PUBLIC UTILITIES PLAN SHEET 2	PU-1002	С
PUBLIC UTILITIES PLAN SHEET 3	PU-1003	D
PUBLIC UTILITIES PLAN SHEET 4	PU-1004	В
PUBLIC UTILITIES PLAN MCPHIE STREET	PU-1011	С
PUBLIC UTILITIES PLAN TREATMENT FOR SERVICES UNDER LEVEE	PU-1020	В
TYPICAL CROSS SECTION	TC-1101	С
CONTROL LINE SETOUT	CL-1101	С
WORKING PLAN AND LONGITUDINAL SECTION SHEET 1	WP-1101	D
WORKING PLAN AND LONGITUDINAL SECTION SHEET 2	WP-1102	D
ANNOTATED CROSS SECTIONS SHEET 1	XS-1101	D
ANNOTATED CROSS SECTIONS SHEET 2	XS-1102	D
ANNOTATED CROSS SECTIONS SHEET 3	XS-1103	D
DRAINAGE CROSS SECTION AND SCHEDULE	DD-1101	С
PROPERTY ACCESS PLAN SHEET 1	PA-1101	С
PROPERTY ACCESS PLAN SHEET 2	PA-1102	С
PUBLIC UTILITIES PLAN	PU-1101	С

# MARANOA REGIONAL COUNCIL ROMA FLOOD MITIGATION PROJECT



LOCALITY PLAN

EXTERNAL REFERENCE FILES	REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	NAME	SCALES AT A1 SIZE DRAWING	DESIGNER	CLIENT
	Α	20/01/201	4 ISSUED FOR CONSTRUCTION	008	AZ	DRAFTER	B. TAYLOR		SMEC	
	B	30/01/201	4 REISSUED FOR SECTIONS B&D	019	AZ	DRAFTING CHECK	J. RAISON			
	Ľ	31/03/201	GIRKETT LEVEE REALIONMENT	030	AZ	DESIGNER	J. RAISON	NTS	SMEC AUSTRALIA PTY LTD	1 100 2/3
						DESIGN CHECK	J. CARROLL		© ABN 47 065 475 149 LEVEL 1 154 MELBOURNE STREET	Inara
						PROJECT MANAGER	A. ZANETTI		SOUTH BRISBANE QLD. 4101 PH 07 3029 6600 FAX 07 3029 6650	MEGIONAL
						PROJECT DIRECTOR	R. STRINGFELLOW		SMEC PROJECT No 30031182	
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EVISION

	DRAWING INDEX		P/	ACK	AC	ε
DRAWING NUMBER	DRAWING TITLE	REVISION	AC	BD	CH	ST
30031182-XS-1004	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 4	(0)		X		
30031182-XS-1005	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 5	202	X	X		
30031182-XS-1006	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 6	$\langle c \rangle$	X			
30031182-XS-1007	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 7	( )	X			
30031182-X5-1008	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 8	( ( (	X			
30031182-XS-1009	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 9	101		X (		
30031182-XS-1010	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 10	>0)	1 I	x		
30031182-XS-1011	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 11	50)		x		
30031182-XS-1012	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 12	(0)	X	x	11	
30031182-XS-1013	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 13	(()	X			
30031182-XS-1014	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 14	A C C	X			
30031182-XS-1015	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 15	()	X		1	
30031182-XS-1016	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 16	$\left  \right\rangle$	x		_	
30031182-GE-0101	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS OVERALL PLAN	c	x	x	x	
30031182-GE-1001	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 1	D	X	x		
30031182-GE-1002	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 2	D	X	X	0	
30031182-66-1003	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 3	c	X	X		
30031182-68-1004	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 4	B	X	-		
30031182-GE-1005	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 5	c	x	x		
30631102-57-1001	SUGET PILE LEVEL WALL PLAN AND ELEVATION SECTION D	*	-		-	*
30031102-51-1002	SHIFT PH F I FYEF WALL PLAN AND FLEVATION SECTION D				-	*
50051102-51-1002						-
30031182-PU-1001	PUBLIC UTILITIES PLAN SHEET 1	C		×		
30031182-PU-1002	PUBLIC UTILITIES PLAN SHEET 2	C	X	X		
30031182-PU-1003	PUBLIC UTILITIES PLAN SHEET 3	D	X	X		
30031182-PU-1004	PUBLIC UTILITIES PLAN SHEET 4	В	X	_		
30031182-PU-1011	PUBLIC UTILITIES PLAN MCPHIE STREET	C		x		-
30031102-PU-1012	PUBLIC UTILITIES PLAN MISCAMBLE STREET	- 0	*	-	-	-
30031182-PU-1020	PUBLIC UTILITIES PLAN TREATMENT FOR SERVICES UNDER LEVEE	В	X	x	x	X
30031182-TC-1101	CARNARVON HIGHWAY - TYPICAL CROSS SECTION	0	+		x	$\vdash$
					х	
30031182-CL-1101	CARNARVON HIGHWAY - CONTROL LINE SETOUT	¢	⊢		X	-
30031182-WP-1101	CARNARVON HIGHWAY - WORKING PLAN AND LONGITUDINAL SECTION SHEET 1	D			x	-
30031182-WP-1102	CARNARVON HIGHWAY - WORKING PLAN AND LONGITUDINAL SECTION SHEET 2	D			x	
30031182- 25-1101	CARNARYON HIGHWAY - ANNOTATED CROSS SECTIONS SHEET 1	D	⊢	-	X	⊢
30031102-X3-1101	CADNADVON HIGHWAY - ANNOTATED (DOSS SECTIONS SHEET 2	0	+	-	x	
30031102-A3-1102		0	+	-	Ŷ	$\vdash$
20021102-22-1103	CARMETON INDINAT - ANNUTATED CRUSS SECTIONS SILETS			t	x	
30031182-DD-1101	CARNARVON HIGHWAY - DRAINAGE CROSS SECTION AND SCHEDULE	D			X	
30031182-PA-1101	CARNARVON HIGHWAY - PROPERTY ACCESS PLAN SHEET 1	6	+	$\vdash$	X	$\vdash$
30031182_04_1102	CARNARYON HIGHWAY - PROPERTY ACCESS DI AN SHEFT 2	0	+	1	x	
50031102-PA-1102	CARGARY DISTRAT - FROPENT LACLESS FERSINELS 2		+	1	X	
20021192 011 1101	CADNADVON HIGHWAY - PURIT UTILITIES PLAN	0	1	1	x	

	DRAWING INDEX		PA	1CK	AC	j
DRAWING NUMBER	DRAWING TITLE	REVISION	AC	BD	CH	ł
30031182-DI-0001	LOCALITY PLAN		X	x	X	1
30031182-DI-0002	DRAWING INDEX	6)	x	x	x	-
30031182-DI-0011	GENERAL NOTES AND LEGEND	- <del>{  </del> }	×	×	x	
10031103 644 0001	SLUDE VE MUDIC VND KEA DI VNC		x	×	×	
30031182-3W-0001	SCOPE OF WORKS AND REFFERING		-	~	-	i
30031182-TC-0101	TYPICAL CROSS SECTION AND DETAILS SHEET 1	(E)	х	x		
30031182-TC-0102	TYPICAL CROSS SECTION AND DETAILS SHEET 2	-	X	X		
30031182-TC-0103	TYPICAL CROSS SECTION AND DETAILS SHEET 3	E	X	X	F	
30031182-CL-1001	CONTROL LINE AND FENCING SETOUT SHEET 1	C		x		
30031182-CL-1002	CONTROL LINE AND FENCING SETOUT SHEET 2	D	X	X		
30031182-CL-1003	CONTROL LINE AND FENCING SETOUT SHEET 3	E	X	X		1
30031182-CL-1004	CONTROL LINE AND FENCING SETOUT SHEET 4	B	X			1
30031182-CL-1005	CONTROL LINE AND FENCING SETOUT TABLES	$\Delta(F)$	x	х		
30031182-WP-1001	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 1	- 6	$\vdash$	x	-	
0031182-WP-1002	WORKING PLAN AND LONG SECTION CONTROL LINE METO SHEET 2	(02		X		
10031182-WP-1003	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 3	(1)	x	-		
0031182_WP_1004	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 4	()E)	x	x		
0031182-WP-1005	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 5	1 D S		x		
0031182-WP-1006	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 6	1 ( E (	X	X	-	
80031182-WP-1007	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 7	( E 2	x		-	
80031182-WP-1008	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 8	(r)	x			
20021192 64 0001	GENERAL ADANGEMENT - LEVER CONSSINGS MERIES STREET - PLAN AND SECTIONS		-	x	-	
30031182-64-0007	GENERAL ARANGEMENT - LEVEL CROSSINGS MISCAMBLE STREET - PLAN AND SECTIONS	B	x	1		
30031182-GA-0003	GENERAL ARANGEMENT - LEVEE CROSSINGS PRIVATE ACCESSES	D		x		
	ARADIAYU 177767 STUFF PROPENCE ARADIATU 177555 CUEST 1	0	-	×	-	
30031182-PA-0001	PROPERTY ALLESS - LEVEL LRUSSINGS PROPERTY ALLESS SHEET		-	^	-	
30031182-PA-0002	PROPERTY ALLESS - LEVEL LROSSINGS PROPERTY ALLESS SHEET 2		^	V	-	
30031182-PA-0003	PROPERTY ALLESS - LEVEL LRUSSINGS PROPERTY ALLESS SHEET 3		V	^	-	
30031182-PA-0004	PROPERTY ALLESS - LEVEL LRUSSINGS PROPERTY ALLESS SHEET 4		÷	-	-	
30031182-PA-0005 30031182-PA-0101	PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS DETAILS	- î	x	x	-	
STRATE CONTRACTOR						
30031182-PD-0011	PAVEMENT GENERAL NOTES	3	X	X		
30031182-PD-1001	PAVEMENT DETAILS	(	X	X	-	
30031182-DD-0001	DRAINAGE CULVERT SECTIONS SHEET 1	E	x	x		
30031182-DD-0002	DRAINAGE CULVERT SECTIONS SHEET 2	C	x	X		
30031182-DD-0111	PENSTOCK DETAILS SHEET 1	E	X	x		
30031182-DD-0112	PENSTOCK DETAILS SHEET 2	(	X	x	-	
30031182-XS-1001	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 1	(1)		x		
30031182-XS-1002	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 2	(		X		
30031182-XS-1003	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 3	(0)		X		
CKAGE KEY = SECTIONS / = SECTIONS / = SECTIONS /	A AND C DRAWING DELETED/ REMOVED FROM SET	A			1	

> **ISSUED FOR CONSTRUCTION** SMEC AUSTRALIA PTY LTD AIN 47 064 475 140 LEVEL 1 154 MELBOURNE STREET SOUTH BRISBANE QLD 4101 PH67 3029 6650 FXX 07 3029 6650 SMEC PROJECT NO 30031182

/

REV DATE AMENDMENT / REVISION DESCRIPTION WVR Bo. 
 BYU
 Date
 A 2040000177610000 DECORTON

 A
 204201/2014
 ISSUED FOR CONSTRUCTION

 B
 30/20172014
 RESUED FOR SECTORS BAD

 C
 31/03/2014
 BIRKETT LEVEE REALIGNMENT

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 30/201/2014
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SCALES AT AT SIZE DRAWING

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PROJECT MANAGER A. ZANETTI

PROJECT DIRECTOR R. STRINGFELLOW

B. TAYLOR

J. RAISON

J. RAISON

J. CARROLL

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EXTERNAL REFERENCE FILES

## PROJECT DITLE

SCALE

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MARANDA REGIONAL COUNCIL ROMA FLOOD MITIGATION PROJECT DRAWING INDEX

PHASE 30031182-DI-0002 CONSTRUCTION

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#### NOTES

#### GENERAL

- 1. THE FOLLOWING NOTES APPLY GENERALLY TO THE DRAWINGS AND SHALL BE READ IN CONJUNCTION WITH THE NOTES ON THOSE DRAWINGS AND THE SPECIFICATIONS.
- UNLESS OTHERWISE SHOWN ALL DIMENSIONS ARE IN METRES AND ALL LEVELS ARE TO AUSTRALIAN HEIGHT DATUM.
- DO NOT OBTAIN DIMENSIONS BY SCALING FROM DRAWINGS.
- DESIGNERS SITE REPRESENTATIVE IS ABBREVIATED AS (DSR)
- ALL LOCATIONS, ORIENTATION AND LEVELS SHALL BE 5. VERIFIED ON SITE PRIOR TO COMMENCING ANY WORK. ANY DISCREPANCIES ARE TO BE REFERRED TO THE DSR.
- 6. THE CONTRACTOR SHALL CONSTRUCT THE LEVEES, DRAINS AND ASSOCIATED STRUCTURES IN STRICT CONFORMITY WITH THE LOCATIONS AS SHOWN ON THE DRAWINGS EXCEPT WHERE OTHERWISE ADVISED IN WRITING BY THE DSR
- MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE SPECIFICATIONS, RELEVANT AUSTRALIAN AND INTERNATIONAL STANDARDS AND STATUARY REGULATIONS.
- PROVISION FOR TRAFFIC SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE AUSTRALIAN STANDARD FOR MANUAL TRAFFIC CONTROL DEVICES, PART 3 (AS1742.3-1996).
- (01) DENOTES REFERENCE TO DETAIL NUMBER ON 9 1001/ REFERENCE DRAWING:
- 10. PRIOR TO COMMENCEMENT OF CLEARING OR EXCAVATION, THE CONTRACTOR SHALL
  - MAKE COMPLETE ENQUIRIES AND INVESTIGATIONS FOR THE ASSESSMENT AND PROTECTION OF PUBLIC AND PRIVATE SERVICES UTILITIES AND PROPERTY FROM DAMAGE AND CONSTRUCT RELEVANT PROTECTION WORKS AS REQUIRED.
  - UNDERTAKE THE RELEVANT ENVIRONMENTAL, CULTURAL AND HISTORICAL ASSESSMENTS AND ENSURE THAT PLANS, PERMITS AND APPROVALS HAVE BEEN ASSESSED AND APPROVED BY THE RELEVANT AUTHORITIES.
- 11. DESIGNERS SITE GEOTECHNICAL REPRESENTATIVE IS ABBREVIATED AS (DSGR)
- 12. EXISTING SURFACE LEVELS ARE INDICATIVE ONLY.
- 13. ACCESS TO PROPERTIES TO BE MAINTAINED AT ALL TIMES.
- 14. CHAINAGES ARE EXPRESSED IN METRES.
- 15. TOPSOIL TO BE PLACED 150mm THICK.

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#### FOUNDATION AND EARTHWORKS

- THE LEVEE AND STRUCTURE FOUNDATION DESIGN IS BASED ON ASSUMPTION THAT THE FOUNDATION MATERIAL IS STIFF CLAY (OR BETTER). THE FOUNDATION MATERIAL SHALL BE INSPECTED BY THE DSGR TO CONFIRM THAT THE EXPOSED MATERIAL SATISFIES THE DESIGN ASSUMPTION INCLUDING CONFIRMING ABSENCE OF HIGH PERMEABILITY FOUNDATION MATERIAL. ANY MATERIAL ASSESSED TO BE UNSUITABLE SHALL BE REMOVED AND REPLACED WITH APPROVED LEVEE
- REMOVAL OF UNSUITABLE MATERIAL SHALL BE BASED ON AN ASSESSMENT BY THE DSGR
- IF THE SITE CONDITIONS ENCOUNTERED ARE SUBSTANTIALLY DIFFERENT FROM THE CONDITIONS DEPICTED ON THIS PLAN, THE DSR IS TO BE NOTIFIED IMMEDIATELY FOR INSTRUCTION.

- CLEARING AND DISTURBING OF VEGETATION SHALL BE KEPT TO A MINIMUM AND SHALL BE LIMITED TO AREAS CONTAINING THE EXCAVATIONS FOR THE CONSTRUCTION
- WHERE EXCAVATION IS REQUIRED ADJACENT TO EXISTING 5. STRUCTURES, THE CONTRACTOR SHALL DESIGN AND INSTALL TEMPORARY SUPPORT AND SELECT APPROPRIATE FOURPMENT TO ENSURE THAT NO DAMAGE OCCURS TO THE EXISTING STRUCTURE
- ALL EXCAVATIONS SHALL BE MADE SECURE AGAINST 6 MOVEMENT BY MEANS OF STABLE BATTERS, TIMBERING, SHEET PILING OR PROTECTIVE SHIELDS.
- 7. ADEQUATE TEMPORARY PROTECTION AND MAN PROOF SITE FENCING SHALL BE PROVIDED FOR ALL PERSONS, PLANT AND ANIMALS DURING CONSTRUCTION AS PER MRC AGREEMENT.
- LEVEE AND EMBANKMENT FILL MATERIALS AND PLACEMENT 8. SHALL BE TESTED AND PLACED IN ACCORDANCE WITH THE EARTHWORKS SPECIFICATION FOR THIS PROJECT.
- ALL DISTURBED AREAS SHALL BE HYDROMULCHED AT THE 9. COMPLETION OF THE WORK. EQUIVALENT REVEGETATION METHODS PERMITTED SUBJECT TO APPROVAL BY THE DESIGNER
- 10. TOPSOIL SHALL BE FERTILE, FRIABLE SOIL CONTAINING ORGANIC MATTER WHICH IS REASONABLY FREE SUBSOIL REFLISE TREE ROOTS LARGER THAN 20mm IN DIAMETER AND 300mm IN LENGTH, NOXIOUS WEEDS, CLAY LUMPS AND STONES LARGER THAN SOMM
- AAA 11. ALL EXCAVATION, BACKFILLING, COMPACTION AND CONSTRUCTION WORKS SHALL BE CARRIED OUT STRICTLY IN ACCORDANCE WITH THE SPECIFICATION AND AS SHOWN ON THE DRAWINGS.
- 12. LEVEL 1 SUPERVISION AND TESTING OF THE WORKS SHALL BE UNDERTAKEN IN ACCORDANCE WITH AS3798-2007.
- 13. CLEARING TO BE COMPLETED AS PER SPECIFICATIONS.

#### DRAINAGE STRUCTURES

- DESIGN LOADINGS FOR STRUCTURES ARE OPERATIONAL LOADS ACTING ON COMPLETED STRUCTURES. A SEPARATE ASSESSMENT SHALL BE UNDERTAKEN TO DETERMINE CONSTRUCTION LOADS AND REQUIRED TEMPORARY BRACING. PROPPING AND THE LIKE REQUIRED DURING CONSTRUCTION. STRUCTURES SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERSTRESSED.
- ANY CONSTRUCTION VEHICLE LOADING TO STRUCTURES SHALL 2 BE SUBMITTED TO THE DSR FOR APPROVAL.
- ALL WELDS TO COMPLY WITH AUSTRALIAN STANDARD AS 3. 1554. FILLET WELDS TO BE NOT LESS THAN 6mm UNO.
- ALL SETOUT POINTS TO CENTRE OF STRUCTURE UNLESS 4 NOTED OTHERWISE.
- FOUNDATIONS FOR ALL DRAINAGE STRUCTURES SHALL BE 5 INSPECTED BY THE DSGR AND THE NEED FOR FOUNDATION IMPROVEMENT IN THESE AREAS ADVISED.
- PIPE LENGTHS FOR DRAINAGE SHALL BE TO THE NEAREST 6 MULTIPLE OF 122m FOR PIPES AND FOR BOXES.
- 7. HEADWALL APRONS TO BE TYPE 3 AS PER TMR STANDARD DRAWING 1318.

#### REINFORCEMENT

- REINFORCEMENT STEEL TO BE AUSTRALIAN MADE GRADE 500N 13 TO AS/NZ54671 UNO.
- 2. STEEL DOWELS SHALL BE GRADE R250N TO AS/NZS 4671.
- BAR SHAPES ARE SHOWN ON TMR STANDARD DRAWING 1043 3. AND 1044 UNO.

- 4. LAPS AND SPLICES IN REINFORCEMENT SHALL ONLY BE MADE AT THE POSITIONS SHOWN ON THE DRAWINGS UNLESS APPROVED BY THE DSR.
- SIDE AND END LAPS IN WELDED MESH FABRIC SHALL BE MADE 5 SO THAT THE TWO OUTERMOST TRANSVERSE WIRES OF ONE SHEET OF MESH OVERLAP THE TWO OUTERMOST TRANSVERSE WIRES OF THE SHEET BEING LAPPED. REFER AS3600 FOR FURTHER DETAILS.
- THE MINIMUM DEVELOPMENT LENGTHS AND LENGTHS OF LAPS 6 FOR GRADE 500N BARS SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

BAR SIZE	N12	N16	N20	N24	N28
MINIMUM LAP LENGTH	375	500	750	900	1200

#### COVER TO REINFORCEMENT

IF MINIMUM CLEAR COVER TO REINFORCEMENT IS NOT COVERED BY THE STANDARD DRAWINGS, SPECIFICATIONS OR PROJECT DETAILS, COVER TO BE AS FOLLOWS:

LOCATION	EXPOSURE	COVER	CONCRETE
WHERE STANDARD	82	45mm	40MPa/20mm
COMPACTION IS USED	c	70mm	50MPa/20mm
WHERE CAST AGAINST THE GROUND IF THE	82	55mm	40MPa/20mm
DAMP-PROOF MEMBRANE	c	80mm	50MPa/20mm
WHERE CAST AGAINST	B2	65mm	40MPa/20mm
THE GROUND	C	100mm	SOMPa/20mm

- SHALL BE IN ACCORDANCE WITH THE 'MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES' (MUTCD).
- WHERE LINEMARKING MATCHES ONTO EXISTING ROADS THE SETOUT IS TO BE CONFIRMED BY THE DSR PRIOR TO COMMENCING WORK
- 3. MRTS 45 UND.

- ACCORDANCE WITH THE MUTCO.
- DIMENSIONS OF SIGN FACES SHALL NOT BE SCALED OFF THE
- SIGN POSTS TO BE 50NB CHS AND INSTALLED IN ACCORDANCE WITH TMR STANDARD DRAWING 1368.

#### BARRIERS

FOR DETAILS OF BARRIER SYSTEMS, INCLUDING STEEL BEAM GUARDRAILS, TERMINALS, TRANSITIONS, CONNECTIONS ETC. REFER TO THE TMR STANDARD DRAWINGS.

#### FENCING

- FENCING AND GATES TO BE INSTALLED IN ACCORDANCE WITH MARANOA REGIONAL COUNCIL STANDARDS.
- FENCES TO BE REINSTATED TO MATCH THE EXISTING FENCE LINES AS REQUIRED.
- FENCE STRUTS AT GATE LOCATIONS TO BE ROTATED 90\* TO 3 RUN PARALLEL WITH LEVEE

#### ROAD EDGE GUIDE POSTS

- ROAD EDGE GUIDE POSTS ARE TO BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS OF THE MUTCO.
- 2. REFER TO TMR STANDARD DRAWING 1356 FOR DETAILS.

RED DELINEATOR ROAD EDGE GUIDE POSTS WHITE DELINEATOR

RED DELINEATOR DELINEATOR ON GUARDRAIL WHITE DELINEATOR

LOCATION SHOWN ON THE DRAWINGS ARE A GUIDE ONLY. LOCATIONS AT TURNOUTS, PRIVATE ENTRANCES & CULVERT STRUCTURES SHALL BE AS SHOWN ON THE DRAWINGS. THE MAXIMUM DISTANCE BETWEEN GUARDRAIL DELINEATORS SHALL BE 40m. AT ALL OTHER LOCATIONS, THE ACTUAL LOCATIONS SHALL BE DETERMINED IN ACCORDANCE WITH STANDARD DRAWING 1356 & PART 2 CLAUSE 4.2.4.4 OF THE MUTCO

#### PUBLIC UTILITIES

- 1. THE PRESENT AND/OR PROPOSED POSITIONS OF PUBLIC UTLITIES/SERVICES, FITTINGS, PIPES, POLES, MANHOLES, ETC. MAY BE INDICATED ON THE DRAWINGS, HOWEVER THE DRAWINGS ARE NOT AN ACCURATE REPRESENTATION OF THEIR PRESENCE OR OMISSION THEREOF
- THE LOCATION OF PUBLIC UTILITIES/SERVICES SHOWN ON THESE DRAWINGS HAVE BEEN APPROXIMATED FROM DIAL-BEFORE-YOU-DIG DATA. POTHOLING INFORMATION SUPPLIED BY SERVICE AUTHORITIES.
- 3 ALL NECESSARY AUTHORITIES SHALL BE CONTACTED TO VERIFY THE SIZE, TYPE, LOCATION AND LEVEL OF EXISTING PUBLIC UTLITIES/SERVICES PRIOR TO ANY EXCAVATION.
- 4. ALL NECESSARY MEASURES SHALL BE PROVIDED TO PROTECT PUBLIC UTILITIES/SERVICES DURING THE COURSE OF THE WORK
- 5. NOT ALL PUBLIC UTILITY/SERVICE CONFLICTS HAVE NECESSARILY BEEN IDENTIFIED AND SHOWN ON THE DRAWINGS.
- 6. THE DSR IS TO BE ADVISED IMMEDIATELY OF ANY CONFLICT WITH PUBLIC UTILITY/SERVICE NOT IDENTIFIED ON THE DRAWINGS
- 7. NO WORK IS TO BE CARRIED OUT OVER OR WITHIN 3.0m OF PUBLIC UTILITY/SERVICES WITHOUT THE PRIOR NOTIFICATION OF THE RELEVANT SERVICE AUTHORITY.
- 8. TELECOMMUNICATION CABLE IS TO BE MANUALLY EXPOSED AND PROTECTED WHEN EXCAVATING, OR WORKING, CLOSER TO THE TELECOMMUNICATION CABLE THAN 3.0m.

#### MATCH TO EXISTING

- 1. PRIOR TO COMMENCEMENT WORK AT AN AREA WHERE THE WORKS JOIN TO EXISTING CONSTRUCTION, THE CONTRACTOR SHALL
  - UNDERTAKE SURVEY OF THE AREA.
  - . SUBMIT FOR APPROVAL DETAILS OF THE TIE IN AND NEW SURVEY TO THE DSR.

#### LEGISLATION

1. ALL CONSTRUCTION SHALL COMPLY WITH REVENANT CURRENT WORK PLACE HEALTH AND SAFETY LEGISLATION.

#### SCOPE OF WORKS

CARNARVON HIGHWAY LEVEE CROSSINGS



2 .	EXTERNAL REFERENCE PALIS	REV DATE AND MENTION DESCRIPTION	VOR	APPROV	WAL TITLE	KANE	SCALES AT AT SIZE DRAWING	DESIGNER	CLENT
-		A 28/01/2014 ISSUED FOR CONSTRUCTION	008	AZ	ORAFTER	B. TAYLOR		SMEC	
*		B 30/01/2014 REISSUED FOR SECTIONS BAD	019	AZ	<b>DRAFTING CHECK</b>	J. RAISON			
2		D 31/03/2014 BIRKETT LEVEE REALIGNMENT	925	AZ	DESIGNER	J. RAISON	NTS	SMEC AUSTRALIA PTY LTD	1 ma
-		E 10/04/2014 NOTES AMENDED	033	AZ	DESIGN CHECK	J. CARROLL		LEVEL 1 154 MELBOURNE STREET	IIICI
2	-	F 16/95/2014 LEGEND UPDATED	032	AZ	PROJECT MANAGER	A ZANETTI		SOUTH BRISBANE QLO 4101 PH 07 3029 6600 FAX 07 3029 6650	REGION
Ξ.		H 7/07/2014 NOTE AMENDED		A	PROJECT DIRECTOR	R. STRINGFELLOW		SMEC PROJECT No 30031182	

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  - ALL LINEMARKING PAINT TO BE TYPE 3 IN ACCORDANCE WITH

#### SIGNS

- THE LOCATION AND HEIGHT OF SIGNS SHALL BE IN 1
- 2



DRAINAGE FENCING

ELECTRICAL -E-E-E---

-X XEX X X X XEX X

OPP -dE-dE-

TELECOMMS

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UTILITIES

**ISSUED FOR CONSTRUCTION** 

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ranoa AL COUNCIL

EXTENT OF WORKS

**DB** 

EXISTING PROPERTY BOUNDARY PROPOSED CLEARING BOUNDARY EXISTING FEATURES EXISTING TREE EXISTING TREE TO BE REMOVED

EARTHWORKS **EROSION PROTECTION - PLACED ROCK** d50 = 300mm **EROSION PROTECTION - PLACED ROCK** d50 = 550mm **EROSION PROTECTION - TURF** REINFORCEMENT MATTING **EROSION PROTECTION - RENO MATTRESS** 

BARRIER LINE - DOUBLE 081 BARRIER LINE - ONE DIRECTION BL1 LANE LINE - SINGLE ш EDGE LINE - SINGLE EL

SIGN LOCATION EXISTING SIGN LOCATION EXISTING SIGN TO BE RELOCATED

RELOCATED SIGN LOCATION

EXISTING SIGN TO BE REINSTATED

CULVERT WITH HEADWALL OVERLAND FLOW PATH FREE DRAINING DRAINAGE CHANNEL

FENCE AND POST INACCORDANCE WITH DRG CMDG-G-010 GATE INACCORDANCE WITH DRG CMDG-G-017 EXISTING FENCE AND POST

EXISTING OVERHEAD ELECTRICITY EXISTING ELECTRICAL CONDUIT CONFLICT UNDER LEVEE TO BE TREATED. REFER DRG. TC-0103 EXISTING OVERHEAD ELECTRICITY TO BE REMOVED BY OTHERS EXISTING POWERPOLE DBYD ABOVE GROUND ELECTRICITY DBYD UNDERGROUND ELECTRICITY

EXISTING TELSTRA EXISTING TELSTRA CONDUIT CONFLICT UNDER LEVEE TO BE TREATED. REFER DRG. TC-0103 **DBYD TELSTRA** 

EXISTING WATER EXISTING WATER MAIN CONFLICT UNDER LEVEE TO BE TREATED. REFER DRG TC-0103

MARANOA REGIONAL COUNCIL ROMA FLOOD MITIGATION PROJECT **GENERAL NOTES** 

AND LEGEND

30031182-DI-0011 CONSTRUCTION

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	EXTERNAL REPERENCE FILES	REV	DATE	AMENDMENT / REVISION DESCRIPTION	SVR No.	APPROVAL	TIPLE	NAME	SCALES AT AT SIZE DRAWING	DESCHER	CLIONT
-		A	20/01/201	SSUED FOR CONSTRUCTION	608	AZ	ORAFTER	B. TAYLOR		SMEC	
2		B	30/01/201	REISSUED FOR SECTIONS BAD	019	AZ	DRAFTING CHECK	J. RAISON			
2		c	31/03/201	4 BIRKETT LEVER REALIGNMENT	930	AZ	DESIGNER	J. RAISON	NTS	SMEC AUSTRALIA PTY LTD	mai
a		E	16/05/201	GOLDERS LEVEE REALIGNMENT	032	AZ	DESIGN CHECK	J. CARROLL		LEVEL 1 154 MELBOURNE STREET	
2		- <b>F</b>	7/67/201	A HEIO LEVELS AMENDED	180		PROJECT MANAGER	A. ZANETTI		SOUTH BRISBANE QLD 4101 PH 07 3029 6600 FAX 07 3029 6650	REGION
	(L					( i	PROJECT DIRECTOR	R. STRINGFELLOW		SMEC PROJECT No 30031182	

**4 STRAND BARBED WIRE** CMDG-G-010 FN31 42 **4 STRAND BARBED WIRE** CMDG-G-010 **FN32** 29 WITH GATE **4 STRAND BARBED WIRE** CMDG-G-010 **FN33** 15 **4 STRAND BARBED WIRE** 48 CMDG-G-010 FN34 WITH GATE ISSUED FOR CO

ENCE SETOUT	FI				MC10	ROL LINE	CONT		· · · · · · · · · · · · · · · · · · ·		MC10	ROL LINE	CONT		
TYPE	LENGTH (m)	FENCE NUMBER	RADIUS	BEARING	LEVEL	NORTHING	EASTING	CHAINAGE	ADIUS PT	BEARING	LEVEL	NORTHING	EASTING	CHAINAGE	PT
MATC	73	EN01	2	80*28'45.85"	304.836	7062954.442	678694.417	2229.456	TC	103°45'38.77"	304.75	7063695.937	677003.432	0	IP1
	10	1.0x1	500 )		304.632	7062978.52	678837.983	2371.111	IP14	103°45'38,77"	304.945	7063663.038	677137.77	138.308	TC
4 STRAND BARBED WI	4	FN02		112*56'39.77"	304.401	7062921.771	678972.037	2512.767	50 CT		304.787	7063655.242	677169.6	167.317	IP2
	1224		15	112*56'39.77"	304.32	7062893.938	679037.784	2584.162	TC	170*14"37.03"	304.957	7063622.946	677175.154	196.326	(T
4 STRAND BARBED WI	191	FN03	100		304.305	7062866.612	679102.336	2645.301	iP15	170*14'37.03*	304.971	7063412.88	677211.274	409.474	TC
4 STRAND BARBED WI	4	EN04		183°00'13.66"	304.254	7062796.61	679098.663	2706.439	-100 CT		304.975	7063345.945	677222.783	469.135	IP3
				183*00'13.66"	304.208	7062741.195	679095.755	2761.93	TC	101*52'37.98"	304.979	7063331.967	677289.246	528 796	CT
4 STRAND BARBED WI	4	FN05	-300		304.177	7062703.043	679093.753	2799.93	IP16		304.779	7063286.132	677507.177	751.495	IP4
4 STRAND BARBED WI			4	168*29'20.48*	304.146	7062665.608	679101.377	2837.929	CT		304.783	7063316.653	677501.28	782.58	IPS
Im FROM TOE OF BATT	85	FN06		168*29'20.48"	303.946	7062540.356	679126.884	2965.752	TC	79*09'38.53"	304.99	7063335 369	677599.029	882.104	TC
TYPICALLY			100		303.822	7062502.421	679134.61	3002.689	50 IP17		304.99	7063339.596	677621.104	903.227	íP6
4 STRAND BARBED WI	57	FN07		210*48'55.95"	303.698	7062469.172	679114.778	3039.626	CT	127*34'12.94"	304.99	7063325.891	677638.918	924.349	CT
WITH GATE			5	210*48'55.95"	303.55	7062431.548	679092.335	3083.436	TC	127*34'12.94"	304.991	7063310.157	677659.372	950.155	TC.
4 STRAND BARBED WI	.7	FN08	-100		303.425	7062397.918	679072.275	3120.759	10 IP18		304.991	7063307.584	677662.716	954.148	IP7
4 STRAND BARBED WI	32	ENAG	)	168*02*47.82*	303.356	7062359.609	679080.386	3158.081	CT	173*19'41.86"	304.991	7063303 393	677663.207	958.141	CT
WITH GATE		F NV3		168*02*47.82*	302.516	7061774.346	679204 289	3756.315	TC	173*19'41.86"	304.991	7063245.29	677670.003	1016.64	TC
4 STRAND BARBED WI	44	EN10	-300		302 468	7061756.679	679208.03	3774.353	-10 IP19		304.991	7063239.516	677670.678	1021.905	IP8
		15-0415	<	161°09'24.23"	302.42	7061739.587	679213.863	3792.391	CT	112*59'07.96"	304.983	7063237,245	677676.03	1027.172	CT.
WITH GATE	47	FN11	)	161*09'24.23"	302.167	7061622.32	679253.882	3916.299	TC	112*59'07.96"	304.922	7063216.282	677725.453	1080.857	TC
			300 )		302.071	7061544.862	679280.316	3996.198	-10 IP20		304.955	7063215.054	677728.347	1083.903	IP9
		*2	5	191*40'34.03"	302.014	7061464.712	679263.753	4076.098	CT	78*04'51.29"	304.987	7063215.703	677731.423	1086.949	CT
2	12	23	<	191*40'34.03"	301.632	7061270.54	679223.626	4274.373	TC	78*04'51.29"	304.991	7063226.679	677783.423	1160.095	TC
			500		301.613	7061250.283	679219.44	4295.045	10 IP21		304.992	7063228.793	677793.439	1148.065	IP10
2		÷.		196°24'50.07"	301.594	7061230.442	679213.595	4315.718	CT	169*25'02.98*	304.992	7063218.732	677795.318	1156.036	CT
4 STRAND BARBED WI	151200		5	196°24'50.07"	301 365	7060912.388	679119.903	4647.285	TC	169*25'02.98*	304.992	7063207.035	677797.504	1167.935	TC
WITH GATE	186	FN15	-300		301.359	7060869.947	679107.401	4691.212	-100 IP22		304.992	7063201.087	677798.615	1173.978	IP11
4 STRAND BARBED WI	12	EN16	1	179*38'06.71	301.105	7060825.705	679107.682	4735.139	CI	162*29'34.09"	304.992	7063195.317	677800.435	1180.021	CT
WITH GATE		1110		179*38'06.71	301.223	7060729.666	679108.294	4831,18	TC	162*29'34.09"	304.99	7063108.698	677827.758	1270.848	TC
4 STRAND BARBED WI	32	EN17	-300		301,151	7060565.726	679109.338	4981.22	-200 IP23	Conservation of the second	304.987	7063029.561	677852.72	1349.505	IP12
BIIII GAIL	1		5	122*19'27.04"	300.676	7060478.065	679247.875	5131.26	13	117*25'32.95"	304.985	7062991341	677926.374	1428.161	CT.
4 STRAND BARBED WI	38	FN18	<	122*19'27.04"	300.231	7060467.01	679265.346	5151.934	1P24	117°25'32.95"	304.978	7062896.882	678108.403	1633.24	TC
4 STRAND BARBED W	99	CNIG	)					ALC: THE P	-200		304.976	7062866.107	678167.709	1697.724	IP13
WITH GATE	<u>, 20    </u>	1003	)							80*28'45.85"	304.974	7062877.158	678233.604	1762.207	CT
4 STRAND BARBED W WITH GATE	43	FN20	$\sim$	~~~	~~	~~~	~~~	~~~	T	~~~	~~		~~~	in	~
4 STRAND BARBED W WITH GATE	42	FN21				A									
4 STRAND BARBED W WITH GATE	42	FN22				- CL-3									
4 STRAND BARBED W WITH GATE	44	FN23													
4 STRAND BARBED W	43	EN24													

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ENCE NUMBER	LENGIH (m)	4 STRAND BARBED WIR	E CUDE C LU
FN35	21	WITH GATE	LMDG-G-010
FN36	49	4 STRAND BARBED WIR WITH GATE	CMDG-G-010
FN37	85	4 STRAND BARBED WIR	E CMDG-G-010
FN38	31	4 STRAND BARBED WIR WITH GATE	E CMDG-G-010
3	545		4
FN40	45	4 STRAND BARBED WIR	E CMDG-G-010
FN40a	78	4 STRAND BARBED WIR	E CMDG-G-010
EN41	40	4 STRAND BARBED WIR	RE CMDG-G-010
	144	WITH GATE 4 STRAND BARBED WIR	E cupe e au
FN4Z	37	WITH GATE	CMDG-G-010
FN43	37	WITH GATE	CMDG-G-010
FN44	37	4 STRAND BARBED WIR WITH GATE	CMDG-G-010
FN45	38	4 STRAND BARBED WIR WITH GATE	CMDG-G-010
FN46	137	4 STRAND BARBED WIF	CMDG-G-010
EN47	38	4 STRAND BARBED WIF WITH GATE	CMDG-G-010
FN48	35	4 STRAND BARBED WIF	CMDG-G-010
FN49	34	4 STRAND BARBED WIF	E CMDG-G-010
FN50	39	GALVANISED STEEL PO	ST WITH CAP AND GATE, 1m
EN51	44	GALVANISED STEEL	POST WITH CAP , Im DEEP
ENEO	6	GALVANISED STEEL	POST WITH CAP , 1m DEEP
1492	3	CONCRE GALVANISED STEEL	TE FOOTING POST WITH CAP . Im DEEP
FN53	52	CONCRE	TE FOOTING
FN54	115	4 STRAND BARBED WIE WITH GATE	CMDG-G-010
<b>FNSS</b>	46	4 STRAND BARBED WIF	RE CMDG-G-010
FN56	163	4 STRAND BARBED WIE WITH GATE	CMDG-G-010
EN57	-30	REMOVE AND	REPLACE EXISTING
FN58	220	4 STRAND BARBED WIF	RE CMDG-G-010
FN59	95	4 STRAND BARBED WI	RE CMDG-G-010
FN60	13	4 STRAND BARBED WI	RE CMDG-G-017
EN61	12	4 STRAND BARBED WI	RE CMDG-G-010
EN42	41	WITH GATE 4 STRAND BARBED WI	RE CHOG_G_010
1102		WITH GATE 4 STRAND BARBED WI	RE CHOC C AN
FN63	46	WITH GATE	CMDD-G-010
FN64	179	WITH GATE	CMDG-G-010
FN65	89	4 STRAND BARBED WI WITH GATE	CMDG-G-010
FN66	135	4 STRAND BARBED WI WITH GATE	CMDG-G-010
FN67	16	4 STRAND BARBED WI	RE CMDG-G-010
		NOTES	
NSTR	UCTIO	N 1. FOR GEN 30031182	ERAL NOTES REFER TO DRAY
	PROJECT TITLE	MARANOA REG	GIONAL COUNCIL
	C	ONTROL LINE AN	D FENCING SETOUT
inoa	6	TA	BLES
COUNCIL	SCALE	Privast	PROJECT / DRAWING No.
	NTS	CONSTRUCTION	30031182-CL-1005

REFERENCE DRAWING

CMDG-G-010

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CMDG-G-010

**4 STRAND BARBED WIRE** 

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FN25

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**FN27** 

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	2442	2 492	2.458	2,451	81.7 6	2166	2 208	2.058
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AS SHOWN CONSTRUCTION 30031182-WP-1008

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CONTROL LINE MCP10										
PT	CHAINAGE	EASTING	NORTHING	LEVEL	BEARING	RADIUS				
START	0	679003.109	7060807.915	298.543	359°35'48"					
TC	12	679003.024	7060819.915	298.231	359°35'48"					
IP	20.078	679002.951	7060830.374			10				
СТ	28.156	679013.402	7060829.978	298.173	92°09′59"					
TC	56.958	679042.183	7060828.89	298.087	92°09′59"					
IP	67.062	679054.147	7060828.437			- 15				
END	77.167	679057.241	7060840.003	297.839	14°58′32″					
				24		1				

	CONTROL LINE MCB1									
PT	CHAINAGE	EASTING	NORTHING	LEVEL	BEARING					
TART	0	679078.816	7060837.288	297.628	90°16'26.34"					
END	58.555	679137.371	7060837.008	297.382	90°16'26.34"					

	SETOUT	DETAILS	5 MKA1		
CHAINAGE	EASTING	NORTHING	LEVEL	RADIUS	CHAIN
0	678941.290	7060822.768	298.156		0
2	678942.297	7060821.040	298.143	-20	2
4	678943.389	7060819.365	298.13	-20	4
6	678944.642	7060817.808	298.117	-20	6
8	678946.045	7060816.383	298.104	-20	8
10	678947.583	7060815.106	298.091	-20	10
12	678949.240	7060813.988	298.079	-20	12
14	678951.001	7060813.042	298.066	-20	14
16	678952.848	7060812.276	298.055	-20	16
18	678954.762	7060811.698	298.045	-20	18
20	678956.724	7060811.314	298.036	-20	18.4
22	678958.714	7060811.128	298.028	-20	19.8
23.004	678959.718	7060811.110	298.024	-20	

NAME

PROJECT MANAGER A. ZANETTI PROJECT DIRECTOR R. STRINGFELLOW

B. TAYLOR

J. RAISON

J. RAISON

J. CARROLL

WVR No. APPROV

AZ AZ AZ DRAFTER

DESIGNER

DESIGN CHECK

DRAFTING CHECK

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		SETOUT	DETAILS	6 MKA2	
	CHAINAGE	EASTING	NORTHING	LEVEL	RADIUS
	0	678961.653	7060804.124	298.019	-17.5
	2	678959.658	7060803.995	298.026	-17.5
	4	678957.691	7060803.641	298.039	-17.5
	6	678955.777	7060803.064	298.057	-17.5
	8	678953.941	7060802.272	298.08	-17.5
	10	678952.208	7060801.277	298.109	-17.5
	12	678950.599	7060800.09	298.138	-17.5
	14	678949.137	7060798.728	298.163	-17.5
	16	678947.839	7060797.207	298.184	-17.5
	18	678946.723	7060795.549	298.2	-17.5
	18.427	678946.509	7060795.179	298.203	-17.5
	19.875	678945.802	7060793.916	298.211	
1					

SCALES AT A1 SIZE DRAWING

2.5 0 SCALE 1:250

SETOUT DETAILS MKA3							SETOUT	DETAILS	6 MKA4	
)	CHAINAGE	EASTING	NORTHING	LEVEL	RADIUS	CHAINAGE	EASTING	NORTHING	LEVEL	RADIUS
)	0	678993.077	7060812.345	298.212	-7.5	0	679005.024	7060819.929	298.191	-7.5
)	2	678995.052	7060812.624	298.237	-7.5	2	679005.303	7060817.955	298.243	-7.5
)	4	678996.883	7060813.413	298.246	-7.5	4	679006.093	7060816.124	298.318	-7.5
)	6	678998.441	7060814.657	298.239	-7.5	6	679007.337	7060814.565	298.415	-7.5
)	8	678999.617	7060816.268	298.223	-7.5	8	679008.947	7060813.39	298.512	-7.5
)	10	679000.326	7060818.132	298.203	-7.5	10	679010.811	7060812.68	298.592	-7.5
)	11.781	679000.524	7060819.898	298.181	-7.5	11.781	679012.577	7060812.482	298.649	-7.5

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SMEC

SMEC AUSTRALIA PTY LTD (a) ABN 47 065 475 149 LEVEL 1 154 MELBOURNE STREET SOUTH BRISBANE OLD, 4101 PH 07 3029 6600 FAX 07 3029 6650 SMEC PROJECT No 30031182

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REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR N
Α	20/01/2014	ISSUED FOR CONSTRUCTION	008
в	30/01/2014	PRIVATE ACCESS ALIGNMENT REVISED	019
С	31/03/2014	BIRKETT LEVEE REALIGNMENT	030

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EXTERNAL REFERENCE FILES







CONTROL LINE MCP6									
PT	CHAINAGE	EASTING	NORTHING	LEVEL	BEARING				
START	0	677174.022	7063663.153	302.789	225*01'01.02"				
END	52.107	677137.166	7063626.318	303.336	225*01'01.02"				

EXTERNAL REFERENCE FLES	θlγ.	DATE	AMENDMENT / REVISION DESCRIPTION	WVX No.	APPROVAL	101	NAME	SCALES AT AT SIZE DRAMING	DESIGNER	a state
		16/05/2014	ISSUED FOR CONSTRUCTION	032	10-	DRAFTER	8. TAYLOR	Table Chronic Lines of All Theory Steel		SM
	1	1.11.11.5.5		1005	-	DRAFTING CHECK	J. RAISON			
	L 1					DESIGNER	J. RAISON	stars 1988 25 14 5 19		SMEC AUSTRALIA PTY
1	L					DESIGN CHECK	J. CARROLL			LEVEL 1 154 MELBOURNE S
						PROJECT MANAGER	A. ZANETTI			SOUTH BRISBANE GLD, 4 PH 07 3029 6600 FAX 67 303
						PROJECT DIRECTOR	R STRINGFELLOW			SMEC PROJECT No 300



		CONT	ROL LINE	MCP7	4	
PT	CHAINAGE	EASTING	NORTHING	LEVEL	BEARING	RADIUS
START	0	678536.646	7062867.206	301.113	355°32'41.09"	
TC	14.219	678535.541	7062881.382	301.113	355*32"41.09"	
IP	16.675	678535.35	7062883.839	301.155		25
CT	19.132	678535.642	7062886.286	301.258	6°48'17.49"	
TC	35.88	678537,626	7062902.916	302.829	6°48'17,49"	
IP	39.882	678538.105	7062906.924	303.229		-75
CT	43.884	678537.297	7062910.879	303.629	348°27'40.58"	
END	107.584	678524.555	7062973.292	301427	348°27'40.58"	

TE PALES.	REV	DATE	AMENCHENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	AME	SCALES AT AT SIZE DRAWING	DESEMP CONTRACTOR	Dit .
		16/05/2010	ISSUED FOR CONSTRUCTION	032	AZ	DRAFTER	B. TAYLOR		SMEC	
						ORAFTING CHECK	J. RAISON	SCALE 1250 23 10		
						DESIGNER.	J. RAISON		SMEC AUSTRALIA PTY LTD	marar
					1	DESIGN CHECK	J. CARROLL		C) ANN 47 OIG 475 148 LEVEL ) 154 MELBOURNE STREET	Amara
					1 3	PROJECT MANAGER	A. ZANETTI		SOUTH BRISBANE QLD, 4101 PH 07, 3029 6600 EAX 07, 3039 6650	HEGIONAL CO
				_		PROJECT DIRECTOR	R. STRINGFELLOW		SMEC PROJECT No 30031182	





TYPICAL DUAL PROPERTY ACCESS CROSSING PLAN SCALE 1:200

TYPICAL SINGLE PROPERTY ACCESS CROSSING PLAN SCALE 1:200

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SMEC AUSTRALIA PTY LTD © ABN 47 065 475 149 LEVEL 1 54 MELBOURNE STREET SOUTH BRISBANE QLD. 4101 PH 07 3029 6600 FAX 07 3029 6650 SMEC PDC JECT No. 30031182	1 mar

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EXTERNAL REFERENCE FILES	REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	NAME	SCALES AT A1 SIZE DRAWING
	A	20/01/2014	ISSUED FOR CONSTRUCTION	008	AZ	DRAFTER	B. TAYLOR	
	B	30/01/2014	REISSUED FOR SECTIONS B&D	019	AZ	DRAFTING CHECK	J. RAISON	2 0 2 4 6 8 10
	Ľ	517 057 2014		050	AL	DESIGNER	J. RAISON	SCALE 1:200
						DESIGN CHECK	J. CARROLL	1 0 1 2 3 4 5
						PROJECT MANAGER	A. ZANETTI	SCALE 1:100
						PROJECT DIRECTOR	R. STRINGFELLOW	
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anoa	PROJECT TITLE	MARANOA F ROMA FLOOD I PERTY ACCESS PROPERTY A	REGIONAL COUNCIL MITIGATION PROJECT S - LEVEE CROSSI ACCESS DETAILS	NGS
	scale 1:200	PHASE CONSTRUCTION	PROJECT / DRAWING No. 30031182-PA-0101	C

NOTES 1. FOR GENERAL NOTES REFER TO DRAWING 30031182-DI-0011  $\overline{}$ 

### NOTES

#### GENERAL PAVEMENT NOTES

- 1. DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE FOLLOWING DOCUMENTS DETAILED DESIGN REPORT
  - TMR STANDARD DRAWINGS

### TECHNICAL SPECIFICATIONS

SUPPLEMENTARY SPECIFICATIONS

#### WRITTEN INSTRUCTIONS

- ALL CONSTRUCTION MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT SPECIFICATION FOR THE WORKS TOGETHER WITH THE REQUIREMENTS OF ALL THE RELEVANT CODES OF PRACTICE REFERRED TO THEREIN AND THE REQUIREMENTS OF ALL THE STATUTORY AUTHORITIES WHERE APPLICABLE.
- 3. ALL LOCATIONS, ORIENTATION AND LEVELS SHALL BE VERIFIED ON SITE BEFORE COMMENCING ANY WORK. DISCREPANCIES SHALL BE REFERRED TO THE CONTRACT ADMINISTRATOR.
- 4. DO NOT OBTAIN DIMENSIONS FROM SCALING OFF DRAWINGS. 5. ANY PERMITS AND APPROVALS REQUIRED FOR CONSTRUCTION
- OF PERMANENT OR TEMPORARY WORKS SHALL BE OBTAINED BY THE CONTRACTOR.
- 6. TMR STANDARD DETAILS TO BE ADOPTED UNLESS OTHERWISE STATED.
- DRAINAGE, EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IN ACCORDANCE WITH ENVIRONMENTAL MANAGEMENT PLAN (CONSTRUCTION).
- THE WORKS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACTOR'S APPROVED CONSTRUCTION WORKS PROCEDURES/ WORK METHOD STATEMENT.
- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING ANY INFRASTRUCTURE (E.G. CULVERTS OR STORMWATER DRAINAGE PIPES) WILL NOT INHIBIT CONSTRUCTION OF THE NOMINATED PAVEMENT THICKNESSES GIVEN.
- ALL VARYING WIDTHS AND DEPTHS SPECIFIED ON THE DRAWINGS SHALL BE CONFIRMED ON SITE BY THE CONTRACT ADMINISTRATOR.
- 11. ALL WORKS ARE TO BE SUPERVISED BY A SUITABLY QUALIFIED RPEQ FAMILIAR WITH THE TYPE OF WORK BEING UNDERTAKEN IN ACCORDANCE WITH THE REQUIREMENTS OUTLINED IN THE ENGINEERING ACT 2002.
- 12 ALL MATERIALS ARE TO BE SUPPLIED AND PLACED IN ACCORDANCE WITH THE FOLLOWING SPECIFICATIONS: PROVISION FOR TRAFFIC - MRTS 02

PRIME - MRTS20

POLYMER MODIFIED SEALS - MRTS 11, MRTS 18 & MRTS22

PLANT MIXED STABILISED PAVEMENT USING CEMENT OR CEMENTITIOUS BLENDS - MRTS08

PLANT MIXED STABILISED PAVEMENT USING CEMENT OR CEMENTITIOUS BLENDS - MRTS07B

EARTHWORKS - MRTS04

GEOFABRIC - MRTS27

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EXTERNAL REFERENCE FLES

#### UNBOUND PAVEMENT GRAVELS - MRTS 05

ENVIRONMENTAL MANAGEMENT - MTRS 51

- 13. ALL PROPRIETARY PRODUCTS ARE TO BE SUPPLIED AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- 14. COLOUR PRINTS OF DRAWINGS ONLY SHOULD BE USED AS REFERENCE FOR CONSTRUCTION.
- 15. WORKS ARE TO BE UNDERTAKEN AT TIMES TO MINIMISE THE INFLUENCE OF RAIN AND/OB SEASONAL GROUND WATER
- 16 LOCAL DRIVEWAYS AND ACCESS POINTS ARE TO BE 150mm TMR TYPE 2.3 BASE COURSE WITH A SPRAY SEAL OR AS OTHERWISE REQUESTED BY THE LAND OWNER

- SUBGRADE
- THE SUBGRADE IS TO BE INSPECTED PRIOR TO PLACEMENT OF ANY PAVEMENT LAYERS IN ACCORDANCE WITH MRTS04.
- 2. THE CONTRACT ADMINISTRATOR MAY DIRECT ADDITIONAL TESTING OF THE SUBGRADE FOLLOWING THE INSPECTION AND PRIOR TO PLACEMENT OF ANY PAVEMENT LAYERS.
- 3. ONCE THE SUBGRADE HAS BEEN INSPECTED AND SUBGRADE TEST RESULTS ARE RECEIVED BY THE CONTRACT ADMINISTRATOR THE NEED FOR ANY SUBGRADE IMPROVEMENT. SUBGRADE CUTTING TREATMENT OR ADJUSTMENT TO THE PAVEMENT THICKNESSES WILL BE ADVISED BY THE CONTRACT ADMINISTRATOR.
- 4. PAVEMENT TRANSITIONS SHALL BE CONFIRMED ON SITE BASED ON ACTUAL SUBGRADE CONDITIONS. THE CONTRACTOR SHALL OBTAIN CONFIRMATION OF ALL PAVEMENT TRANSITIONS FROM THE CONTRACT ADMINISTRATOR PRIOR TO THE PLACEMENT OF ANY PAVEMENT LAYERS.

#### EARTHWORKS

- ANY FILL MATERIAL SHALL COMPLY WITH THE PROJECT SPECIFIC REQUIREMENTS.
- 2. MATERIAL PLACED SHALL BE PROTECTED FROM RAIN DAMAGE BY SEALING, ROLLING OR COVERING AT THE END OF EACH WORK DAY AND/OR PRIOR TO INCLEMENT WEATHER.
- THE EXCAVATION WORK SHALL BE UNDERTAKEN IN SUCH A WAY TO PREVENT ANY PONDING OF STORMWATER ACROSS ALL AREAS OF THE WORK.

#### WORKING PLATFORMS

 EXCESS EXISTING MATERIAL EXCAVATED FROM VARIOUS PAVEMENT BOXES MAY BE RE-USED FOR SUCH PURPOSES IF FOUND TO BE SUITABLE. VERIFICATION OF THE SUITABILITY OF ANY MATERIAL SHALL BE AT THE DISCRETION OF THE CONTRACT ADMINISTRATOR.

#### CMB

- THE CMB MAY BE ACHIEVED USING PLANT MIXED OR INSITU STABILISATION PROCESSES IN ACCORDANCE WITH THE RESPECTIVE TMR SPECIFICATIONS.
- 2. THE THICKNESS OF CMB SHOWN IS THE MINIMUM THICKNESS REQUIRED, AND IF INSITU STABILISATION OPTION IS ADOPTED, THE CONTRACTOR SHALL PROVIDE A BUFFER OF 20MM OF ADDITIONAL GRAVEL TO BE STABILISED OVER THE CBR 35% SELECT FILL TO ENSURE THAT THE CBR 35% MATERIAL IS NOT UNINTENTIONALLY INCORPORATED INTO STABILISATION BLEND.
- THE BINDER CONTENT FOR THE CMB SHALL BE DETERMINED BY LABORATORY TESTING OF REPRESENTATIVE SAMPLES TO ACHIEVE 0.8MPA < UCS < 1.6 MPA AT 7 DAYS. UCS RESULTS ARE TO BE CONFIRMED DURING CONSTRUCTION - RESULTS OUTSIDE THIS RANGE ARE NOT ACCEPTABLE.
- 4. THE CEMENT USED IN THE UCS TESTING SHALL CONSIST OF A 65/35 CEMENT/SLAG BLEND. THE TESTING SHOULD BE IN ACCORDANCE WITH ASS101.4 2008 USING 115.5 X 105MM DIAMETER CYLINDER, COMPACTION AT 100% STANDARD OR 98% MODIFIED COMPACTION USING 7 DAY CURING AT 650C FOR 65/35 CEMENT/SLAG SLOW SETTING BINDER BLEND.

#### SPRAY SEALS

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- 1. SPRAY / SPREAD RATES NOMINATED ARE INDICATIVE ONLY AND PROVIDED FOR ESTIMATION PURPOSES. SPRAY SEAL DESIGNS, APPROPRIATE FOR THE CONDITIONS AT THE TIME OF PLACEMENT, SHALL BE UNDERTAKEN BY THE CONTRACT ADMINISTRATOR. THE CONTRACTOR IS REQUIRED TO PROVIDE AT LEAST 48 STANDARD WORKING HOURS' NOTICE OF THEIR PROPOSED SEAL DATES TO HELP FACILITATE THIS PROCESS.
- 2. BITUMEN EMULSION SEALS ARE TO BE SUPPLIED AND APPLIED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS
- 3. THE PMB BINDER TYPE SHALL BE \$0.75 UNLESS \$0.38 IS OTHERWISE APPROVED BY TMR.

- SIGNS AND LINEMARKING
- ALL LINE MARKING, GUIDE POSTS AND RAISED PAVEMENT MARKERS SHALL BE IN ACCORDANCE WITH TMR MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.
- ALL SIGN SUPPORTS ARE TO BE INSTALLED AS PER TMR STANDARD DRAWING NO.'S 1363, 1365 AND 1368.

#### SERVICES

- 1. LOCATION AND LEVEL OF ALL SERVICES MUST BE OBTAINED PRIOR TO CONSTRUCTION. ALL LEVELS SHALL BE CHECKED FOR CONFLICT WITH ANY SERVICES AND ANY IDENTIFIED CONFLICTS RESOLVED WITH THE CONTRACT ADMINISTRATOR. ALL OVERHEAD CLEARANCES TO BE CONFIRMED PRIOR TO PAVEMENT WORKS TO ENSURE ENERGEX MINIMUM CLEARANCE REQUIREMENTS ARE MET. CLEARANCE REQUIREMENTS TO BE IN ACCORDANCE WITH ENERGEX 'OVERHEAD DESIGN MANUAL SECTION 4 - CLEARANCES'.
- PROTECTION DETAILS TO BE PROVIDED BY RELEVANT SERVICE PROVIDERS IF REQUIRED.

## **ISSUED FOR CO**

	REA	DATE	AHE	DADMENT / REVISION DESORPTION	WVR Hs.	APPROVAL	TITLE	NAME	SCALES AT AT SIZE DRAWING	OLDONES.	115	CLIENT	
	A	30/01/201	4 1550	JED FOR CONSTRUCTION	019	AZ	ORAFTER	B. TAYLOR			SMEC		
	8	28/02/20	A NOT	ES AMENDED ATT LEVER DEALVIONENT	25	RS	DRAFTING CHECK	J. RAISON					
	0 10/62/2014 NOTE ADDED E 16/05/2014 NOTES AMENDED	E ADDED	2.1	AZ	DESIGNER	L SWANN		SMEC AUSTRALIA PTY LTD SMEA 47 065 4/5 148 LEVEL 1 1544 MELOORREL STREET SOUTH BRIESBARE QLD, 4101 PY(07 1020 6600, FAX 07 3020 6500	SMEC AUSTRALIA PTY LTD	a mar	narann		
		ES AMENDED	032	Re	DESIGN CHECK	S. RUNGE			LEVEL 1 154 MELOOLRINE STREET		Indidity		
					PROJECT MANAGER	A. ZANETTI			SOUTH BRISBANE QLD. 4101 PH 07 3029 6600 FAX 07 3029 6650		COLOBAL COUN		
							PROJECT DIRECTOR	R. STRINGFELLOW			SMEC PROJECT No 30031182	· · · · · · · · · · · · · · · · · · ·	
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NSTRUC	TIO	N			
	PROJECT TITLE	MARAN ROMA FL	OA REGIONAL	COUNCIL IN PROJECT	
noa		FAVENE	NT GENER	AL NUIES	
	SCALE NTS	CONSTRUCTION	N 30031	182-PD-0011	E





LVERT	LENGTH (L)	MINIMUM ROCK SIZE (d <sub>50</sub> )	DEPTH	VOLUME (m³)	
POZA	2.4 MIN.	200mm	300mm NOM.	0.7	
P02B	1.8 MIN.	200mm	300mm NOM.	0.5	
POLA	~2.4 MIN	200mm	300mm NOM.	-0.7	
P05A	1.8 MIN.	200mm	300mm NOM	0.5	
/P08B	2.4 MIN.	200mm	300mm NOM.	0.7	
/P08C	2.4 MIN.	200mm	300mm NOM.	0.7	
P080	2.4 MIN	200mm	300mm NOM	0.7	







301.74

WP01G

ROCK SCOUR PROTECTION DETAIL N.T.5

TAE	TABLE A - ROCK SCOUR PROTECTION										
CULVERT	LENGTH (L)	MINIMUM ROCK SIZE (d50)	DEPTH	VOLUME (m <sup>3</sup> )							
WPOIF	2.4 MIN	200mm	300mm NOM	0.7							
WP01G	2.4 MIN	200mm	300mm NOM.	0.7							
WP01H	2.4 MIN	200mm	300mm NOM	0,7							

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## **ISSUED FOR CONSTRUCTION**

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s <u>s</u>								155
¥ 3	EXTERNAL REFERENCE PLES REV	DATE AMENGMENT / REVISION DESCRIPTION	WVR ML AP	PROVAL	mu	NAME	SCALES AT AS SOT DRAWING	ICHAR AND
2	<u>^</u>	31/03/2014 ISSUED FOR CONSTRUCTION	029	47	DRAFTER	B. TAYLOR	-	SMEC
		16/05/2014 CULVERT WPOTH AMENDED	032	6	DRAFTING CHECK	J. RAISON		
2		The second s		-	DESIGNER	L. EVANS	SCALE 1900	SMEC AUSTRALIA PTY LTD (C) ABN 47 005 478 149
				-	DESIGN CHECK	A. ZANETTI	NAD \$45 (1993) (1993) (1994) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (1995) (	LEVEL 1 154 MELBOURNE STREET
				-	PROJECT MANAGER	A. ZANETTI	4	PH 07 3029 6600 FAX 07 3029 6650
¥				- 1	PROJECT DIRECTOR	R STRINGFELLOW		SMEC PROJECT No 30031182





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ITERNAL REFERENCE FILES	REV	DATE	AHENDHENT / REVISION DESCRIPTION	WVE No.	APPROVAL	TITLE	NKHE	SCALES AT AT SIZE DRAWING	DESCHER	distant in	0.0
	A	31/03/2014	ISSUED FOR CONSTRUCTION	929	AZ	DRAFTER	B. TAYLOR		SMEC AUSTRALIA PTY LTD	SMEC	
	8	16/05/2014	N REINFORCING NOTE AMENDED IN REINFORCING ARRANGEMENT REVISED	632	AZ AZ	DRAFTING CHECK	J. RAISON				
						DESIGNER	L. EVANS			SMEC AUSTRALIA PTY LTD	
						DESIGN CHECK	A ZANETTI	SCALE 139 HARD BLOOD HARD		(c) ABN 47 DES 475 149 LEVEL 1 154 MELBOURNE STREET	
						PROJECT MANAGER	A. ZANETTI			SOUTH BRISBANE QLD. 4101 PH 07. 3028 6600 FAX 07. 3029 6650	
						PROJECT DIRECTOR	R. STRINGFELLOW			SMEC PROJECT No 30031182	

2 9 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		A 30/01/2014 ISSUED FOR CONSTRUCTION 8 31/03/2014 BIRKETT LEVEE REALIGNMENT C 7/07/2014 LEVEE LEVELS AMENDED	019 036 - AZ DRAFTING CHECK J. RAISON OESKONER J. RAISON DESKON CHECK J. CARROLL PROJECT MANAGER A. ZANETTI		SMEC ALIA PTY LTD 65 475 148 UBOUNDE STREET HARE QLD 4101 FAX 07 3029 6650	T man
59 00 00128	<i>کر</i>				ISSUE	D FOR CO
10 80	5		위 월[월] CHAINAGE 0.000		16-	러 원 CHAINAGE 200.00
05	Ś	OFFSETS	500 304. 20 304.	OFFSETS	681 302.	500 302
110 120	Ş	EXISTING SURFACE	747 304	EXISTING SURFACE	912 302	962 304
138 140	5	Datum 302.00	05L 05L	DESIGN HEIGHT	2912	126.
	<pre>}</pre>	CONTROL LINE MC10 X = 677003.432 Y = 7063695.937 Z = 304.750	CHAINAGE 50.000 జై ఇక్	CONTROL LINE MC10 X = 677175.776 Y = 7063619.324 Z = 304.957	M IS	EVE
	- }	OFFSETS	-4, 436 -1.500 1.500			다AINAGE 250.00
	~ {	EXISTING SURFACE	304,235 304,222 304,278 304,278	OFFSETS	172 3025	500 3024
	{	DESIGN HEIGHT	304, 235 304, 969 304, 278		3025	531 304.5
$\supset$		CONTROL LINE MC10 X = 677051.997 Y = 7063684.043 Z = 304.969 Datum 302.00	CHAINAGE 100.000 <u>ع من المرابع</u> <u>المرابع</u>	CONTROL LINE MC10 X = 677184.249 Y = 7063570.047 Z = 304.961	203	961
	Ş	OFFSETS	-6.613 -1.500 5.872			CHAINAGE 300.00 분 문
	}	EXISTING SURFACE	203.663 203.664 203.664	OFFSETS	-11:921	-1.500
	Ş	DESIGN HEIGHT	303.663 304.941 304.941	EXISTING SURFACE	302.359	302.636 302.719
	ł	CONTROL LINE MC10 X = 677100.562 Y = 7063672.150 Z = 304.941 Datum 302.00	N3 26.4	Datum 300.00	302,359	304.964
	{	UFFSEIS	<u>କ୍ଟିମ୍ମ</u> ର CHAINAGE 150.000	CONTROL LINE MC10 X = 677192.722 Y = 7063520.771	¥ 114-1	EN
	ł	EXISTING SURFACE	11 303.273 0 303.161 303.162 6 303.226	-		17 - M21
	Ę.	DESIGN HEIGHT	303.273 204.940 304.940 303.2266	_		
	Ş	Y = 7063658.960 Z = 304.914 Datum 302.00	й й 18-4 18-4 18-4 18-4			









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	BH01 (Exist.)	677453	7063	248		-	-26.540003	148.781314		BH21	1	679099	.075	7062589.83	0 –	-	-	C C C C C C C C C C C C C C C C C C C
	BH02 (Exist.)	677688	7063	066		-	-26.541898	148.783590		BH22	2	679106	959	7062548.42	4 –	-	-	
	BH03 (Exist.)	677952	7062	372		-	-26.548128	148.786336		BH23	3	679115.	154	7062511.163	3 –	-	-	YXXXXXX
	BH04 (Exist.)	678205	7062	444		-	-26.547447	148.788865		TPB0	01	679427	.815	7062423.42	7 300.227	-	-	RHAN
	BH05 (Exist.)	679045	7062	2614		-	-26.545806	148.797270		TPB0	)2	679897	.103	7062391.824	4 313.198	-	-	
	BH06 (Exist.)	679052	7061	259		-	-26.558034	148.797531		TPB0	)3	680286	.923	7062331.850	0 318.613	-	-	
_	BH07 (Exist.)	679104	7060	)819		-	-26.561998	148.798114		TPB0	)4	679659	.915	7062294.93	3 299.444	-	-	
15(	BH08 (Exist.)	678877	7060	352		-	-26.566241	148.795902		TPB0	)5	680056	.375	7062233.91	1 313.861	-	-	BOREHOLE
14.0	BH09 (Exist.)	678829	7060	048		-	-26.568991	148.795463		TPB0	)6	679739	.057	7062195.238	8 302.401	-	-	TPB30
130	BH10 (Exist.)	678621	7059	939		-	-26.570001	148.793391		TPB0	)7	680240	.829	7062094.72	4 312.438	-	-	TPB31
	BH11 (Exist.)	679161	7059	642		-	-26.572613	148.798852		TPB0	)8	679823	.467	7062073.69	4 305.710	-	-	TPB32
12	BH12 (Exist.)	679145	7059	508		-	-26.573820	148.798709		TPB0	)9	679840	.540	7061655.350	0 301.807	-	-	ТРВ33
110	BH13 (Exist.)	680978	7059	736		-	-26.571533	148.817075		TPB1	10	678031	.116	7062797.33	3 300.586	-	-	TPB34
100	BH14	677931.047	706295	52.37	8	301.490	-	-		TPB1	11	678060	.447	7062993.219	9 301.193	-	-	TPB35
。	BH15	678613.170	706290	)9.38:	2	301.182	-	-		TPB1	12	677950	.583	7062930.87	3 301.331	-	-	TPB36
	BH16	679125.878	706201	16.65	7	299.783	-	-		TPB1	13	679251	.711	7060870.03	8 297.778	-	-	TPB37
8	BH17	679212.439	706135	9.34	3	299.011	-	-		TPB1	14	679259	.915	7061190.953	3 297.918	-	-	TPB42
70	BH18	677526.818	706335	51.668	8	302.420	-	-		TPB1	15	679417.	496	7062078.016	6 299.608	-	-	TPB43
o.	BH19	677560.166	706335	57.90	3	301.476	-	-		TPB1	16	678560	.800	7063094.08	0 301.700	-	-	TPB44
	BH20	677590.388	706336	50.720	6	301.207	-	-		TPB2	29	677940	.067	7062760.720	6 300.880	-	-	
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0.4	EXTERNAL REFERENCE FILES			REV	DATE	AMENDMENT / REVISI	ON DESCRIPTION		WVR No.	APPROVA	AL TITLE			20	SCALES AT A1 SIZE DRAWING		DESIGNER	
õ				B	30/01/2014	REISSUED FOR CONS	CTIONS B&D		008	AZ	DRAF		B. TAYLO	NK N				SMEL
				c	31/03/2014	BIRKETT LEVEE R	EALIGNMENT		030	AZ	DESIG		<u>1. KAISUI</u> 1. CPACII	ΥΥ Υ.Μ	20 0 SCALE 1:2000	20 40 60 80 100	SI	MEC AUSTRALIA PTY LTD
2											DESIG		4. I AW		50 0 1	00 200 200	IF	C ABN 47 065 475 149
9											PROJE	CT MANAGER	A. ZANEI	771	SCALE 1:7500		PH	SOUTH BRISBANE QLD. 4101
0											PROJE	CT DIRECTOR	R. STRIN	GFELLOW			SM	IEC PROJECT No 30031182

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	BOREHOLE SETOUT									
	EASTING	NORTHING	R.L.	LATITUDE	LONGITUDE					
	677934.061	7062789.307	300.965	-	-					
	677967.614	7062790.223	299.536	-	-					
	677946.255	7062912.751	301.302	-	-					
	677895.510	7062940.236	301.895	-	-					
	678052.110	7063041.470	301.160	-	-					
	678093.002	7062994.458	301.256	-	-					
	678070.194	7063016.144	301.220	-	-					
	678049.585	7062997.314	301.164	-	-					
	678092.404	7062356.942	303.080	-	-					
	678131.573	7062359.779	302.999	-	-					
	678170.320	7062362.000	302.824	-	-					
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DCP EASTING NORTHING	TPOL	677171172	7063551 745	BH20	617590 383	1063360 726			24, 1 -		Contraction of the	
0CP001 677054.850 7063662.440	TPOS	677220 515	7043350.080		211270 202					1 75 8 8 8	- 44 ·	100
ICP102 677112.701 7063667.633	TPOS	677409465	2063285 983	In the second		and the second state	and on the state of	-	- 10 The S		15 100	
ICPE03 677158.812 7063650.015	TP08	677725.058	2063238566	and the second s		125		100-317	1.1.1	057 H		
CP304 677176.773 7863604.675				a car	100 miles (20)	Section Soft	Caller -	111111		2 3 1 2 B	S-46-5	
CP105 677171172 7063554.745	-			4.00	×.	All a state	1200					
ICP206 677193.719 7063506.122	TEST	PIT LOCAT	ONS	the states and	100	the second	dia man		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		Constant of the	-
CP007 677202.192 7963456.845	TEST PIT	EASTING	NORTHING			- CLAR	1 1 1	1	1. A. J	Wall real feet	1 Strength	63 M
CP108 677210.665 7863407.568	TPx1	677483.893	7063387.895	and the second second	1	Lini Day	No. No. 1	in the	1.0	- 1 · · · ·		23
ICP009 677220.510 7063350.080	TPx2	677557.020	7063047.447			Set 2		- Horsen	6.63			11-1
3CP010 677269.519 7063331.719				1 Start	and the		20		TRACE	and the second	The fer	
DCP011 677317.934 7063319.774	Distance of the	C PRIME N	1. PARSE			100		X	增加		at the second	
XP012 671367.050 7063310.418	1214 AL.A	TANID CORN		all a	C. Strat. B		1		1 Statement		1.524	
CF013 677409.460 1063285.983	1940	24 550	IN SUBT	A CONTRACTOR	Part U. Mel	Sal	Contraction of the second	40		A STORES	COLORADO L	
CP014 677465.284 7063291.706			AL ST			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	The second		NY 15	Carles.	C. States and	3
XP015 677495.178 7863308.403	Mar Bar / Sam		12	A. Carlos	1/2011/10/10	a dance a	I-Fingle	the state	and the second s	102	58	y.
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CP019 677682.519 7063279.251	Mar a Mar			and States	1079.079192		2.6	in the	1427		1 m 1 m 1	
0020 671725.098 7063278.565		1	Water of the	- + X S	5 \$ S		# 8/5 A. C.	Dec TO LAW	Section of	A.	BPI19	1
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	TPB34 TPB35 TPB34 TPB35 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB11 TPB35 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15 TPB15	B SME
	TPOP DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP025 DCP0	TEST BORROW PIT LOCATIONS           TEST PIT         EASTING         NORTHING           TPB10         678031.116         7062797.333           TPB11         678060.447         7062993.219           TPB12         677950.583         7062930.873           TPB29         67794.0.067         7062789.307           TPB30         677934.061         7062789.307           TPB31         677967.614         7062790.223           TPB32         67794.6.255         7062912.751           TPB33         677895.510         7062940.236
		TEST PIT LOCATIONS           TEST PIT LOCATIONS           TEST PIT CASTING           TP09         6777796.270         7063175.14.9           TP10         677879.331         7062988.084           TP11         678076.589         7062995.051           TP12         678269.393         7062995.051           TP13         678268.228         7062998.0927
$\supset$	BOREHOLE SETOUT         TPB30           B0REHOLE         EASTING         NORTHING           BH14         677931.064         7062952.375           BH15         678613.170         7062909.382	TP14         678742.627         7062959.463           TP15         678861.513         7062955.674
s   134   186   156	0527 0007 0557 0007 0551 0001 0551 0001 0551 0007 0007	PO47 DCP048 DCP050 05E2 0072 0572
96   196   12	DCP SETOUT         DCP SETOUT         DCP SETOUT         DCP SETOUT         DCP SETOUT           DCP         EASTING         NORTHING         DCP         EASTING         DCP         EASTING         NORTHING         DCP         EASTING	
am DN 00000441	DCP025         677796.256         7063175.145         DCP032         678031.457         7062935.113         DCP039         678366.169         7062897.855         DCP046         678711.413         7062955.450           DCP026         677816.747         7063126.799         DCP033         678075.820         7062912.129         DCP040         678415.459         7062906.102         DCP047         678361.216         7062955.450         DCP048         678811.162         7062955.450         DCP048         678811.162         7062958.645         DCP048         678811.162         7062958.645         DCP049         678860.792         7062952.733         DCP049         678905.927         DCP049         678905.927         DCP049         678905.927         DCP049         678860.792         7062952.733         DCP049         678909.595         DCP049         678909.595         7062941.8171         SSUED FOR CONSTR	NOTES 1. FOR NOTES AND LEGEND REFER TO DRAWING 30031182-DI-0011.
AT 15 19 10	EXTENSIL REFERENCE TIES     HV     BATE     Alternative scope rank     VVP RL     MME     Iffile     MME       A     20/31/2014     SSSED FOR CONSTRUCTION     608     42       B     36/31/2014     RESIDE FOR CONSTRUCTION     608     42       C     B/48/2014     RESIDE FOR CONSTRUCTION     608     42       D     10/48/2014     GOLDERS LEVEE REALIGNMENT     608     42       PROJECT MANAGER     A     A       PROJECT MANAGER     A     CRAFTING CHECK     J. RAISON       D1     10/48/2014     GOLDERS LEVEE REALIGNMENT     012       D2     10/48/2014     1000000000000000000000000000000000000	AND BORROW PIT LOCATIONS SHEET 2 SCALE PASE ON THE STREET OF STR

TEST BOI	ROW PIT LO	OCATIONS
TEST PIT	EASTING	NORTHING
TPB10	678031.116	7062797.333
TPB11	678060.447	7062993.219
TPB12	677950.583	7062930.873
TPB29	677940.067	7062760.726
TP830	677934 061	7062789 307
TPB31	677967.614	7062790.223
TP832	677946.255	7062912.751
TPB33	677895.510	7062940.236
TP834	678052.110	7063041.470
TP835	678093.002	7062994.458
TPB36	678070.194	7063016.14.4
TPB37	678049.585	7052997.314

TEST PIT	LOCATIONS
1 - 0 1 1 1	LOOMIONO

TEST PIT	EASTING	NORTHING
TP09	677796.270	7063175.149
TP10	677879.331	7062988.084
TP11	678076.589	7062915.051
TP12	678268.228	7062884.459
TP13	678563.393	7062930.927
TP14	678742.627	7062959.463
TP15	678861.513	7062955.674







TEST PIT LOCATIONS				DCP SETOU	T		DCP SETOU	Г		DCP SETOU	DCP SETOL		
TEST PIT	EASTING	NORTHING	DCP	EASTING	NORTHING	DCP	EASTING	NORTHING	OCP	EASTING	NORTHING	DCP	EASTING
TP16	679049.636	7062889.809	DCP051	678957.026	7062926.197	DCP058	679099.152	7062654.733	DCP069	679130,459	7062124.107	DCP076	679205.097
TP17	679101.596	7062708.730	DCP052	679003.193	7062906.932	DCP062*	679108.021	7062460.311	DCP070	679141.134	7062075.207	DCP077	679215.813
TP19	679083.168	7062342.102	DCP053	679048.892	7062886.784	DCP063*	679082 386	7062417.599	DCP071	679151.837	7062026.414	DCP078	679230.679
TP20	679132.694	7062127,988	DCP054	679084.197	7062852.182	DCP064*	679077.370	7062368.395	DCP072	679162.492	7061977.558	DCP079	679246.811
TP21	679175.344	7061932.593	DCP055	679098.276	7062804.700	DCP065	679087.810	7062319.485	DCP073	679173.187	7061928.675	DCP080	679261.249
TP22	679218.008	7061737.183	DCP056	679098.500	7062754.757	DCP066	679098.544	7062270.647	DCP074	679183,777	7061879.863	. NOT YET UND	ERTAKEN DUE TO
			DCP057	679098.706	7062704.759	DCP068*	679119.851	7062172.942	DCP075	679194.708	7061830.832		

NME

DRAFTER

**DESIGNER** 

DESIGN CHECK

RAFTING CHECK

8. TAYLOR

J. RAISON

A. LAW

PROJECT MANAGER A. ZANETTI

PROJECT DIRECTOR R. STRINGFELLOW

O CRACIUN

SCALES AT AS SIZE DRAWING

SCALE 11000

WVR No.

AZ

AZ

A

# **ISSUED FOR CONSTRUCTION**

	SMEC	
30 40 50	SMEC AUSTRALIA PTY LTD Ann 47 005 475 148 LTYL1 11 55 MELDOWRE BTIRET SOUTH WIDDEWE DIA 101 PH 67 3029 6500 FAX 07 3029 6550 SMEC PROJECT No 30031182	1 mara

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EXTERNAL REFERENCE PLUS

DATE

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AMENDMENT / REVISION DESCRIPTIO

28/01/2014 ISSUED FOR CONSTRUCTION 30/01/2014 REISSUED FOR SECTIONS B&D 31/03/2014 BIRKETT LEVEE REALKAMENT

16/85/2014 MCPHE ST REALIGNMENT





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FERENCE FILES	REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	NAME	SCALES AT A1 SIZE DRAWING	DESIGNER	
	A	20/01/2014	ISSUED FOR CONSTRUCTION	008	AZ	DRAFTER	B. TAYLOR			SMEC
	в	31/03/2014	BIRKETT LEVEE REALIGNMENT	030	AZ	DRAFTING CHECK	J. RAISON			
						DESIGNER	O. CRACIUN			SMEC AUSTRALIA PTY LTD
						DESIGN CHECK	A. LAW			C ABN 47 065 475 149 LEVEL 1 154 MELBOURNE STREET
						PROJECT MANAGER	A. ZANETTI			SOUTH BRISBANE QLD. 4101 PH 07 3029 6600 FAX 07 3029 6650
						PROJECT DIRECTOR	R. STRINGFELLOW			SMEC PROJECT No 30031182
104004001 CIDIDIICIAL CEIDADAMOD CE 4444 [0] 1					AF 14 - 0	45 37 50		$\wedge$		



TIONS						
	NORTHING					
	7061544.963					
	7061347.858					
	7061153.992					
	7060962.135					
	7060765.397					
	7060572.685					

LOCATIONS						
	NORTHING					
	7060870.038					
	7061190.953					
		5				

TIONS						
	NORTHING					
	7060810.796					
	7060809.143					
	7060807.014					

DCP SETOUT							
DCP	EASTING	NORTHING					
DCP080	679261.249	7061590.411					
DCP081	679268.028	7061540.952					
DCP082	679266.909	7061491.329					
DCP083	679257.685	7061442.094					
DCP084	679247.362	7061392.881					
DCP085	679237.306	7061343.938					
DCP086	679227.101	7061294.896					
DCP087	679216.327	7061246.059					
DCP088	679202.473	7061198.126					
DCP089	679188.381	7061150.174					
DCP090	679174.141	7061102.163					
DCP091	679160.027	7061054.270					
DCP092	679145.994	7061006.255					
DCP093	679131.890	7060958.338					
DCP094	679117.847	7060910.445					
DCP095	679108.118	7060861.135					
DCP096	679106.279	7060811.424					
DCP097	679106.609	7060761.424					
DCP098	679107.471	7060711.432					
DCP099	679114.918	7060662.029					
DCP100	679130.399	7060614.571					
DCP101	679153.541	7060570.286					
DCP102	679183.671	7060530.479					
DCP103	679219.951	7060496.147					
DCP104	679261.133	7060467.899					

PROJECT TITLE MARANOA REGIONAL COUNCIL ROMA FLOOD MITIGATION PROJECT								
GEOTECHNICAL INVESTIGATION								
AND BORROW PIT LOCATIONS								
	SHI	EET 4						
SCALE	PHASE	PROJECT / DRAWING No.	REVISION					
1:1000	CONSTRUCTION	30031182-GE-1004	В					
-	project title scale 1:1000	PROJECT TITLE MARANDA RE ROMA FLOOD M GEOTECHNICAL AND BORROW SHI SCALE PHASE 1:1000 CONSTRUCTION	PROJECT TITLE MARANDA REGIONAL COUNCIL ROMA FLOOD MITIGATION PROJECT GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 4 SCALE PHASE PROJECT / DRAWING NO. 30031182-GE-1004					

BORROW TEST PIT LOCATIONS							
TEST PIT	EASTING	NORTHING					
TPB01	679427.815	7062423.427					
TPB02	679897.103	7062391.824					
TPB03	680286.923	7062331.850					
TPB04	679659.915	7062294.933					
TPB05	680056.375	7062233.911					
TPB06	679739.057	7062195.238					
TPB07	680240.829	7062094.724					
TPB08	679823.467	7062073.694					
TPB09	679840.540	7061655.350					
TPB15	679417.4962	7062078.016					
TPB17	679700.608	7062784.210					
TPB18	679698.333	7062762.661					
TPB19	679696.761	7062735.526					
TPB20	679734.936	7062785.471					
TPB21	679735.403	7062760.806					
TPB22	679728.831	7062726.024					
TPB23	679767.657	7062789.857					
TPB24	679768.107	7062751.403					
TPB25	679771.749	7062714.629					
TPB26	679798.232	7062789.909					
TPB27	679811.761	7062756.517					
TPB28	679819.055	7062730.728					

TPB23<sup>TPB21</sup>TPB22 TPB20

TPB17 TPB17



EXTERNAL REFERENCE FILES	REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	NAME	SCALES AT A1 SIZE DRAWING	DESIGNER		CLIENT
	A	20/01/2014	ISSUED FOR CONSTRUCTION	008	AZ	DRAFTER	B. TAYLOR			SMEC	- 23
	В	30/01/2014	REISSUED FOR SECTIONS B&D	019	AZ	DRAFTING CHECK	J. RAISON				
	Ľ	31/03/2014	BIRKETT LEVEE REALIGNMENT	030	AZ	DESIGNER	O. CRACIUN	20 0 20 40 60 80 100		SMEC AUSTRALIA PTY LTD	
						DESIGN CHECK	A. LAW	SCALE 1:2000		C ABN 47 065 475 149 LEVEL 1 154 MELBOURNE STREET	
						PROJECT MANAGER	A. ZANETTI			SOUTH BRISBANE QLD. 4101 PH 07 3029 6600 FAX 07 3029 6650	L.P
						PROJECT DIRECTOR	R. STRINGFELLOW			SMEC PROJECT No 30031182	





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 REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR NR.	APPROVAL	THUE	NAME	SCALES AT AN SIZE DRAMING	DESIGNER
1	30/01/201	ISSUED FOR CONSTRUCTION	906	AZ	DRAFTER	B. TAYLOR		S S
В	28/03/205	DESIGN AMENDED	026	AZ	ORAFTING CHECK	J. RAISON	2011 A 22	A65 (00)
C TOPUS/2014 LEVER A	LEVEL ALIGNMENT APERDED AND INCO REPORED	932	m	DESIGNER	J. RAISON	25 ut 3 19	SMEC AUSTRALI	
					DESIGN CHECK	J. CARROLL	SCALE (234	LEVEL 1 154 MELBOU
					PROJECT MANAGER	A. ZANETTI		PH 07 3028 6600 FAX
					12020101000000000	a arounarculati		00000 0000 000000



anoa	TRI SAM	EATMENT FOR	SERVICES UNDER LE	VEE
	PROJECT TITL	MARANO	A REGIONAL COUNCIL D MITIGATION PROJECT	
NSTRU	стю	)N	NOTES 1. FOR NOTES AND LEGEND RI DRAWING 30031182-DI-001	EFER TO
@ CH 3720				
ETE CUT-OFF WALL ALLY IN LEVEE. TAIL ON DRG. TC-01	LOCATED			
BOTTOM OF F	ERVICE TR	N		

OB



Levee 14 RP855185



SURVEY STATIONS									
Station	Туре	Easting	Northing	Height					
PSMK	71141	677872.780	7061307.661	299.710					
PSMK	38332	677647.060	7062476.827	301.001					
PSMK	84725	677608.381	7062839.287						
PSMK	85088	677576.185	7063005.568	303.051					
PSMK	38339	677508.729	7063198.878	303.425					
PSMK	MR3SK	677515.460	7063326.300	302.995					
PSMK	34690	677432.408	7063603.307	302.310					
PSMK	639981	677318.603	7064184.564	304.921					
PSMK	85030	675552.242	7061880.159						

DESIGNER		CLIENT
	ISSUE	DF

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CONTROL LINE MCC1

3712.329 677513.505 7063253.673 304.881

Northing

7063076.128

7063441.677

Level

7063032.384 303.327 349\*08'40.56"

303.392

Bearing

302.829 349'04'40.96"

Easting

677555.923

3531.552 677547.534

3903.801 677477.226

Chainage

3487.011

Pt

START

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IP

END

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REV	DATE	AMENDMENT / REVISION DESCRIPTION	WVR No.	APPROVAL	TITLE	NAME	SCALES AT A1 SIZE DRAWING	DESIGNER	
A	30/01/2014	ISSUED FOR CONSTRUCTION	019	AZ	DRAFTER	B. TAYLOR			SMEC
В	28/02/2014	TMR COMMENTS	025	RS	DRAFTING CHECK	J. RAISON			
Ĺ	31/03/2014	BIRKETT LEVEE REALIGNMENT	030	AZ	DESIGNER	J. RAISON	0 10 20 30 40m		SMEC AUSTRALIA PTY LTD
					DESIGN CHECK	J. CARROLL			C ABN 47 065 475 149 I EVEL 1 154 MELBOURNE STREET
					PROJECT MANAGER				SOUTH BRISBANE QLD. 4101
						Λ ΖΑΝΕΤΤΙ			CMEC DEC LECT No 20021182

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EXTERNAL REFERENCE FILES







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					<b>B</b> BS	MEC
FLICT WITH SI is locations shall b int authorities prior hencing. During con id out within 3.0m coting the relevant of	ERVICES e confirmed to construc struction, no of any serv authority.	on sit ction wo o work vice with	e with the ork shall be nout first			
The Regional I these Drawings Engineer has e Drawings and	Director does and/or Sp executed sou Specification	s not c becificat und enq is, and	iccept any ions. It is jineering j that all s	expected that udgement when ite conditions of	for the accurd the Certifying preparing the and Transport	acy of e and
Main Roads re	quirements	have be	een taken	into account.		
Road: Carnary	on Highway	AND	MAIN	Parish of Ror		
T.Dist: 3.499	km – 3.914	l km		Aux Dwg No's		
	No Cial	12.1	Downs	LG: Maranoa Reg	jional Council	
Q	ueensla	ands	outh West	Revision A Mi	crofilmed	
CUERT G	overnm	ent	Region	TMR Dwg No.		
	EN	IGINEERIN	IG CERTIFIC	ATION (RPEQ)		DATE
DRAINAGE	A. ZANETTI			A. ZANETTI	12112 31/	/03/14
pavement 300 pavement mm	300 Ne	ew pave	ment	2 Coat spray spray seal des 200mm Type 2 575mm Select CBR 35% - Bidim A29 or equivalent geo - Subgrade	seal (refer to sign for details 2.1 Granular t Fill approved fabric	typical a)
ISTING PAVEME	<u>n new f</u> ENT CH3	<u>2AVEN</u> 687.0	<u>IENT AR</u> ) <u>11</u>	<u>1D</u>		
Existing pavemen	t Omm past	paveme	nt			
		Existing overlay modify	Pavemen (Subtype to a dept	t — 140mm m 3.2) and then h of 200mm	in Granular insitu	
JOINT BETWEE	N NEW	PAVEN	<u>MENT A</u>	<u>ND</u>		
ISTING PAVEMI	<u>LINI UHJ</u>	094.	30	NOTES For notes drawing 30	and legend re 031182-DI-00	fer to D11.
<b>NSTRU</b>	<b>IOIT</b>	N				
anoa	PROJECT TITLE	R( <b>AN[</b>	MARANOA DMA FLOO WOF D LONG	REGIONAL COL MITIGATION P RKING PLAN TUDINAL S	INCIL ROJECT I ECTION	
I COUNCIL	scale 1:500	phase CONST	RUCTION	PROJECT / DRAWIN 30031182	<sup>™™</sup> -WP-1102	

C = 677534.650 C = 7063143.352	MCC1		CONTROL LINE MCC1		<del>~</del>	
L = 303.491			$\begin{array}{rcl} X &=& 677519.591 \\ - & Y &=& 7063221.922 \end{array}$	JL ML	R CC CER CC	AR
	99 1		Z = 304.393	<u>6.7%</u> in3		2%
DESIGN HEIGHT	<u>303.4</u> 303.4		Datum 302.00	2 4 2	14 08 83 33 14 08 88 83 14 08 19 10 10 10 10 10 10 10 10 10 10 10 10 10	75
XISTING SURFACE	03.352 503.465 503.465		DESIGN HEIGHT	4         303.73           4         303.31           4         303.31           4         303.31	5 304.28 8 304.39 8 304.39 2 304.25 2 304.25 0 303.14	5 303.67
OFFSETS	3.500		EXISTING SURFACE	5 303.73 303.39 303.42	303.59 303.59 303.59 303.59 303.49 303.47 303.39	303.67
	CHAINAGE 3600.000		OFFSETS	-14.98 -6.039	-3.500 -000 -500 -500 -841	6.467
					CHAINIAGE 3680 000	-
= 677538.415 - 7063123 710	DML 14L		CONTROL LINE MCC1			
= null -			$\begin{array}{cccc} - & Y &= & 077323.333 \\ Y &= & 7063202.280 \\ 7 &= & 303.956 \\ \end{array}$	IDL IDL		IAR
atum 300.00			7 Datum 302.00	<u>3.0% 1 in -5</u>		4%
ESIGN HEIGHT	302.941		DESIGN HEIGHT	03.500 03.317 03.317	03.851 03.956 03.956 03.851 03.851 03.851 03.851 03.851 03.851 03.824 03.804 03.804	03.553
XISTING SURFACE	2 302.953		EXISTING SURFACE	03.500 3 03.306 3 03.329 3	03.449         3           03.594         3           03.594         3           03.594         3           03.594         3           03.594         3           03.594         3           03.491         3           03.491         3           03.491         3           03.445         3           03.446         3           03.449         3           03.449         3           03.449         3           03.449         3           03.449         3           03.449         3	03.553 3
OFFSETS	-14.202 -11.090		OFFSETS	15.026 3 15.026 3 8.882 3 6.500 3	3.500 3 0000 3 00000 3 0000 3 00000 3 00000 3 00000 3 00000 3 00000000	6.494 3
ONTROL LINE MCC1	CHAINAGE 3580.000				CHAINACE 3660 000	-
= 677545.944 = 7063084.425	_ ₹		V - 677527 120		CHAINAGE 5000.000	
= null	≤ ⊃ 1 <u>in −10</u>		$\begin{array}{c} X = 077327.120 \\ Y = 7063182.637 \\ - 7 = 303.633 \end{array}$		CER MCC1 DWL DWL	IAR
atum 302.00	2 5		Z = 505.855	<u>3.5% 1 in -1</u>	0 -3.0% -3.0% -3.0% -4.0% 1 in <u>-10</u>	5.0%
ESIGN HEIGHT	4 303.01		DESIGN HEIGHT	03.500 03.261 03.261	03.528 03.633 03.633 03.633 03.498 03.438 03.439 03.265 03.265	03.465
XISTING SURFACE	303.13		EXISTING SURFACE	3.500 3 3.222 3 3.301 3	3.399 3 3.477 3 3.477 3 3.477 3 3.477 3 3.477 3 3.476 3 3.394 3 5.296 3 5.296 3 5.296 3 5.296 3	3.465 3
FFSETS	-9.854 -8.331		OFFSETS	.070 30 278 30 500 30	500 30 500 30 50 30 50 30 51 30 51 30 51 30	363 30
•	CHAINAGE 3540.000				- 3.50 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	16.6
ONTROL LINE MCC1		L.	CONTROL LINE MCC1		CHAINAGE 3640.000	
= 677555.923 = 7063032.384			$\begin{array}{rcl} X &=& 677530.885 \\ Y &=& 7063162.995 \end{array}$	Ĭ	VCC1	
= null			Z = 303.520	يع 	3.5%3.5%	
atum 300.00		2 2	Datum 302.00	<u> </u>	2 2 0 0 2 2	
ESIGN HEIGHT		303.12	DESIGN HEIGHT	303.24	303.52 303.52 303.52	
XISTING SURFACE		303.071	EXISTING SURFACE	303.269	303.514	
)FFSETS		7.732	OFFSETS	-6.973	-3.500	
	CHAINAGE 3487.011				CHAINAGE 3620.000	
ICE FILES	REV DATE AMENDMENT / REVISION DESCRIPTION	WVR No. APPROVAL TITLE	NAME SCALES AT A1 SIZE DI	RAWING	ISSU	
	A 30/01/2014 ISSUED FOR CONSTRUCTION B 28/02/2014 TMR COMMENTS	019 AZ DRAFTER 025 RS DRAFTING	B. TAYLOR CHECK J. RAISON		<b>SMEC</b>	
	C 31/03/2014 BIRKETT LEVEE REALIGNMENT D 10/04/2014 BAR TREATMENT ADDED	- AZ DESIGNER	J. RAISON	0 2 4 6 8m	SMEC AUSTRALIA PTY LTD © ABN 47 065 475 149	1 m
		DESIGN CH	ECK J. CARROLL		LEVEL 1 154 MELBOURNE STREET SOUTH BRISBANE QLD. 4101	REG

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The Regional these Drawin Engineer has Drawings and Main Roads	Director does not o gs and/or Specificat executed sound eng Specifications, and requirements have be	accept any re ions. It is ex gineering judo that all site een taken int	esponsibility for pected that th gement when p conditions and o account.	the ac ne Certi preparine d Trans	ccuracy fying g the port and	of			
TRANSPORT AND MAIN ROADS USE ONLY									
Road: Carnarvon HighwayParish of: RomaT.Dist: 3.499 km - 3.914 kmAux Dwg No's.									
Downer LG: Maranoa Regional Council									
Queensland South West Revision A Microfilmed									
ARE C	Government	Region TM	TMR Dwg No.						
	FNGINFERIN	NG CERTIFICATIO	N (RPFQ)			_			
FNG. ARFA	NAME	SIGN		NO.	DATE	-			
DRAINAGE	A. ZANETTI	A. Z	A. ZANETTI 12112 31/03/14			14			
NSTRU	CTION	NC 1.	TES For notes and drawing 3003	d legen i1182-[	d refer DI-0011.	to			
100	PROJECT TITLE	MARANOA R OMA FLOOD M	EGIONAL COUNC IITIGATION PRO	IL IJECT					
noa		ANNOTATED CROSS SECTIONS SHEET 1							
	SCALE PHASE 1:200 CONST	RUCTION	PROJECT / DRAWING N 30031182-X	(S-110	1				



PROJECT DIRECTOR A. ZANETTI

802 802 667 697 301.151 363 697 667 303.8 303. 303. 303. 301 .815 .665 720 790 903 903 180 363 302. 302. 302. 301. 301 500 050 468 0.000 5.500 1.500 ö CHAINAGE 3840.000 MCC ESR ESR E E -3.0% -3.0% IAR Я 7.0% 288 288 153 183 183 153 908 701 304. 304. 304. 304 20 302.838 302.912 302.931 302.853 600 842 308 303. 20 02 -4.500 -3.500 11.857 16.272 000 3.500 4.500 ö CHAINAGE 3820.000 ESL CEL MCC1 CER ESR -3.0% -3.0% IAR Ш 7.1% .656 572 .226 521 551 304. 304 02. 304 304 302.209 007 414 044 932 572 303. 503. 303 8 4.500 1.385 258 0.000 3.500 4.500

MCC1

-3.0% -3.0%

CER ESR

IAR

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4.8%

IDR

CHAINAGE 3800.000

CONTROL LINE MCC1 X = 677500.683 Y = 7063320.118 Z = 304.906	IAR	1 in -7		-3.0%	-3.0%	CER ESR	1 in		
Datum 300.00				_	_		_		
design height	303.219	304.733	304.771 304.801	304.906	304.906 204.901	304.771	304.762	304.279	
EXISTING SURFACE	303.219	303.081	303.196 303.246	303.316	303.316	303.161	303.141	302.883	
OFFSETS	-15.409	-5.375	-4.500 -3.500	0.000	0.000	4.500	4.746	8.162	

## CHAINAGE 3780.000 ISSUED FOR CO



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1 1 344344001 C 101 DUICE 00 CUE 344

0 A1

The Regional	Director does not a	ccept any	responsibility for	the ac	ccuracy	of			
Engineer has executed sound engineering judgement when preparing the Drawings and Specifications, and that all site conditions and Transport and Main Roads requirements have been taken into account.									
TRANSPORT AND MAIN ROADS USE ONLY									
Road: Carnarvon HighwayParish of: RomaT.Dist: 3.499 km - 3.914 kmAux Dwg No's.									
. State	Star Carlie	Downs	LG: Maranoa Regio	nal Coun	cil				
下部的(	<b>Jueensland</b> so	outh West	Revision A Micr	ofilmed_					
CREE C	Government	Region	TMR Dwg No.						
	ENGINEERIN	G CERTIFIC	ATION (RPEQ)						
ENG. AREA	NAME	:	SIGNATURE	NO.	DATE				
DRAINAGE	A. ZANETTI		A. ZANETTI	12112	31/03/	/14			
			NOTES I. For notes an drawing 3003	d legen 31182-[	d refer DI-0011	to			
NSTRU	CTION								
	PROJECT TITLE	MARANOA MA FLOO	REGIONAL COUNT D MITIGATION PRO	CIL DJECT					
anoa	ANN	OTATEL	) CROSS SEO SHEET 2	TION	S				
	SCALE PHASE 1:200 CONSTI	RUCTION	PROJECT / DRAWING N 30031182->	(S-110	2				



## **ISSUED FOR CO**



SCALES AT A1 SIZE DRAWING

0 2 4 6 8m

T 1 D

The Regional Director does not accept any responsibility for the accuracy of these Drawings and/or Specifications. It is expected that the Certifying Engineer has executed sound engineering judgement when preparing the Drawings and Specifications, and that all site conditions and Transport and Main Roads requirements have been taken into account.									
Т	RANSPORT AND	MAIN	ROADS USE	ONLY					
Road: Carna T.Dist: 3.499	Road: Carnarvon Highway Parish of: Roma T.Dist: 3.499 km — 3.914 km Aux Dwg No's.								
Downs LG: Maranoa Regional Council									
	Queenslands	outh West	Revision A Micr	ofilmed_					
BERNA (	Government Region								
	ENGINEERIN	IG CERTIFIC	ATION (RPEQ)						
ENG. AREA	NAME		SIGNATURE	NO.	DATE				
DRAINAGE	A. ZANETTI		A. ZANEIII	12112	31/03/14				
NSTRU	NOTES 1. For notes and legend refer to drawing 30031182-DI-0011. ISTRUCTION								
	project title R( ANN	MARANOA DMA FLOO OTATEI	A REGIONAL COUNT D MITIGATION PRO	IL UECT	s				
COUNCIL			SHEET 3		-				
	scale phase 1:200 CONST	RUCTION	PROJECT / DRAWING N 30031182->	(S-110	3 D				



**B** 



			CONT	ROL LINE	MCP12		
ľ	Pt	Chainage	Easting	Northing	Level	Bearing	Radius
	START	0	677499.282	7063327.379	304.827	79°04'40.9"	
X	END	34.418	677533.077	7063333.900	302.837	79°04'40.9"	

CONTROL LINE	E MCP2
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Chainage	Easting	Northing	Level	Bearing	Radius
0	677501.843	7063313.742	305.209	259°03'54.72"	
14.738	677487.373	7063310.946	304.513	259°03'54.72"	
21.029	677481.062	7063309.727	303.933		25
27.32	677474.945	7063311.703	303.435	287°54'10.43"	
35.927	677466.755	7063314.349	303.163	287°54'10.43"	
	Chainage 0 14.738 21.029 27.32 35.927	Chainage         Easting           0         677501.843           14.738         677487.373           21.029         677481.062           27.32         67747.945           35.927         677466.755	Chainage         Easting         Northing           0         677501.843         7063313.742           14.738         677487.373         7063310.946           21.029         677481.062         7063309.727           27.32         677474.945         7063311.703           35.927         67746.755         7063314.349	Chainage         Easting         Northing         Level           0         677501.843         7063313.742         305.209           14.738         677487.373         7063310.946         304.513           21.029         677481.062         7063309.727         303.933           27.32         677474.945         7063311.703         303.435           35.927         677466.755         7063314.349         303.163	Chainage         Easting         Northing         Level         Bearing           0         677501.843         7063313.742         305.209         25903'54.72"           14.738         677487.373         7063310.946         304.513         25903'54.72"           21.029         677481.062         7063309.727         303.933            27.32         677474.945         7063311.703         303.435         28754'10.43"           35.927         677466.755         7063314.349         303.163         28754'10.43"

304.984

304.889

304.776

304.661

-7.5

-7.5

-7.5

-7.5

REV DATE AMENDMENT / REVISION DESCRIPTION

 A
 30/01/2014
 ISSUED FOR CONSTRUCTION

 B
 28/02/2014
 TMR COMMENTS

 C
 31/03/2014
 BIRKETT LEVEE REALIGNMENT

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11.781

677495.37

#### CONTROL LINE MCP3

Pt	Chainage	Easting	Northing	Level	Bearing	Radius
START	0	677503.434	7063305.865	305.255	79°04'40.96"	
TC	5.811	677509.139	7063306.966	305.076	79°04'40.96"	
IP	10.899	677514.14	7063307.931	304.828		100
CT	15.988	677519.213	7063308.383	304.459	84°54'32.47"	
IP	32.263	677535.424	7063309.827	303.24		
END	37.314	677540.447	7063310.35	303.156	84°03'58.41"	

NAME

PROJECT MANAGER A. ZANETTI

PROJECT DIRECTOR A. ZANETTI

B. TAYLOR

J. RAISON

J. RAISON

J. CARROLL

#### SETOUT DETAILS MK31

Chainage	Easting	Northing	Level	Radius
0	677516.103	7063306.244	304.679	-7.5
2	677514.174	7063305.74	304.810	-7.5
4	677512.446	7063304.745	304.921	-7.5
6	677511.041	7063303.33	305.013	-7.5
8	677510.058	7063301.594	305.083	-7.5
10	677509.568	7063299.661	305.128	-7.5
12	677509.604	7063297.668	305.147	-7.5
12.293	677509.654	7063297.379	305.148	-7.5

#### SETOUT DETAILS MK21 SETOUT DETAILS MK22 SETOUT DETAILS MK32 Radius Chainage Easting Northing Radius Level Level 305.128 -7.5 677489.657 7063313.934 304.666 -7.5 0 677491.547 7063314.569 304.785 -7.5 305.107 -7.5 2 -7.5 7063315.68 304.878 -7.5 305.059 4 677493.204

677494.508 7063317.188 304.945

677495.726 7063320.948 305.003

677495.598 7063322.721

7063318.987

304.987

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WVR No. APPROVA

019 025 030

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-7.5

AZ RS AZ

DRAFTER

DESIGNER

DESIGN CHECK

DRAFTING CHECK

Chainage	Easting	Northing	Level	Radius
0	677505.626	7063318.255	305.050	-7.5
2	677506.261	7063316.364	305.050	-7.5
4	677507.371	7063314.708	305.022	-7.5
6	677508.879	7063313.403	304.965	-7.5
8	677510.677	7063312.541	304.886	-7.5
10	677512.639	7063312.184	304.789	-7.5
11.018	677513.656	7063312.205	304.733	-7.5

SCALES AT A1 SIZE DRAWING

0 2 4 6 8 10m

#### SETOUT DETAILS MK33

	OLIOUI			
Chainage	Easting	Northing	Level	Radius
0	677513.656	7063312.205	304.718	
2	677515.648	7063312.383	304.609	
4	677517.64	7063312.56	304.467	
4.596	677518.233	7063312.613	304.421	0.6
5.538	677518.884	7063312.069	304.385	0.6
6	677518.925	7063311.609	304.395	
7.438	677519.053	7063310.176	304.423	

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A1 0

130 120 110

150 14.0

Chainage

0

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EXTERNAL REFERENCE FILES

Easting

Northing

677499.387 7063303.083

677498.752 7063304.973

677497.641 7063306.629

677496.133 7063307.934

677494.335 7063308.795

677492.373 7063309.152

677490.601 7063309.024

	<u>\</u>	2				
$\mathcal{E}$	~ ~ ~	~ `	~ ~	~ ~ ~	~ ~ ~	~ ~
>		SI	ETOUT	DETAILS	MK121	
(	Chainage	Ec	isting	Northing	Level	RADIUS
(	0	6775	501.939	7063337.363	304.57	-7.5
>	2	6775	502.573	7063335.473	304.587	-7.5
	4	6775	503.684	7063333.816	304.577	-7.5
(	5.89	677	503.36	7063329.999	304.545	-7.5
>	6	6775	505.191	7063332.511	304.542	-7.5
5	8	677	506.99	7063331.65	304.474	-7.5
(	10	6775	508.952	7063331.292	304.365	-7.5
>	11.781	6775	510.724	7063331.42	304.233	-7.5
5						
(						
>		SE	etout	DETAILS I	MK122	
5	Chainage	Ed	isting	Northing	Level	RADIUS
(	0	677	511.406	7063327.885	304.305	
>	2	6775	509.516	7063327.25	304.455	
5	4	6775	507.859	7063326.14	304.573	
(	5.938	6775	503.949	7063326.446	304.657	-7.5
~	6	6775	506.554	7063324.632	304.659	
5	8	6775	505.692	7063322.834	304.721	
(	10					
1	10	6775	505.335	7063320.872	304.765	
The Regional [ hese Drawings	Director does and/or Spe	6775 6775 not c	505.335 505.481 accept a ions. It	7063320.872 7063319.008 ny responsibili is expected th	304.765 304.79 ty for the nat the Cer	accuracy of tifying
the Regional I hese Drawings ingineer has e trawings and tain Roads re	Director does and/or Specented sour Specifications, quirements ho	not c cificat d eng and ave be	505.335 505.481 accept a ions. It gineering that all een take	7063320.872 7063319.008 ny responsibili is expected th judgement w site conditior n into accour	304.765 304.79 ty for the nat the Cer hen prepari is and Tran it.	accuracy of tifying ng the sport and Y
the Regional I hese Drawings ingineer has a trawings and tain Roads re TR	Director does and/or Specered sour specifications, quirements ho ANSPORT	6775 6775 not c cificat and eng ave be AND	505.335 505.481 ioccept a ions. It gineering that all een take MAIN	7063320.872 7063319.008 ny responsibili is expected th judgement w site condition n into accour <b>ROADS U</b>	304.765 304.79 ty for the nat the Cer hen prepari is and Tran it.	accuracy of tifying ng the isport and Y
he Regional [ hese Drawings ngineer has e rawings and lain Roads re TR Road: Carnar	Director does and/or Spe executed sour Specifications, quirements ho ANSPORT on Highway	6775 6775 not c cificat ave be ave be	505.335 505.481 ions. It gineering that all een take MAIN	7063320.872 7063319.008 ny responsibili is expected th judgement w site conditior n into accour <b>ROADS U</b> Parish of:	304.765 304.79 ty for the nat the Cer hen prepari is and Tran t. <b>ISE ONL</b>	accuracy of tifying ng the sport and Y
he Regional [ hese Drawings ngineer has a rawings and lain Roads re TR/ Road: Carnarv (Dist: 3.499)	Director does and/or Spe executed sour, specifications, quirements he ANSPORT on Highway km - 3.914	6775 6775 not c cificat ave be AND km	accept a ions. It gineering that all een take	7063320.872 7063319.008 ny responsibili is expected th judgement w site conditior n into accour <b>ROADS U</b> Parish of: Aux Dwg N	304.765 304.79 ty for the nat the Cer hen prepari is and Tran t. ISE ONL Roma o's.	accuracy of tifying ng the sport and Y
he Regional [ nese Drawings ngineer has a rawings and ain Roads re TR/ Road: Carnarv Dist: 3.499	Director does and/or Spe executed sour Specifications, quirements ho ANSPORT on Highway km - 3.914	6775 6775 not c cificat d eng , and ave be AND km	accept a ions. It jineering that all een take MAIN	7063320.872 7063319.008 ny responsibili is expected th judgement w site conditior in into accour <b>ROADS U</b> Parish of: Aux Dwg N LG: Maranoa	304.765 304.79 ty for the nat the Cer hen prepari is and Tran is and Tran t. <b>ISE ONL</b> Roma o's. Regional Cou	accuracy of tifying ng the sport and Y
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	Pt	Chainage	Easting	Northing	Level	Bearing
72"	START	0	677526.127	7063188.047	303.700	79°03'54.72"
72"	END	16.552	677542.378	7063191.187	303.500	79°03'54.72"

Radius	Chainage	Easting	Northing	Level	Radius
-7.5	0	677527.554	7063204.384	303.849	-7.5
-7.5	2	677528.189	7063202.494	303.789	-7.5
-7.5	4	677529.3	7063200.838	303.697	-7.5
-7.5	6	677530.808	7063199.533	303.575	-7.5
-7.5	8	677532.606	7063198.672	303.467	-7.5
-7.5	10	677534.568	7063198.315	303.411	-7.5
-7.5	11.781	677536.34	7063198.443	303.405	-7.5



**APPENDIX B – CONSTRUCTION SPECIFICATIONS** 

Project Name:	Roma Flood Mitigation Project
Project Number:	30031182
Report for:	Maranoa Regional Council

#### PREPARATION, REVIEW AND AUTHORISATION

Revision #	Date	Prepared by	Reviewed by	Approved for Issue by
1 – Tender	24/5/2013	M Vranes		A Zanetti
2 – 50%	25/10/2013	J Carroll	A Law	A Zanetti
3 – 80% A & C	29/11/2013	J Carroll	A Law	A Zanetti
4 – 80% B & D	20/12/2013	J Carroll	A Law	A Zanetti
A – IFC A & C	17/01/2014	J Carroll	A Law	A Zanetti
B – IFC B & D	30/01/2014	J Carroll	A Law	A Zanetti
C – IFC CH	26/02/14	J. Carroll	S. Runge	A. Zanetti
D – IFC Reissue	16/05/14	J. Carroll	A. Law	A. Zanetti
E – Verifier Comments	02/06/14	J. Carroll	A. Law	A. Zanetti

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#### SMEC COMPANY DETAILS

SMEC Australia Pty Ltd	
PO Box 5333 West End, QLD, 4101	

Tel: 07 3029 6747

Fax: 07 3029 6650

#### Email: Ashley.Zanetti@smec.com

#### www.smec.com

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# CONSTRUCTION SPECIFICATION FOR

# **ROMA FLOOD MITIGATION**

# DESIGN AND CONSTRUCT CONTRACT

For: Maranoa Regional Council June, 2014

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## 1.1 **PROJECT BACKGROUND**

The Roma Township is located in the Bungil Creek floodplain and has an extensive history of flooding, with a number of major flood events in the recent years resulting in significant damage to private property, public infrastructure and utilities. Three significant large flood events have occurred between March 2010 and February 2012, with the February 2012 flood being the largest flood event ever recorded in Roma and resulting in significant damage to over 500 premises.

As a result of the recent significant flood events the Roma Flood Study and Mitigation Project was developed, with an aim to minimize the flood risk to the Roma community and alleviate property inundation from the Bungil Creek catchment regional flooding. This project was broken up into three stages incorporating:

Stage 1: Flood study including hydrology and hydraulic modelling of selected design rainfall events and model calibration for the March 2010, April 2011, and February 2012 flood events;

Stage 2: Public consultation, development of a flood mitigation strategy and concept design of mitigation measures; and

Stage 3: Detailed design, construction and implementation of flood mitigation strategy.

## 1.2 **DESCRIPTION OF THE SCHEME**

The scope of the work executed under this Contract is to construct a levee approximately 5km long and associated infrastructure to protect the Roma Township.

### 1.3 **DESCRIPTION OF WORKS**

The principal items of work to be executed under this Contract as described herein and as shown on the drawings, include the following:

- 1) Mobilisation and demobilisation;
- 2) Dealing with water, including any pumping;
- 3) Creation of stockpiles areas;
- 4) Clearing and disposal of site vegetation;
- 5) Removal and stockpiling of topsoil;
- 6) Foundation excavation and preparation;
- 7) Construction of earthen levees to the design levels;
- 8) Disposal of spoil at locations as directed by the Principals Representative;
- 9) Topsoiling, hydromulching and seeding the crest and batters of the levee;
- 10) Installation of erosion protection including rip-rap;
- 11) Construction of cross drainage pipes;
- 12) Provision of temporary access roads where the works impact on existing roads and private accesses;
- 13) Property access ramps;
- 14) Road construction crossings at the Carnarvon Highway;
- 15) Construction of permanent road diversions where existing roads are cut off by the levee; and

### 1.4 STANDARDS

All materials, equipment and fittings supplied shall be new, and in accordance with the requirements of the relevant Australian Standard. Throughout this Specification, "AS" shall mean Australian Standard.

If any requirement of this Specification conflicts with the Standard requirements, then the specified requirements shall apply.

Wherever an Australian Standard or Code is specified herein, it shall mean the latest edition and/or amendment of that standard/code irrespective of the particular edition referred to in this Specification. The Standard Specification referred to in this Specification shall be considered to be part of the Contract Documents insofar as they apply to the works of this Contract.

#### 1.5 **INTERPRETATION**

Principal means the Maranoa Regional Council.

Principals Representative means the person authorized by the Principal, to act with the full authority of the Principal. For the purposes of this Specification, the Principals Representative is Ross Drabble of Maranoa Regional Council.

Defects Liability Period means the period during which the Construction Contractor is responsible for making good defects in a part or the whole of any works. The Defects Liability Period shall also include routine maintenance items.

The Contractor means 'Ostwald Brothers'.

The Designers Site Geotechnical Representative or DSGR is an experienced geotechnical professional from the design team. The Designers Site Representative is an experienced engineering professional from the design team.

#### 1.6 WORK EXCLUDED

Not Used

#### 1.7 **PROCEDURE OF DOCUMENTS**

In the event of any inconsistency between this document and any other documents, unless otherwise provided, the documents will rank in order of precedence in the order in which they are listed below:

- Project and company standards;
- Maranoa Regional Council Specification and Drawings
- DTMR specifications and standards;
- References to applicable Australian and International Standards; and
- "Issued for Construction" drawing

## 1.8 LOCATION OF WORKS

The site of the works of this Contract shall be in Roma (QLD) in the area bounded by Bungil Creek to the east, Roma Airport to the west, and Lovell Street to the south.

### 1.9 MATERIALS

All materials used in the works shall be handled, transported and stored in accordance with the relevant Australian Standard and the manufacturer's recommendations. Wherever any material or equipment is specified by a proprietary name or by the name of the manufacturer, such Specification is used for the purpose of describing the required standard and shall be considered as if followed by "or approved equivalent".

#### 1.10 DRAWINGS

The following Drawings are issued with this Specification and shall be read in conjunction with this Specification and shall form part of the Contract Documents:

Table 1.1 – Drawing	Schedule	
DRAWING NUMBER	DRAWING TITLE	REVISION
DI-0001	LOCALITY PLAN	С
DI-0002	DRAWING INDEX	E
DI-0011	GENERAL NOTES AND LEGEND	F
SW-0001	SCOPE OF WORKS AND KEY PLANS	С
TC-0101	TYPICAL CROSS SECTION AND DETAILS SHEET 1	С
TC-0102	TYPICAL CROSS SECTION AND DETAILS SHEET 2	С
TC-0103	TYPICAL CROSS SECTION AND DETAILS SHEET 3	С
CL-1001	CONTROL LINE AND FENCING SETOUT SHEET 1	С
CL-1002	CONTROL LINE AND FENCING SETOUT SHEET 2	С
CL-1003	CONTROL LINE AND FENCING SETOUT SHEET 3	D
CL-1004	CONTROL LINE AND FENCING SETOUT SHEET 4	В
CL-1005	CONTROL LINE AND FENCING SETOUT TABLES	E
WP-1001	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 1	C
WP-1002	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 2	С
WP-1003	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 3	C

Table 1.1 – Drawing Schedule
DRAWING NUMBER	DRAWING TITLE	REVISION
WP-1004	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 4	С
WP-1005	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 5	В
WP-1006	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 6	C
WP-1007	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 7	С
WP-1008	WORKING PLAN AND LONG SECTION CONTROL LINE MC10 SHEET 8	C
GA-0001	GENERAL ARANGEMENT - LEVEE CROSSINGS McPHIE STREET - PLAN AND SECTIONS	В
GA-0002	GENERAL ARANGEMENT - LEVEE CROSSINGS MISCAMBLE STREET - PLAN AND SECTIONS CONFORMING LAYOUT	В
GA-0003	GENERAL ARANGEMENT - LEVEE CROSSINGS MISCAMBLE STREET - PLAN AND SECTIONS ALTERNATIVE LAYOUT	C
PA-0001	PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS SHEET 1	C
PA-0002	PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS SHEET 2	С
PA-0003	PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS SHEET 3	С
PA-0101	PROPERTY ACCESS - LEVEE CROSSINGS PROPERTY ACCESS DETAILS	С
PD-0011	PAVEMENT GENERAL NOTES	E
PD-1001	PAVEMENT DETAILS	С
DD-0001	DRAINAGE CULVERT SECTIONS	D
DD-0111	DRAINAGE AND SERVICES DETAILS	В
DD-0111	PENSTOCK DETAILS SHEET 1	D

DRAWING NUMBER	DRAWING TITLE	REVISION
DD-0112	PENSTOCK DETAILS SHEET 2	В
XS-1001	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 1	В
XS-1002	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 2	В
XS-1003	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 3	С
XS-1004	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 4	С
XS-1005	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 5	С
XS-1006	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 6	В
XS-1007	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 7	В
XS-1008	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 8	В
XS-1009	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 9	В
XS-1010	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 10	В
XS-1011	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 11	В
XS-1012	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 12	С
XS-1013	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 13	В
XS-1014	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 14	В
XS-1015	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 15	В
XS-1016	ANNOTATED CROSS SECTION CONTROL LINE MC10 SHEET 16	В
GE-0101	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS OVERALL PLAN	D
GE-1001	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 1	C

DRAWING NUMBER	DRAWING TITLE	REVISION
GE-1002	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 2	C
GE-1003	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 3	C
GE-1004	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 4	В
GE-1005	GEOTECHNICAL INVESTIGATION AND BORROW PIT LOCATIONS SHEET 5	C
ST-1002	WALL LEVEE STRUCTURAL DETAILS	В
PU-1001	PUBLIC UTILITIES PLAN SHEET 1	С
PU-1002	PUBLIC UTILITIES PLAN SHEET 2	C
PU-1003	PUBLIC UTILITIES PLAN SHEET 3	C
PU-1004	PUBLIC UTILITIES PLAN SHEET 4	В
PU-1011	PUBLIC UTILITIES PLAN MCPHIE ST	В
PU-1020	PUBLIC UTILITIES PLAN TREATMENT FOR SERVICES UNDER LEVEE	В
TC-1101	CARNARVON HIGHWAY - TYPICAL CROSS SECTION	С
CL-1101	CARNARVON HIGHWAY - CONTROL LINE SETOUT	С
WP-1101	CARNARVON HIGHWAY - WORKING PLAN AND LONGITUDINAL SECTION SHEET 1	D
WP-1102	CARNARVON HIGHWAY - WORKING PLAN AND LONGITUDINAL SECTION SHEET 2	D
XS-1101	CARNARVON HIGHWAY - ANNOTATED CROSS SECTIONS SHEET 1	D
XS-1102	CARNARVON HIGHWAY - ANNOTATED CROSS SECTIONS SHEET 2	D
XS-1103	CARNARVON HIGHWAY - ANNOTATED CROSS SECTIONS SHEET 3	D
DD-1101	CARNARVON HIGHWAY - DRAINAGE CROSS SECTION AND SCHEDULE	D
PA-1101	CARNARVON HIGHWAY - PROPERTY ACCESS PLAN SHEET 1	C

DRAWING NUMBER	DRAWING TITLE	REVISION
PU-1101	Public Utilities Plan Carnarvon Highway	С

- 1) The Drawings issued with this Specification will be supplemented or superseded by such additional general and detailed Drawings as may be necessary as the work progresses.
- 2) Such additional general and detailed Drawings will show dimensions and details necessary for construction purposes more completely than are shown on the attached Drawings for all features of works. The Contractor will be required to perform the work on these features and in accordance with the additional general and detailed Drawings mentioned above. The Contractor shall check all Drawings carefully before construction and advise the Designers Site Representative of any discrepancies, errors or omissions and full instructions will be furnished to the Contractor should any discrepancies, errors or omissions be found. The Contractor shall allow at least 8 working days for the Designs Representative to issue his instruction.

### 1.11 SERVICES

It is the responsibility of the contractor to ensure all services are located, position marked and hand excavated as appropriate prior to undertaking any works at the site. It shall be the sole responsibility of the Contractor to locate all existing buried formal and informal, public and private services prior to and during works in the area and to advise the designer site representative to assess the protection measures required. The Contractor must seek the assistance from Authorities in confirming the location and depth of all services.

While not all services require augmentation under this contract, the Contractor is advised that as a minimum the following services are known to exist at the site:

- Telstra Telecommunications underground cables throughout the works area;
- Ergon Energy Overhead power lines;
- Maranoa Regional Council Pressurised water mains, pressure sewer, gravity sewer mains and sewer rising mains throughout the works area;
- Private Irrigation lines; and
- Private plumbing lines.

### 1.12 **SURVEY**

#### **BASE LINES AND BENCH MARKS**

The contractor shall establish base lines and bench marks, as shown on the drawings, which the Contractor shall use to do the detailed set-out. Contractor shall have the bench marks verified with a qualified surveyor before adopting them as being a true benchmark for construction purpose. If such base lines or bench marks are disturbed in any way by the Contractor, they will be re-established by the Principal's Representative at the Contractor's expense.

#### DETAILED SET-OUT

The Contractor shall use appropriately qualified people to do the detailed set-out. Detailed set-out shall include establishment of all lines, levels and grades necessary for the earthworks construction. Full records of all field work shall be kept in accordance with good surveying practice. Field books shall be available for inspection by the Principal's Representative at any time.

#### APPROVAL OF SET-OUT

The Contractor shall be responsible for the accuracy of the detailed set-out. On completion of the detailed set-out of each portion of the works, the Contractor shall notify the Principal's Representative, in writing, and shall obtain approval to proceed with construction. The Principal's Representative may elect to check the Contractor's set-out, but approval to proceed, whether or not the Principal's Representative has checked the set–out, shall not diminish the Contractor's responsibility for the accuracy of the work.

### 1.13 SITE SECURITY

The site shall be secured at all times from unauthorised access during the currency of the contract.

# PART B: ENGINEERING SPECIFICATION FOR LEVEE EARTHWORKS, STRUCTURES, AND DRAINAGE

# **1 EARTHWORKS MATERIALS**

### 1.1 GENERAL

This section identifies specification for acceptance of use of the following material types in the placement of earthworks for the works:

- 1) Topsoil
- 2) Type B: General Fill
- 3) Type C: Structural Fill
- 4) Type D: Rip Rap

No earthworks materials other than specified above shall be used on this project, unless formally agreed in writing with the Designer.

Placement of the above type materials shall occur at locations shown on the drawings.

Earthworks Materials management shall be undertaken by the Contractor. The Contractor shall be responsible for estimation of and placement of the relevant:

- Quantities of site won material available for use in the earthworks, and
- Quantities of material required to be imported from offsite to balance cut/ fill requirements

Materials compliance testing, documentation and facilitation of review shall be undertaken by the Contractor in accordance with this specification and the design documentation.

Note that this specification does not include provision for consideration or compliance testing relating to environmental issues. Refer to relevant Environmental reports/ specifications for guidance.

### 1.2 TYPE A- TOPSOIL

Topsoil shall be fertile, friable soil containing organic matter which is reasonably free from subsoil, refuse, tree roots larger than 20 mm in diameter and 300 mm in length, noxious weeds, clay lumps and stones larger than 50 mm diameter. All Topsoil shall comply with AS 4419.

The Contractor shall manage available quantities of topsoil to meet placement requirements.

Topsoil shall be spread over the embankments to achieve the design minimum thickness measured normal to the slope as shown on the drawings. Topsoil shall be placed at moisture content which will allow for uniform spreading and compaction.

### 1.3 **TYPE B- GENERAL LEVEE FILL**

Type B material shall be used for placement in areas specified in the drawings.

The contractor must demonstrate that any Type B material placed meets the minimum material classification (refer Clause 1.6) and placement requirements in accordance with this specification.

It is preferred that Type B materials comprise naturally formed material won from cuttings or borrow pits. If material mixing or treatment is required to allow material to conform to this specification, the contractor shall submit details to the Designers Site Geotechnical Representative (DSGR) for review and approval. The Contractor shall allow 5 working days for review and approval of treated or mixed material. Review and approval of mixed or treated material to be used as Type B material shall constitute a **HOLD POINT**.

The fill shall have a low shrink/swell capacity. If suspected highly reactive soils are encountered (i.e. 'Black Soil') in either the levee foundation or potential borrow areas, relevant testing shall be carried out to further investigate the soil's shrink swell potential. Selected use of soils of moderate reactivity may be possible following verification by test results and confirmation by the DSGR. These soils may be able to be used as Type B select fill under the following provisos:

- Placement if possible be confined to only within the levee foundation or;
- Placement be confined to no nearer than a scraper width from the levee face if placed above the surrounding natural surface level within the embankment, and
- Placement within the embankment shall be confined to no higher than 1m below the final levee crest level.

The fill shall not contain any organic material, rocks, stones or any other hard materials that can retain on 75mm sieve size or any other unsuitable material, which could prejudice the integrity of the levee.

### 1.4 TYPE C- STRUCTURAL FILL

Type C material (Structural fill) shall be used for placement in areas specified in the drawings where special performance is required. Such areas include backfill around culverts and pipes penetrating through the levee. At these locations, low permeability, non- dispersive, fill shall be used to limit and mitigate preferential flow of water along the line of the pipe and through the levee. If non dispersive fill is unable to be sourced economically, consideration may be given to using type B fill modified with 2-3% lime. If this option is desired, the contractor shall submit a proposal to the designer detailing mixing trials, validation and testing proposed to demonstrate acceptability of the revised material.

The contractor must demonstrate that any Type C material placed meets the minimum material classification and placement requirements in accordance with this specification.

### 1.5 **TYPE D- RIP RAP**

Type D material (Rip Rap) shall be used for placement in areas specified in the drawings where it is sought to provide erosion/ scour resistance for the works. From available site investigations, it is considered likely that rip rap will be imported from off site. Type D fill shall comprise material that is clean, hard, dense, durable, and resistant to weathering, free from overburden, spoil, shale and organic matter. Laminated, fractured porous or physically weak rock shall not be used.

The contractor must demonstrate that any material placed in designated Type D area meets the minimum material classification and placement requirements in accordance with this specification. The Contractor shall submit to the Designer details of any product to be used as an alternative **HOLD POINT**.

### 1.6 MATERIALS CLASSIFICATION

This Section defines acceptability criteria for the various types of earthworks materials that will be used in the works, based on material classifications. Materials used in the earthworks shall conform to the requirements shown in Table A1 (Appendix A).

### 2.1 GENERAL REQUIREMENTS

Prior to commencement of clearing or excavation the Contractor shall make all enquires and investigations for the protection of services from damage and construct all temporary or permanent re-arrangements required under any of the relevant clauses of the General and Special Conditions of Contract and the relevant Authorities. The Contractor shall indemnify the Principal and the Principals Representative relating to the Contractor's failure to undertake necessary arrangements, which results in damage to the services.

### 2.2 **PROGRAM**

The Contractor shall not commence excavation for the Works, until in the opinion of the Principals Representative, sufficient plant and material are available at the site to ensure satisfactory, safe and uninterrupted progress of the works.

Some of the work is to be undertaken on private property. The Principal's Representative shall make all necessary enquiries and obtain relevant approvals to facilitate the proposed works within private property. At least seven days' notice to the Principal's Representative of the requirement to commence work within each property is required as a minimum.

### 2.3 **PROTECTION OF SERVICES**

In preparation for any excavation work, the Contractor shall carry out field investigation in association with the relevant Authorities and property owners to locate and mark the exact positions and routes of all services in the area of works. It is the Contractor's responsibility to gain accurate information regarding the depth, size, alignment and other services that may be in close proximity. Special precautions shall be taken where excavations are made near any other services. These include overhead power cables, underground power and telecommunication cables, drains, sewers and water mains and the like. For these assets and any other services not specifically mentioned, the Contractor shall:

- Comply with the notification requirements of the construction conditions on the Design Drawings.
- Take special care to ensure that the services are protected in accordance with the conditions specified by the controlling Authority.
- Arrange for a representative from the controlling Authority to be present, unless the Authority directs otherwise, whenever the Contractor is:
  - Proving the locations of services.
  - Excavating within three (3) metres of the service.
- Arrange with the appropriate Authority for the closure and subsequent restoration of any service that must be shut down while the Works are in progress.
- Adopt a method for uncovering and protecting the service from damage if the service must be uncovered and left exposed, to the satisfaction of the controlling Authority.
- Immediately inform the owner or controlling authority of any damage or interference to any service, structure or property.
- Carry out any temporary bypass and restoration of the services to the satisfaction of the respective Authorities and property owners.

It shall be noted that the assessment and location of services does not comprise part of the IFC design package. Accordingly, the contractor shall take all necessary measures to assess, measure control and mitigate impacts of services on the works and vice versa.

Existing services passing under or through the proposed levee shall be excavated and backfilled in accordance with the design drawings.

# Hand excavation shall be used in close proximity to all such services until the exact location is determined.

Excavations containing underground services shall be backfilled so that the sub-grade is restored as nearly as possible to its original state of compaction. Where selected backfill has been placed by other utilities and has had to be removed, it shall be replaced as specified in the design documentation. All backfill shall be carefully deposited in the trench and around the utility service in layers and adequately compacted by appropriate hand rammers and tampers, or by use of effective mechanical equipment.

Extra care shall be taken by the Contractor to re-compact excavations near existing underground pipework, so that foundations of that pipework are restored.

The Contractor shall be responsible for any damage caused to existing services. The Contractor shall notify owners of utility services that are damaged by construction activities.

Before commencing excavation the Contractor shall ensure adequate measures are taken to protect or to temporarily remove any obstructions which may exist on or adjacent to the service centreline.

### 2.4 **PROTECTION OF PROPERTY**

The Contractor shall protect the property and infrastructure outside the limits of the works.

All care shall be taken to ensure that all properties etc. within and adjacent to the construction site shall not be damaged during the contract. The Contractor shall be responsible for any damage caused and costs incurred.

### 2.5 ALIGNMENT AND SETTING OUT

Set out for the components of the proposed diversions is provided on the drawings. The contractor shall ensure that the set out is correct as per the issued for construction drawings.

### 2.6 CLEARING AND GRUBBING

- The entire areas to be excavated, plus a width of 6 metre from the edge of the excavation of levee cross section and perimeter zone around the structures shall be cleared of vegetation and grubbed by removal of all trees, scrub, grass and stumps, including removal of stumps and roots to a depth of 500mm below the natural surface or the finished surface of the excavation whichever is the lower.
- 2) All clearing shall be carried out to prevent as far as possible the loss of topsoil. Topsoil removed as part of the excavation necessary for the construction of works, shall be stockpiled separately for subsequent surface restoration at regular intervals around the levee in locations which do not interfere with levee works, traffic or normal land use.
- 3) Materials removed in the clearing operations shall be mulched or otherwise disposed of as approved by the Principals Representative and relevant local statutory Authorities. All materials to be mulched shall be piled, cut and split if necessary and when in a suitable condition shall be mulched.
- 4) Grub holes formed in the foundation area of any items in (1) above shall be filled and compacted promptly using sound material to prevent the infiltration and ponding of water by the method of compaction specified for the embankment foundation as described in this specification.

### 2.7 **STRIPPING**

- 1) The entire area where fill is to be placed shall be stripped to remove all topsoil and unsuitable materials. The unsuitable materials shall include all debris, very loose, loose, soft or firm material, clean sand and gravel, topsoil, vegetation and root affected material and all other perishable and or objectionable materials that may be unsuitable for use for permanent construction required under this Specification or support the proper compaction of the materials in the embankments.
- 2) Topsoil so stripped shall be temporarily stockpiled to be spread on newly filled and compacted surfaces to establish a vegetative cover.
- 3) Other material so stripped and excavated shall be stockpiled for rehabilitation of the borrow pits or offsite disposal as directed by the Principals Representative.

Stockpiles shall be in maximum 2 metre high stockpiles with 2 horizontal to 1 vertical slopes or disposal of as directed.

### 2.8 EXCAVATION FOR CUT-OFF TRENCH

- 1) Cut-off trenches shall be excavated to the width and depth as shown on the Drawings, unless otherwise approved in special cases by the DSGR.
- 2) If any shrinkage crack is observed at the bottom of the cut-off trench, prior to backfill, the depth of cut off trench shall be increased until no further crack is observed. The final base width of the cut off wall shall not be less than 0.6m.
- 3) If the material excavated from the trench, in the opinion of the DSGR, is re-useable with or without any treatment, the excavated material shall be separately stockpiled. The material not suitable for re-use shall be stockpiled for the rehabilitation of the borrow pits at regular intervals around the levee that do not interfere with levee works or normal land use as approved by the Principals Representative.

### 2.9 **EXCAVATION FOR FOUNDATIONS**

- 1) Foundations shall be excavated to the width and depth as shown on the Drawings, unless otherwise approved in special cases by the DSGR.
- 2) If any shrinkage crack is observed at the bottom of the cut-off trench, prior to backfill, the DSGR shall be notified.

### 2.10 EXCAVATION FOR STRUCTURES

- Excavation for culverts, cut-off walls, sumps and headwalls shall in general be carried out by open trench or cut methods and unless otherwise stated, the requirements of this Specification and the design documentation shall apply to these methods. The Contractor shall obtain the approval of the DSGR if other methods are envisaged.
- 2) The Contractor shall ensure that the bed level of the excavation remains in an undisturbed condition, suitable for the installation of structures or pipes.
- 3) If, in the opinion of the DSGR, the foundation becomes unusable because of the neglect or other fault of the Contractor, the Contractor shall forthwith after being notified by the DSGR remove the unsuitable material and backfill the over-excavation to the satisfaction of the DSGR and at no expense to the Principal.
- 4) Where a foundation is over-excavated for any reason, the foundation shall be made good at the Contractor's cost in the following manner:-

- a) Backfilling under concrete structure shall be made with concrete of compressive strength, f'c, not less than 15 MPa or with Type C material where detailed on the drawings.
- b) Backfilling in pipe trenches shall be made with the pipe bedding specified in this Specification and written verification given to the Principals Representative.

### 2.11 EXCAVATION FOR BORROW MATERIALS

- All sources of borrow materials shall be submitted for approval by the Contractor to the DSGR twenty working days before the planned commencement of operations. This is to allow for adequate testing and analysis of the material in order to ensure its suitability for use <u>HOLD POINT</u>
- 2) The commencement and on-going excavation of borrow pits will be the responsibility of the Contractor and Level 1 Supervisor to continually monitor the material excavated to ensure its conformance to requirements of the specification. The delivery of conforming material from borrow areas into the levee embankment constitutes a <u>HOLD POINT.</u>
- 3) The Contractor is responsible for the provision of adequate drainage, obtaining any permits required for entry to the land. The contractor shall also comply with any Environmental planning legislation, Local Government Planning Scheme, local laws, local planning policies and land owners as appropriate.
- 4) Borrow Pits require a clear exclusion corridor of 30m of existing undisturbed ground surface between the levee toe and the borrow area top of batter.

### 2.12 SECURING EXCAVATIONS

- 1) The Contractor shall be responsible for securing all excavations against movement by means of:-
  - Stable batters
  - Timbering
  - Sheet Piling
  - Protective Shields

or such other methods or combination of methods that will ensure the safety of all employees, the Public, animals and the protection of the Works, and public and private property. Should the Principals Representative determine that any excavation requires timbering, sheet piling or shields as necessary for security then the Contractor shall provide such measures as the Principals Representative may direct.

- 2) The Contractor shall provide protection for persons and animals as set out in the General and Special Conditions of Contract.
- 3) The Contractor shall make adequate provision for the protection of persons and livestock from injury liable to arise from the presence of the open excavation.
- 4) Removal of trench supports shall be carried out in a safe manner.

### 2.13 DRAINAGE AND DEWATERING

- 1) The Contractor shall take all necessary care to protect the works to prevent any surface water and groundwater entering the excavations.
- 2) In wet ground locations the Contractor shall design, install and operate a dewatering system including all necessary gravel and pipe drains, sumps, pumps and other equipment to maintain the excavation in a dry condition. Where necessary, external

dewatering in the form of a system of well points or other dewatering devices sunk in the ground in the vicinity of the trench shall be installed. The system shall be operated continuously from the time of excavation of the trench to the time of backfilling the trench. The dewatering shall be accomplished in a manner that will prevent loss of fines from the foundation, will maintain stability of the excavated sides and bottom of the excavation and will result in all construction operations being performed in the dry.

- 3) The Principals Representative shall approve the method of disposing water removed or diverted from the excavation.
- 4) The Contractor shall be responsible for and shall repair, at the Contractor's expense and to the approval of the Principals Representative, any damage to foundations, excavated slopes or any other part of the works, either permanent or temporary, caused by water, floods or failure of any part of the dewatering system.
- 5) No excavated material shall be stored so as to endanger the stability of the excavation and in no case stored so that the toe of the spoil bank is closer than 1 metre to the edge of the excavation.

### 2.14 DISPOSAL OF SURPLUS SPOIL

Notwithstanding the requirements of this "Specification", any material surplus after completion of all earthworks, backfilling and site grading, together with any rejected or unsuitable material shall be disposed of at locations approved by the Principals Representative. Dumped spoil shall be placed, spread, levelled and compacted.

### 3.1 **GENERAL**

Embankment foundation preparation shall be conducted in accordance with this specification and design documentation.

The Contractor shall not commence placing any fill on the prepared areas until the DSGR have inspected these areas and given consent to proceed.

Inspection of areas upon which fill is to be placed shall constitute a HOLD POINT.

Where groundwater or seepage is encountered the Contractor shall notify the DSGR and any action to be taken shall, unless otherwise specified, be submitted to the DSGR for review. Review of the Contractor's proposals for the treatment of groundwater shall constitute a **HOLD POINT**.

### 3.2 **PROOF ROLLING**

The following areas shall be proof rolled:

- All foundations for embankments prior to placement of any fill material
- Over all of the formation at design cut floor level
- Over cut-fill transition zones
- Over the access ramp subgrades and top embankment access road

Proof Rolling shall be witnessed by the DSGR and shall constitute a HOLD POINT.

Proof rolling for cut and fill areas:

- Shall be conducted prior to any hauling or further material placement over the prepared area
- Shall be conducted within 2 days of stripping

Equipment nominated for use in test rolling procedures shall be approved by the DSGR before commencement of proof rolling and shall comply with the following requirements:

- Static smooth steel wheeled rollers shall have a mass of not less than 12 tonne and load intensity under either the front or the rear wheels of not less than 6 tonne per metre width of wheel.
- Pneumatic tyred plant shall have a ground contact pressure under either the front or rear wheels of not less than 450 kPa per tyre. The area over which this ground contact pressure shall be applied shall not be less than 0.035 m<sup>2</sup> per tyre.
- Compliance with the test rolling requirements shall be when an area withstands test rolling without visible deformation or springing. Material which is observed to deform or spring under loading is deemed unsuitable material.

### 3.3 UNSUITABLE MATERIAL

Where unsuitable material is encountered the Contractor shall submit to the DSGR for review the proposed in-situ treatment or extent of excavation required based on as encountered ground conditions <u>HOLD POINT.</u>

Unsuitable material is defined as material which, in its current position and state, is unsuitable for the support of layers of fill or pavement. It is identified by:

• Deformation, rutting, softness, yielding, distress, or instability under proof rolling or the loading from construction machinery

- A visual assessment of its properties, such as organic content or moisture content, which indicates that it will be unsuitable for its intended use
- Material that is of variable strength and/or compaction, loose, wet, soft, firm, containing high permeability layers, organics, roots, fissuring

### 3.4 FOUNDATIONS FOR LEVEES

Prior to the placement of fill, unsuitable material shall be assessed and removed by the Contractor. Following test rolling, the approved foundations shall then be scarified to a minimum depth of 50mm and moisture conditioned to ensure that an adequate bond will develop between the foundation and the first layer of filling.

The test rolling shall be carried out on the exposed surface, provided that it is flatter than 1H:4V, in accordance with the Contractor's approved proof rolling method and in accordance with Clause 3.2. Unstable areas detected by test rolling shall be rectified in accordance with the requirements of Clause 3.3.

Prior to earthfill placement, the embankment foundation shall be inspected by the DSGR. The DSGR inspection shall include witnessing the test rolling of the foundation in accordance with requirements listed in Section 3.2 of this Specification. This inspection constitutes a **HOLD POINT**.

# **4 PLACEMENT AND COMPACTION OF EARTHWORKS**

Each load of the materials placed, whether from excavation or stockpile, shall be placed so as to secure, in the levee, the best distribution of the material and avoid pockets of non-homogeneous materials.

Layers of material shall not differ substantially from the surrounding material in the zone and the density shall be uniform throughout each compacted layer.

When each layer of material has been prepared in accordance with this sub clause, it shall be compacted and tested.

Delivery of imported material shall comprise a HOLD POINT.

Verification of conformity of selected material shall comprise a HOLD POINT.

The Contractor shall not proceed with subsequent layers of construction until testing for material and placement compliance has been completed, the work has been inspected and approval to proceed has been issued by the Contractor's Quality Team.

Prior to placement of a new layer of material the surface shall be adequately scarified to a minimum depth of 50mm and moisture conditioned to ensure that an adequate bond will develop between layers. The Contractor shall notify the Principal's Representative and the DSGR of any non-conformances within 24 hours of the results being available.

Minimum Testing and Acceptance criteria for Earthworks Materials shall be undertaken in accordance with Table A3 (Appendix A). The Contractor's testing program shall be such that the number of tests are not less than the testing frequencies specified hereinafter.

Placement and compaction of earthworks shall be completed in accordance with Table A2 (refer Annexure A). Placing and compaction of material should be witnessed by the Contractors Quality Team.

Appropriate dust suppression measures shall be implemented by Contractor.

#### 4.1 Filters

Filter material shall be imported, hard, dense and durable crushed rock, natural sand or other material approved by the Principal, free from perishable matter, lumps or balls of clay or other deleterious materials conforming to the grading requirements shown in Table-1 following placement. The grading of the filter materials shall be approximately parallel to the specified grading limits.

Where filters comprise crushed rock, the source rock used for the production of filter materials shall be igneous or metamorphic rock with:

- a specific gravity not less than 2.65 tonne/m3;
- a Los Angeles Abrasion Loss at 500 revolutions not exceeding 35%;
- a Sodium Sulphate Soundness not exceeding 10% (overall weighted loss);
- a Degradation Factor not less than 45 in the case of acid igneous or metamorphic rock or a Secondary Mineral Content greater than 25% in the case of basic igneous source rock.
- a flakiness index not exceeding 25%
- a wet/dry strength variation not greater than 35%
- water absorption not exceeding 2.5%

Vesicular materials shall not be used for the production of filters. Where filters comprise natural sands, the source material used for the production of filter materials shall have:

- a Sodium Sulphate Soundness not exceeding 10% (overall weighted loss);
- a water absorption not exceeding 2.5%
- a particle density not less than 2.5 tonne/m3

Sieve Size (mm)	Percentage of Particles Passing Sieve (%)		
	Zone 2A (fine filter)	Zone 2B (Coarse Filter)	
75		100	
53		100-92	
37.5		100-84	
26.5		93-71	
19		86-60	
13.2		77-46	
9.5	100	68-33	
6.7	100-94	54-20	
4.75	100-86	42-11	
2.36	100-64	16-0	
1.18	76-40	5-0	
0.6	50-20		
0.425	40-12		
0.3	30-6		
0.15	15-0		
0.075	3-0		

#### Table-1 – Grading Requirements for Filters

The Contractor shall provide details, source and description of the filter materials including test results for the Principal's approval. The Principal may organise further testing of the filter material as required. The approval of the filter materials by the Principal shall constitute a <u>HOLD POINT</u>.

### 4.2 Placement of Filter Materials

Filter material shall be placed to the levels, lines, slopes and dimensions shown on the Drawings. Foundations for filter at the toe of the existing embankment shall be graded to slope toward the downstream batter point.

Blending of filter materials on the dam embankment, to produce the required grading, will not be permitted.

Unless otherwise approved by the Principal, filter material shall be placed in generally horizontal layers of not greater than 300mm finished thickness, except that no less than 2 approximately equal layers shall be placed for each filter zone. The Contractor's procedures for the placement of the filter zones shall be such as to prevent segregation, the formation of voids in the filter zone, contamination of filter materials or damage to the filter layer when adjacent earthworks materials are placed. Filter zones shall be constructed so that there is no differential height between adjacent filter zones unless otherwise shown on the Drawings.

The Contractor shall submit for review by the Principal, his procedures for placement of the filter material. Submission of the Contractor's procedures for placement and compaction of the filter material shall constitute a <u>HOLD POINT</u>.

The filter material shall be placed moist. Care shall be taken to ensure that no contamination of the filter occurs and that the filter is free draining after compaction. All filters below the level of the top of the parapet wall base shall be compacted as follows:

- Zone 2A to a density index not less than 75% as determined in accordance with AS1289.5.6.1; and
- Zone 2B with a minimum of 4 passes of a vibrating smooth drum roller meeting the requirements specified in Clause 5.11

Above the level of the base of the parapet wall there is no compaction requirement (minimum density ratio) specified for filter materials and the degree of compaction obtained shall be only that necessary for stability, safe working and workability considerations.

Once placed, filters shall be maintained in a moist condition until covering materials are placed.

Filter materials that become contaminated by other materials or which, after placement, do not conform to the requirements of the Specification, shall be removed and replaced at the Contractor's expense.

Filter Material	Grading	Compliance Test – one test for each 1000m <sup>3</sup> for each of Zone 2A and 2B material prior to placement.
		Record Test – one test for each 500m <sup>3</sup> for Zone 2A material after compaction
		Record Test – one test for each 1000m <sup>3</sup> for Zone 2B material after compaction.
	LA Abrasion, Wet-Dry Strength, Sodium Sulphate Soundness, Degradation Factor, Flakiness Index, Water Absorption	Compliance Test – One test for each source of filter material carried out prior to delivery. Test to be carried out within the first 500m <sup>3</sup> of material produced.
	Field Density, Field Permeability	Compliance Test – every lot (lot not to exceed one days production or 500m <sup>3</sup> ) for Zone 2A.
		Record Test – one test for each 500m° for Zone 2A materials.
		Record Test – one test for each 1000m <sup>3</sup> for Zone 2B material.

Testing requirements for filters

# **5 TESTING AND ACCEPTANCE OF EARTHWORKS**

The Contractor shall supply written verification that the minimum requirements have been complied with and supply the verification with the lot package. The Contractor shall carry out testing for compliance to the frequency and value requirements listed in Appendix - 1. All testing shall be conducted and verified by a NATA accredited NATA Geotechnical testing authority. The Contractor shall not proceed with subsequent layers of construction until testing for material and placement compliance has been completed, the work has been inspected and approval to proceed has been issued.

The Contractor shall notify the Principal's Representative and the DSGR of any nonconformances within 24 hours of the results being available.

Level 1 supervision of the works shall be undertaken in accordance with the requirements of AS3798-2007. The Level 1 supervisor shall provide written evidence to the Design team that the works witnessed and undertaken were done in accordance with this specification. Verification details shall be submitted to the Principals Representative for review and acceptance of each lot.

Following a non-conformance, a report shall be sent to the DSGR and Principal's Representative clearly describing the details of the non-conformance. The non-conformance report (NCR) shall include details of the proposed resolution and shall have non-conforming verification records attached. The closed out NCR shall have documentation that the proposed resolution measures have been carried and shall identify that all permanent works conform to specification. It is a requirement that all NCRs be accompanied with written and signed evidence by the Level 1 Supervisors daily report verifying the details of the works carried out.

All minimum requirements of the specification are presented in respect to material properties, earthworks levels and position, and earthworks placement and compaction. All test results and associated verification reports shall be submitted to the Principals Representative for review prior to completion of the contract. This shall constitute a **HOLD POINT**.

At least 2 working days prior to commencement of earthworks, the contractor shall submit an inspection and testing plan to the Principals Representative for review and approval. This shall constitute a **HOLD POINT**.

Minimum Testing and Acceptance criteria for Earthworks Materials shall be undertaken in accordance with Table A3 (Appendix A). The Contractor's testing program shall be such that the lot sizes are not greater than the sizes given in Appendix A and the number of tests are not less than the testing frequencies specified hereinafter.

Earthworks Levels and Positions shall be constructed to levels as shown on the Drawings. Acceptable tolerances for Earthworks Levels and Positions are as per Table A4 (Appendix A).

Testing and acceptance of Earthworks with respect to placement and compaction shall be undertaken in accordance with Table A5 (Appendix A).

# **6 HOLD POINTS**

Table A6 (Appendix A) summarises requirements for Hold Points that are relevant to this specification.

### 7.1 **GENERAL REQUIREMENTS**

- 1) The Contractor shall complete the backfilling of all excavations carried out in accordance with this specification without undue delay as soon as testing and/or curing of the works constructed therein permits. For concrete works, this shall be permitted when the concrete strength achieves 25 MPa.
- 2) Except where otherwise directed or approved by the Principals Representative for safety reasons, the Contractor shall remove all supports for the excavations as backfilling progresses. Removal of supports shall be carried out so as not to endanger personnel or the Works. Any voids left by the removal of supports shall be backfilled to the same standard specified for the adjacent excavation.
- 3) All backfilling operations, including removal of supports or excavations, shall be conducted in a manner that will prevent disturbance of the pipes, fittings or pipe bedding and any other damage to the Works.
- 4) The backfill shall not be placed until the pipe bedding has been brought to the required lines and levels as shown on the Drawings and thoroughly compacted.

### 7.2 BACKFILL MATERIAL

- 1) All backfill materials placed within or beneath the levee and around structures shall be Type C in accordance with this Specification and the design documentation.
- 2) The appropriate backfill for different type of backfilling shall be as specified in the relevant Clauses of this Specification and on the Drawings (if required).
- 3) Normal compaction by small tamping rollers may be resumed in the vicinity of the pipes in accordance with pipe manufacturer's recommendations.

### 7.3 **PIPE HAUNCHING AND COVERING**

- 1) All sections of the pipeline shall be backfilled to a minimum height of 300mm minimum above the top of the pipe with Type C fill unless otherwise specified on the Drawings.
- 2) During backfilling operations the backfill shall be placed symmetrically so as to keep the backfill on both sides of the pipeline at the same level. Care shall be taken in compacting to avoid disturbing the pipe.
- 3) The backfill shall be compacted with hand tampers or whacker packers (where possible), in layers not more than 150 mm thick, to obtain a dry density in excess of 98% of the maximum dry density.

### 8.1 **GENERAL**

The batters of the levee are to be covered with topsoil and grass seeding so as to provide low maintenance, long term stabilization of the surface against erosion and to provide acceptable appearance to the area.

The restoration works includes, but is not limited to:

- Supply of topsoil (or reuse of stockpiled topsoil as appropriate), topsoil, grass seeding and water.
- Spreading, fertilising and cultivating topsoil.
- Temporary stabilisation of surfaces prior to grass growth.
- Maintenance of grassed areas for the duration of the Defects Liability Period. Maintenance will include, but not limited to, watering, application of fertiliser and cultivating the grass as appropriate.

### 8.2 GRASS TYPE

All grass seeds shall be Buffel Grass and applied at a rate of 3kg/Ha. Grass seeding shall comply with TMR technical standard MRTS 16C.

The cover seed to be used is seasonal specific. During October to March Japanese Millet, sterile hybrid version. During Aptril to September Wimmera Rye, sterile hybrid version. Both are to be applied at a rate of 10kg/Ha.

The contractor should ensure that any cattle stay off the area until the Buffel grass is established.

### 8.3 FERTILISER

All fertilisers shall be pre-packed and certified commercially prepared mix, having a N:P:K ratio of 8:4:10 (commonly known as 'orchard fertiliser') and shall be delivered to the site in standard bags showing weight, composition and vendor's name and be available for inspection on site.

Fertiliser shall be applied at a rate of 50 g/sq.m.

### 8.4 SUB-GRADE PREPARATION

The Contractor shall prepare sub-grades prior to the spreading of topsoil as specified below.

Spray existing weeds with herbicide Glyphosate in any of its registered formulations at the maximum rate specified on the label for the control of weed(s) two weeks before cultivation. Areas to be ripped shall not be worked when sub-soil is wet or plastic.

Allow for clearing and removing stones exceeding 25 mm and any rubbish brought to the surface during cultivation.

Trim surfaces to shape after cultivation.

# 9 **EROSION PROTECTION**

Where specified on the drawings additional erosion protection is required above the protection provided by a grassed surface. Two different erosion protection treatments have been specified:

- 1) Rip Rap Rock Protection (Type D material)
- 2) Turf Reinforced Matting
- 3) Reno Mattress

### 9.1 **RIP RAP ROCK PROTECTION – Type D**

#### 9.1.1 General

Rip rap rock protection shall be constructed in the locations and in accordance with the Drawings.

Rip rap shall have a uniform appearance overall and shall not have noticeable overall irregularities in horizontal and vertical alignments. Individual rocks shall be hard, durable and clean and should be free from cracks, cleavage planes, joints, seams, chemical alteration or weathering which would result in the breakdown of the rock in a riverine environment.

The overall thickness of rip rap installed shall not be less than twice the D(50) of the rock.

#### 9.1.2 Submittals

At least fourteen (14) days prior to the supply of any rock, the Contractor shall provide documentation to demonstrate that the rock to be supplied complies with the requirements of this Specification. The documentation to be provided shall include but not necessarily be limited to:

- details of the quarry from which the rock is to be supplied, including identification of the sections of the quarry where rock complies with the requirements of this Specification;
- a test report from a NATA registered Independent Testing Authority on the physical and chemical properties of the rock to be supplied. The report shall include the results of a petrographic analysis and laboratory testing of the rock;
- details of the Contractor's Quality Assurance System, including details of the quality control procedures, which ensures that the quality of rock supplied during the Contract complies with this Specification.

Should the Contractor wish to vary the source of rock supplied either within the quarry or by changing quarries, the Contractor shall submit additional documentation (as listed above) to demonstrate that the rock from the new source complies with the requirements of this Specification.

#### 9.1.3 Acceptance of Type D Riprap

Acceptance of rock for the primary and secondary armour layers and riprap material shall satisfy all of the following criteria:

rock shall be rough and angular;

- the ratio of the maximum dimension of any rock to the minimum dimension, measured at right angles to the maximum dimension shall not exceed 2.5;
- rock shall be essentially unweathered without significant quantities of deleterious minerals such as analcime and expansive clay minerals or unfavourable lineations within the microfabric;
- igneous rock shall have not more than 15% (by volume) olivine and shall exhibit no zones of secondary alteration such as chloritisation;
- rock shall show no signs of stress-relief;
- the characteristics listed in the above below:

Dry density:	2,650 kg/m <sup>3</sup> minimum
Saturated point load strength index (is <sub>50</sub> ):	5.0 MPa minimum
Los Angeles abrasion value:	30% maximum
Sodium sulfate weight loss:	12% maximum

 The design drawings note the D<sub>50</sub> for required for the rock, the rock shall also meet the following properties:

D <sub>50</sub> (mm) as specified in the drawings	Minimum D <sub>100</sub> (mm)	Minimum D <sub>10</sub> (mm)
300	400	100
550	750	300

#### 9.1.4 Construction

Rip rap shall be placed in a manner which ensures that the larger rocks are uniformly distributed throughout the protection works and that the smaller rocks effectively fill the spaces between the large rocks without leaving any large voids. The layers of placed rock shall be of even thickness and of even grading.

The placing operations shall minimise the chances of rock running loose and damaging adjacent areas. Rock deposited in areas outside the rock protection zone shall be recovered.

### 9.2 TURF REINFORCED MATTING

#### 9.2.1 General

The following types of Turf Reinforced Matting (TRM) have been specified on the drawings:

1) Enkamat 7018 with Greenarmour

#### 9.2.2 Submittals

1) Certification:

a) The Contractor shall provide the Principals Representative a certificate stating the name of the TRM manufacturer, product name, style, chemical compositions of filaments or yarns and other pertinent information to fully describe the geotextile.

b) The Manufacturer is responsible for establishing and maintaining a quality control program to assure compliance with the requirements of the specification. Documentation describing the quality control program shall be made available upon request.

c) The manufacturer's certificate shall state that the furnished TRM meets MARV requirements of the specification as evaluated under the manufacturer's quality control program. The certificate shall be attested to by a person having legal authority to bind the Manufacturer.

- 2) Manufacturing Quality Control (MQC) test results shall be provided upon request.
- 3) Independent Performance Test Results shall be provided upon request.

#### 9.2.3 Delivery, Storage, And Handling

- 1) TRM labeling, shipment and storage shall follow ASTM D 4873.
- 2) Product labels shall clearly show the manufacturer or supplier name, style name, and roll number.
- 3) Each shipping document shall include a notation certifying that the material is in accordance with the manufacturer's certificate.
- 4) Each TRM roll shall be wrapped with a material that will protect the geotextile from damage due to shipment, water, sunlight, and contaminants.
- 5) The protective wrapping shall be maintained during periods of shipment and storage.
- 6) During storage, TRM rolls shall be elevated off the ground and adequately covered to protect them from the following: Site construction damage, extended exposure to ultraviolet (UV) radiation, precipitation, chemicals that are strong acids or strong bases, flames, sparks, temperatures in excess of 71 deg C (160 deg F)m and any other environmental condition that might damage the TRM.

#### 9.2.4 Quality Assurance Sampling, Testing, And Acceptance

- 1) TRM shall be subject to sampling and testing to verify conformance with this specification. Sampling for testing shall be in accordance with ASTM D 4354.
- Acceptance shall be in accordance with ASTM D 4759 based on testing of either conformance samples obtained using Procedure A of ASTM D 4354, or based on manufacturer's certifications and testing of quality control samples obtained using Procedure B of ASTM D 4354.
- Quality Assurance Sampling and Testing will be waived for ISO 9001:2000 Certified Manufacturing Facilities. Documentation of ISO 9001:2000 Certification shall be provided upon request.

#### 9.2.5 Installation

#### 9.2.5.Enkamat 7018 with Greenarmour:

- 1) Installation shall be as per the manufacturers guidelines and the design drawings.
- 2) Before installation the slope shall have a smooth profile free from vegetation, roots and stones.
- 3) Overlaps shall be a minimum of 150mm and shall be secured at 1metre centres.

- 4) Topsoil and seeding shall be as per Section 8.
- 5) Product Details:

Table 9.1 – Enkamat 7018 with Greenarmour

Property	Test Method	Units	Property Requirement
Thickness	ASTM D-6525	mm	18
Mass Per Unit Area	ASTM D-6566	G/sq m	290
Tensile Strength	ASTM D-6818	kN/m	2.0 x 1.2

Tensile Strength	1/20,000
Tensile Elongation	1/20,000
Light Penetration (% Passing)	1/20,000

### 9.3 Reno Mattress

#### 9.3.1 General

The following reno mattresses have been reviewed and approved for use:

Maccaferri Castoro 230mm thick PVC coated mesh rock mattress

#### 9.3.2 Mattress

Reno mattresses shall be "Castoro" type manufactured from double tweisted, hecagonally woven wire mesh of nominal 60×80 mesh, with 2.4mm o/d frame wire and 2.0mm o/d mesh wire, complete with diaphragms at 1m centres.

Diaphragms shall consist of two layers of mesh having the base of the Mattress and diaphragms manufactured from one continuous mesh panel.

All wire shall be mild steel Galmec coated (95% Zn + 5% Al Mischmetal) and heat bonded through extrusion with an additional 0.5mm heavy duty grey PVC coating.

#### 9.3.3 Rock Fill

The filling rock to be used in gabion baskets and mattresses shall be produced from dense, hard durable and clean rock. The rock shall resist weathering actions of air and water, it shall not be cracked or have visible defects which may reduce ists structural capabilities.

Rock with a density of 2400kg/m3 or higher are preferable. The wet dry strength of the aggregate shall be 100kN or greater

There are two types of rock fill specified depending on the required velocity:

Table 9.5 - Tlend Malless Hockini			
Туре	Size (mm)	D50 (m)	Critical Velocity
Type I	70 – 100	0.085	3.6

#### Table 9.3 – Reno Mattress Rockfill

Туре	Size (mm)	D50 (m)	Critical Velocity
Type II	70 – 150	0.120	4.5

The contractor shall provide details of the quarry from which the rock is to be supplied including identification of the sections of the quarry where rock complies with the requirements of this specification.

The contractor shall also supply a test report from a NATA registered independent Testing authority on the physical and chemical properties of the rock to be supplied. The report shall include the results of a perrogrpahic analysis and laboratory testing of the rock.

#### 9.3.4 Installation

The reno mattress shall be installed as per the manufacturers requirements. The longest side of the reno mattress shall be installed perpendicular to the flow direction. The mattress shall be fixed as per the manufacturers requirements.

# PART C: ENGINEERING SPECIFICATION FOR CONCRETE AND STRUCTURES

# **1 CONCRETE**

### 1.1 **GENERAL REQUIREMENTS**

- 1) Concrete, the constituent materials of the concrete and all workmanship associated with the supply and placing of the concrete shall comply with the following Australian Standards and any additional requirements of this Specification.
  - AS 1012 Methods of Testing Concrete.
  - AS 1141 Methods for Sampling and Testing of Aggregates.
  - AS 1170 Loading Code Parts 1 and 2.
  - AS 1379 The Specification and Supply of Concrete.
  - AS 1478 Chemical Admixtures for Concrete.
  - AS 1527 Two-Part Polysulphide-Based Sealing Compounds.
  - AS 1554 Structural Steel Welding, Part 3 Welding Of Reinforcing Steel.
  - AS 2327 Composite Construction in Structural Steel and Concrete.
  - AS 2758.1 Aggregates and Rock For Engineering Purposes -
  - Concrete Aggregates.
  - AS 3582.1 Supplementary Cementations Materials for Use with Portland and Blended Cement Fly Ash.
  - AS 3600 Concrete Structures Code.
  - AS 3610 Formwork for Concrete.
  - AS 3735 Concrete Structures for Retaining Liquids.
  - AS 3972 Portland and Blended Cements.
  - AS 4671 Steel Reinforcing Materials

ASTM D1752,

60T Type II Resin Bonded Cork Fillers.

- ASTM C309 Liquid Membrane Forming Compounds for Curing Concrete.
- ACI 403 Guide for Use of Epoxy Compounds with Concrete.

BS 2571 Specification for Flexible PVC Compounds.

- 2) No concrete shall be mixed on site for use in the Works unless the Principals Representative or the designer site representative (DSR) has approved the batching and mixing equipment. No ready mixed concrete shall be supplied for use in the Works unless the Principals Representative and DSR has approved the source of supply.
- 3) The Contractor shall submit for approval of the Principals Representative and the DSR, prior to the supply of any concrete, details of all materials to be used including their respective sources and the concrete mix proportions <u>HOLD POINT</u>. Unless approved by the DSR, the Contractor shall not make any subsequent changes to the materials or to the mixes.
- 4) These approvals of equipment, source of supply, material and mix proportions shall not constitute approval of the quality of the concrete.

### 1.2 COMPOSITION OF CONCRETE

- 1) Concrete shall be made with cement, coarse aggregate, fine aggregate and water.
- 2) Cement shall be "Type GP" General-purpose cement conforming to the requirements of AS 3972, Portland and blended cement. The minimum quantity of cement used in one cubic metres of concrete shall be 400 kilograms. The maximum water:cement ratio shall not exceed 0.45 by mass unless otherwise specified on the Drawings.

- 3) Cement shall be supplied from an approved TMR registered Cementitious Materials Supplier. Consignment notes indicating the source of supply of cement, date of manufacture, company test number, type of cement, silo number and date of dispatch shall be forwarded to the DSR for all cement supplies.
- 4) Calcium chloride shall not be used in manufacture of concrete. An approved airentraining admixture shall be used in the concrete. The amount to be added shall be determined by the Contractor and approved by the DSR. Any further admixtures shall not be used except with the approval of the DSR.
- 5) Aggregates shall meet the requirements of AS 2758.1 except that in relation to Clause 9.1 of AS 2758.1, the overall weighted average loss of the aggregate shall not exceed 10% for coarse aggregates and 8% for fine aggregates when tested in accordance with Part 24 of AS 1141 (Sodium Sulphate Soundness Test).
- 6) Samples of the aggregates proposed to be used shall be made available to the DSR for testing if required, prior to initial use and at regular intervals during the contract period. The maximum aggregate size shall not exceed 20mm.

### 1.3 **CONCRETE STRENGTH**

- 1) Concrete shall comply with the requirements for water cement ratio and strength as shown on the Drawings.
- 2) Compressive strength testing shall be the responsibility of the Contractor, in accordance with AS 1012 and the quality assessed as prescribed in AS 3600. Concrete failing to meet the requirements of this Code shall be rejected and replaced by the Contractor at no cost to the Principal.
- 3) Unless otherwise shown, the characteristic compressive strength of structural concrete shall not be less than 32 MPa at 28 days.
- 4) The frequency of compressive strength testing shall be as per MRTS 70.

### 1.4 **REINFORCEMENT**

- 1) The Contractor shall detail, supply, bend and accurately place all steel reinforcement shown on the Drawings.
- 2) All steel reinforcement shall comply with the following Australian Standards:

AS 1302	Steel Reinforcing Bars for Concrete.
AS 1303	Steel Reinforcing Wire for Concrete.
AS 1304	Welded Wire Reinforcing Fabric for Concrete.

- 3) Reinforcement supports shall be of non-corrodible material.
- 4) Unless otherwise shown, clear cover from external faces of in-situ concrete to nearest bar shall be 65 mm in the case of unformed surfaces in contact with earth and 50 mm for all other surfaces.
- 5) Unless otherwise shown, all splices, bends and hooks in reinforcement shall be in accordance with AS 3600.

### 1.5 **SLUMP**

The slump for each grade of concrete shall meet the requirements as specified in Table 11.4 of MRTS 70 and shall be within the tolerances given in AS 3600, viz. of  $\pm$  15 for 80 mm or  $\pm$  30 for greater than 80 mm.

### 1.6 MAXIMUM SIZE OF COARSE AGGREGATE

The maximum size of aggregate shall be 20mm.

### 1.7 PLACEMENT AND FINISH OF CONCRETE FOR STRUCTURES

- 1) All concrete structures shall be built in a workmanlike manner and to the lines, grades and dimensions shown on the Drawings.
- 2) Structures shall be located as indicated on the Drawings
- No concrete shall be placed until the steel reinforcement has been placed and the DSR has approved embedded metalwork, formwork and other surfaces against which the concrete is to be placed <u>HOLD POINT</u>.
- In addition to meeting the requirements of AS 3600 for concreting in extreme weather conditions, no concrete shall be placed when the temperature of the surrounding area is higher than 38°C or less than 5°C.
- 5) The placement of concrete around pipes shall be planned in a manner that will ensure that there is no possibility of the pipes being floated or otherwise moved out of position.
- 6) All concrete shall be thoroughly compacted so as to produce concrete having very low absorption and permeability.
- 7) Loads shall not be applied to concrete placed in permanent work without the prior approval of the DSR and the Principals Representative may at any time direct the Contractor to remove any loading imposed on concrete placed in permanent work.
- 8) Over-excavation under concrete shall be backfilled with concrete as specified in this specification.

Subject to each individual case being approved by the DSR, construction joints may be located at the discretion of the Contractor.

- 9) In the case of concrete surfaces forming the interior surfaces of structures or the exposed exterior surfaces of structures, the permissible variation from the specified lines shall not exceed a rate of 4 millimetres in 1 metre nor shall any dimension vary by more than 12 mm overall. All other concrete surfaces shall be formed so that the thickness of the finished concrete shall not be less than specified.
- 10) Unless otherwise shown in the Drawings, all corners shall be provided with 15mm × 15mm chamfers or fillets.

### 2.1 TRENCHING FOR CROSS DRAINAGE CULVERTS

The Contractor is required to construct cross drainage culverts at the locations shown on the drawings. The Contractor shall also extend the pipework of existing culverts and reinstall the sumps as necessary on these culverts where the raised levee extends over the existing works.

### PRECAST CONCRETE CULVERTS

#### 2.1.1 Pipe Culverts

All new concrete pipe culverts shall be rubber ring jointed in accordance with the design documentation and tested in accordance with AS/NZS 4058.

The supply, handling and installation of all pipe culverts shall be undertaken in accordance with the manufacturer's recommendations.

The bedding material shall be as specified on the drawings and compacted with hand tampers or other similar equipment approved by the DSGR in layer not more than 150mm thick after compaction.

The required degree of compaction shall be as specified in Annexure A of this Specification.

The bedding shall be formed so as to continuously support the pipes at the required levels.

The Contractor shall ensure that the full depth of bedding specified for the pipeline is provided under all the joints and that the joints are neither unsupported nor on points of concentrated load.

Where it is necessary to leave 'bell holes' or otherwise omit temporarily a section of pipe bedding during the making of a pipe joint, the bedding material specified for that section of the pipeline shall be placed and thoroughly compacted to make good the deficiency.

The cut-off trench for the levee foundation is to extend underneath pipe culverts through levees and is to be constructed prior to installing pipe culverts.

If holes are provided to assist in the lifting of precast concrete culverts, they shall be sealed with concrete or plastic plugs as per the manufacturers recommendations.

#### 2.1.2 Box Culverts

Box culverts shall be in accordance with the design documentation and AS 1597.2.

The supply, handling and installation of all box culverts shall be undertaken in accordance with the manufacturer's recommendations.

Box culvert joints shall be covered with a bandage cover in accordance with Clause 12.3.4 of MRTS03.

The box culverts are to be laid on cast in-situ base slabs in accordance with TMR Standard Drawings 1316 and 1318. The foundation for the box culverts and headwalls is to be stripped of topsoil and unsuitable material and to be treated as per the levee foundations in Section 4.4 above. From the foundation level to the underside of base slab and headwalls shall be filled with Type C Material. The required degree of compaction shall be as specified in Annexure A of this Specification.

The cut-off trench for the levee foundation is to extend underneath box culverts and is to be constructed prior to constructing the base slab.

### 2.2 BACKFILLING CULVERTS

#### 2.2.1 Pipe Culverts Under Road Embankments

All precast concrete pipes are to be laid to the grade specified in the drawings. For pipes which don't penetrate through or beneath the levee the bedding shall be "H2" support in accordance with the AS/NZS 3725.

#### 2.2.2 Pipe Culverts Through Levees

For all pipes which penetrate through the levee the bedding shall be on a minimum of 100mm for pipe diameters 600mm or less, 150mm for pipe diameters of 1800mm, of Type C non dispersive material (Section 2.4) in accordance with the drawings (bedding class U in accordance with the AS/NZS 3725).

The haunch zone shall be backfilled with an approved proprietary flowable fill product or Controlled Low Strength Material (CLSM).

Overlay material up to the top of the levee shall be Type C material. The required degree of compaction shall be as specified in Annexure A of this Specification.

#### 2.2.3 Box Culverts

Box culverts are to be backfilled with Type C non dispersive material (Section 1.4 above) as specified on the drawings. The required degree of compaction shall be as specified in Annexure A of this Specification.

### 2.3 CONCRETE CUTOFF WALLS

Concrete cut-off walls for all precast concrete drainage pipes shall be provided to the extent and details shown on the drawings. Cut off-walls shall be a minimum of 200mm thick grade N32 concrete.

Cut-off walls shall be founded on undisturbed natural material or where in fill, on fill compacted in accordance with requirements for Type C fill noted in this Specification and undisturbed after compaction by trenching or other excavation activity. All cut-off walls shall extent a minimum of 300mm below the bottom of the bedding material and 300mm beyond the side of pipe trenches into undisturbed ground.

### 2.4 **PENSTOCKS**

Penstocks are to be provided on all pipe penetrations through levees. Penstocks shall be Model 25S-T Stainless Steel Penstocks (open top frame) by Ludowici or an approved equivalent. Penstocks are to be installed as per the manufacturer's instructions and the design drawings.

### 2.5 **CONCRETE HEADWALLS**

Headwalls shall be constructed in accordance with the design documentation.

### 2.6 FLOWABLE FILL

Flowable fill or CLSM shall be supplied from a manufacturing plant which has third party certification to AS 9001 by a JAS-ANZ accredited assessment body. The Contractor must be able to provide:

- (a) the flowable fill or CLSM mix design, including:
- the source, type and proportions of the constituent materials;
- aggregate gradings and saturated surface-dry densities;
- chemical admixtures details and manufacturer's recommended method of use;
- the nominated slump and where a super-plasticizer is used, the final slump; and
- documentary evidence that the mix will comply with the specified requirements of the Contract under consideration, from either previous production of the mix (the test results shall not be more than 12 months old) or full details of a trial mix undertaken.
- (b) evidence, either through trial mixes (in accordance with AS 1012.2) or production testing, that the flowable fill or CLSM will comply with the requirements of this Specification.

#### 2.6.1 Materials

Cement shall comply with AS 3972.

Admixtures shall comply with AS 1478.1.

Fly Ash shall comply with AS 3582

Aggregates shall be free of reactive or expansive materials and be compatible with the CLSM flow characteristics.

The maximum size of coarse aggregates used shall not exceed the values as specified in Section 1.6

#### 2.6.2 Properties

The flowable fill or CLSM shall comply with the properties detailed below.

- (a) 28 day Strength 5MPa
- (b) Slump 150 200mm
- (c) Maximum size of coarse aggregate 10mm
- (d) Flowable fill or CLSM shall be homogeneous, free of lumps of unmixed material and without segregation.

#### 2.6.3 Placement

The method of placement shall be such as to ensure no foreign materials enter the mix.

Where CLSM is used as conduit and culvert backfill, the Contractor shall establish a placement process to ensure conduits and culverts will not float, or otherwise become dislodged, during placement of CLSM. CLSM shall be installed in accordance with AS 2566.2 Part 2: Appendix K.

The Contractor shall retain a copy of the delivery information specified in AS 1379: Clause 1.8. 3 "Identification Certificate"

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### 1.1 **GENERAL**

Unless shown otherwise on the Drawings or as specified in the Scope of Work; all earthworks, including subgrades; fill material; backfill; and pavements; shall be compacted to the requirements specified herein. Whereby there is a conflict of information, the order of preference for information will be the Contracts Administrator, Issued for Construction (IFC) drawings, Main Roads Specification and Annexure followed by this construction specification.

The Contractor shall provide and operate sufficient rollers for compaction of the work. Drawn-type rollers shall be provided with a tractor having sufficient weight and power to pull the roller at a satisfactory speed under actual working conditions. The rollers so provided, and their towing tractors, if any, shall be maintained in good mechanical condition. Equipment shall be approved before transport to site.

At locations where it would be impractical to use mobile power compacting equipment, materials shall be compacted by hand held or excavator mounted equipment that will achieve the specified compaction.

The formation shall receive a final shaping with a grading machine supplemented with handwork where necessary to ensure a smooth surface and uniform cross sections. When final shaping is complete, the finished surface shall conform to the line, grade and cross section shown on the drawings. There shall be no roots, sods, other deleterious matter or stones, which do not meet the requirements of this specification.

### 1.1.1 COMPACTION STANDARDS

Compaction testing shall be in accordance with AS 1289.5. The reference density for each type of material shall be established and monitored in accordance with AS 3798 section 7.3 and 7.4 and tested in accordance with AS 1289.5.4.1 or AS 1289.5.4.2 as applicable.

Where the Principal's Representative cannot test the material, owing to the presence of large stones or for other reasons, he may direct that the material be compacted by a minimum 8 passes of a smooth wheeled roller of minimum 10 tonne static weight, or other equipment considered necessary to achieve the required compaction. The Contractor shall carry out any additional work to achieve the specified compaction according to Section 16.5.

### 1.2 MOISTURE CONTENT

Compaction of cohesive materials shall be carried out with the moisture content of the material within the tolerance for optimum moisture content specified in Table 5. The correct uniform moisture content shall be obtained by watering or drying and mixing the material

### 1.3 NUCLEAR DENSITY GAUGES

Nuclear density gauges may be used for field density testing. Testing shall comply with AS 1289 method 5.8.1, or with the relevant AASHTO or ASTM Standard, or approved equivalent.

In addition to the required standard operational checks and calibration, at least three initial field density results shall be compared to densities obtained by sand replacement
or other approved methods to AS 1289 or AASHTO, ASTM, or approved equivalent standard, for the same test locations.

If corresponding results differ by more than 1%, the Principal's Representative may direct that use of the nuclear gauge be discontinued. The Principal's Representative reserves the right to request further density correlation checks if the gauge, the operator, or the fill material is changed during the course of the work, or for any other reason.

All test results shall be reported in accordance with the approved testing standard.

#### 1.4 **COMPACTION REQUIREMENTS**

#### **1.4.1 Cohesive Materials**

Unless otherwise specified or shown on the drawings, cohesive materials shall be compacted to achieve relative compaction, as determined by AS 1289.5.1.1 for Standard Compaction Method and AS1289.5.2.1 for Modified Compaction Method, of not less than as specified in Table 5.

Material	Tolerance on moisture content relative to Optimum Moisture Content (OMC)	Relative Compaction and Method
Sub-grade in excavations and trenches	+1.0%, -2.0%	98 standard
Sub-grade for fill and backfill	+1.0%, -1.0%	98 standard
Sub-grade for pavements and slabs	+1.0%, -1.0%	100 standard
Fill	+1.0%, -1.0%	100 standard
Backfill	+1.0%, -2.0%	98 standard
Top 300 mm of backfill in trench under pavement	+1.0%, -1.0%	98 modified
Pavement material	+1.0%, -1.0%	98 modified

Table 5 - Compactio	n Requirements f	or Cohesive Materials
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#### 1.4.2 Cohesionless Materials

Unless otherwise specified or shown on the drawings, cohesionless materials shall be compacted to achieve density indices, when determined in accordance with AS 1289.5.5.1 and AS 1289.5.6.1 using Standard Compaction, of not less than those stated in Table 6.

#### Table 6 - Compaction Requirements for Cohesionless Materials

Material	Density Index
Sub-grade in excavations and trenches	65
Sub-grade for fill and backfill	65
Sub-grade for pavements and slabs	70
Fill	70
Backfill	70

#### 1.5 COMPACTION TESTING AND ACCEPTANCE

The Contractor shall test all subgrades, fill, backfill and pavements.

The Contractor shall carry out testing for compliance with the requirements of this Specification using the acceptance criteria of Appendix 1. The Contractor shall not proceed with subsequent layers of construction until testing for compliance has been completed; the work has been inspected and written approval to proceed has been issued. The Contractor shall notify the Principal's Representative of any non-conformances within 24 hours of the results being available.

The Contractor's testing program shall be such that the lot sizes are not greater than the sizes given in Table 7 and the testing frequencies and number of tests per lot are not less than those specified hereinafter.

Construction Activity	Compaction Test* Required	Maximum Lot Size	Testing Frequency
Fill Area Surface	MDR	n/a	1 test per material type
	RC, DI	10,000M2	1 test per 2,000m <sup>2</sup>
			minimum of 2
Excavated	MDR	n/a	1 test per material type
Subgrade	RC, DI	5,000m2	1 test per 500m <sup>2</sup>
			minimum of 4
Embankments Below Subgrade	MDR	n/a	1 test per material type
	RC, DI	500m3	1 test per 50m <sup>3</sup>
			minimum of 4
Embankments at	MDR	n/a	1 test per material type
subgrade	RC, DI	5,000m2	1 test per 500m <sup>2</sup>
			minimum of 4
Pavements	MDR	2,500t	1 test per material type
	RC, DI	5,000m2	1 test per 500m <sup>2</sup>
			minimum of 4
* MDR - Moisture De	ensity Relationship		
RC, DI - Relative cor	npaction, density inc	dex	

#### Table 7 – Compaction Test Requirements

# 2 TOLERANCES

Earthworks and road works shall be finished to smooth and uniform surfaces which shall conform within the following tolerances to the lines, grades and cross-sections shown on the drawings.

#### 2.1 FORMATION LEVEL

Finished surfaces in fill shall not vary more than +50, -0 mm from the specified level. The surface shall not vary more than 25 mm over 3 m.

Finished surfaces formed by blasting shall not vary more than +0, -400 mm from the specified levels. Finished surfaces formed by ripping shall not vary more than +0, -150 mm from specified levels.

#### 2.2 **PAVEMENT SUBGRADE WIDTH**

The subgrade width shall not be less than the specified width or more than 150 mm greater than the specified width. The edges of the subgrade shall not vary more than 50 mm inside or 100 mm outside the specified position.

#### 2.3 FORMATION WIDTH

The widths measured on each side from the road centreline to the toes of cut batters and the tops of fill batters shall not be less than the widths specified, and no portion of cut batters shall encroach within these widths.

#### 2.4 BATTER SLOPES

At all levels, the average slope of the batters shall not be steeper than the slope specified, and no point on the completed batter shall vary from the average plane more than 100 mm in the case of cut batters in earth, or 150 mm in the case of cut batters in rock and in the case of fill batters.

#### 2.5 **BATTER LINE**

Unless approved otherwise, cut batters shall be constructed such that the top of the batter is not more than 150 mm outside the batter line shown on the drawings.

Fill batters shall be constructed such that the toe of the batter is not more than 150 mm outside the batter line shown on the drawings.

# **3 ROADWORKS**

#### 3.1 GENERAL

Over the whole area and distances shown on the Drawings or as specified in the Scope of Work, the hardstand areas and roads shall be formed, reformed, graded, boxed, trimmed and piped.

Excavation shall be made to specified depths and widths, and the table pipes, side pipes, Vee-pipes and catch pipes to the boundaries of the areas shall be constructed to the details shown on the drawings.

#### 3.2 **PREPARATION OF ROADBED**

The roadbed is that portion of the subgrade or existing formation upon which the pavement is to be constructed. Preparation of the roadbed shall include all required work to restore, correct or prepare the roadbed to a condition suitable for the construction of the specified pavement. It shall also include the trimming and compaction of the shoulder formation. Immediately prior to placing subsequent material thereon, the finished roadbed and the shoulder formation shall be compacted as specified in Section 1.4. The surface of the roadbed and shoulder formation shall be smooth and uniform, true to the specified levels, grades and cross sections within the tolerance specified in Section 2.2 to the lines, grades and cross sections shown on the drawings.

Unless otherwise specified, existing pavements shall be scarified to a depth of not less than 75 mm before additional material is spread.

Before any pavement materials are placed on the compacted roadbed, the roadbed shall be tested and test rolled in accordance with Section 1.5.

#### 3.3 GRAVEL AND CRUSHED ROCK PAVEMENT CONSTRUCTION

#### 3.3.1 GENERAL

Gravel and crushed rock pavement construction includes the supply, spreading, compaction and trimming of crushed rock, gravel or sand, or mixtures thereof to the lines, grades, thicknesses and cross sections shown on the Drawings or as specified in the Scope of Work.

#### 3.3.2 MATERIALS

The Contractor shall be responsible to investigate and obtain the materials for pavement construction. The Contractor shall arrange for all material quality testing and shall submit details of the proposed materials to the Principal's Representative for approval prior to delivery to site.

Should the Contractor wish to construct the pavement using different materials then the Contractor shall submit the proposed materials to the Engineer for approval. Such submission shall be made sufficiently early so that the works programme is not delayed.

Plant production materials used shall comply with the requirements of MRTS 05 for the relevant categories.

Testing of materials shall be in accordance with AS 1141 and AS 1289.

The Contractor shall blend and process the pavement materials such that the grading of materials complies with MRTS 05 for the relevant classification.

Materials from borrow pits shall comply with MRTS 05 for relevant classification.

#### 3.3.2.1 Pavement Materials Naturally Occurring (Gravels and Composites)

The crushed limestone for the sub-base course shall be obtained from an approved source and shall be free from sand, roots and other foreign material.

Naturally occurring paving material gravels shall be selected and processed where necessary to confirm with the specified requirements. Processing shall include, where necessary, screening, separation, recombining, addition of other approved materials and any other operation necessary to produce a paving material to conform to the specified requirements.

The material for use in any course shall conform to the grading in Table 8.

#### Table 8

AS Metric Sieve mm	(%) Passing by Weight				
	1	2	3	4	
75.0	100			100	
37.5	80-100	100		80-100	
19	50-80	70-100	100	60-100	
9.5	35-65	50-80	70-100	50-95	
4.75	25-50	35-65	50-80	40-80	
2.36	15-40	25-50	35-65	30-65	
0.425	7-20	10-30	15-35	20-50	
0.075	3-13	4-16	6-20	5-25	

#### Grading Requirements – Naturally Occurring Pavement Material

#### Table 9

#### Grading Requirements – Naturally Occurring Pavement Material

Properties	Maximum values
Liquid Limit	25
Plasticity Index (P.I.)	6
Linear Shrinkage	3
Base course CBR (4 days soak)	80
Sub-base CBR (4 days soak)	50

The above table is based on aggregate of uniform specific gravity and the Engineer may require that appropriate corrections be applied to the percentage of the test sample passing the various sieves when aggregates of varying specific gravities are used.

#### 3.3.2.2 Crushed Rock Base

The whole of the material supplied shall be crushed from freshly quarried stone of an approved source and consist of hard durable rock free from sand, organic matter or other deleterious matter.

The pavement material shall have a nominal size of 20 mm and when compacted in the pavement and shall comply with the following requirements.

AS Sieve Size (mm)	% Passing by Weight
19.0	100
9.5	70-80
4.75	45-65
2.36	30-50
0.425	12-30
0.075	3-12

The ratio of the portion passing 0.075mm sieve to the portion passing 0.425mm sieve shall fall within the range 0.4 to 0.6.

1. Properties	2. Maximu m values
3. Liquid Limit	4. 25
5. Plasticity Index (P	P.I.) 6. 5
7. Linear Shrinkage	8. 3
9. Base course CBR days soak)	10. 90
11. LA Abrasion test	12. 40

The Los Angeles Abrasion Loss shall not exceed 40%.

#### 3.3.3 SPREADING

Where pavement material is to be spread on a prepared base other than existing pavement, the first layer shall be spread and compacted immediately after the subgrade has been approved, in order to minimise the entry of water, or drying out, or deformation of the subgrade.

The pavement material shall be spread in even and equal layers to the width shown on the drawings, at such a rate that after compaction the final thickness shall not be less than that specified. The maximum thickness of layers before compaction shall be 200 mm.

Each layer shall be spread and thoroughly compacted before the next layer is spread, and the Contractor shall keep the lower layer in good condition during the spreading of the succeeding layer. No layer of pavement material shall be spread until the previous layer has been tested by the Contractor for conformance to this specification and approval obtained.

Care shall be taken to prevent segregation of pavement. Where segregation does occur, the materials shall be remixed and respread to the approval of the Principal's Representative.

#### 3.3.4 COMPACTION OF PAVEMENT

Following inspection and acceptance of the sub-grade the sub-base shall be placed in a single layer and in such a manner that the sub-grade is not disturbed.

The sub-base shall be watered and mixed, so that the moisture content does not exceed optimum, graded and rolled to achieve the specified shape and density. Each layer of material shall be fully compacted in accordance with Section 16 of this specification.

Compaction shall begin at the sides of the pavement and progress gradually to the centre. Each pass of the roller shall be parallel with the centreline of the roadway and uniformly over-lap each preceding pass. On sections of pavement with one-way cross-fall, compaction shall begin at the low side of the pavement and progress gradually to the high side. Any deficiencies in level shall be made good as compaction proceeds. Where directed the compacted or partially compacted materials shall be scarified after level correction to ensure adequate bonding.

Compaction shall commence immediately after spreading and before existing moisture in the material has evaporated. Where necessary, the Contractor shall water the material to assist in compaction. All material spread each day shall be compacted sufficiently to provide a dense surface to prevent the entry of excessive amounts of water. Any unstable areas that develop shall be removed, replaced and re- compacted before further layers are added.

#### 3.3.5 PAVEMENT COURSE TOLERANCES

All pavement courses, consisting of one or more layers of the same material, shall be finished to smooth and uniform surfaces and shall conform to the lines, grades, thicknesses and cross section shown on the drawings, within the following tolerances:

Level - The top of each pavement course shall not vary from the specified level by more than 12 mm. Where pavements are constructed against a kerb and channel or pipeage pit concrete apron the edge of the pavement shall be constructed flush with the edge of the channel unless otherwise specified or shown on the Drawings or as specified in the Scope of Work.

Thickness - The thickness of the base course of the pavement shall not be less than that specified or shown on the drawings. The total thickness of the pavement shall not be less than the specified thickness by more than 12 mm.

Shape - No point on the finished surface shall vary more than 10 mm from a 3 m straight edge laid parallel or perpendicular to the centreline of the roadway

# PART E: SPECIFICATION FOR MONITORING AND TESTING

#### 1.1 MONITORING

Prior to the end of the Defects Liability Period, an inspection of the entire levee system should be undertaken by the Designer and Principals Representative. From consideration of the significance that the role the levee and diversion system plays in protecting the community of Roma, it is recommended that the Principal arrange for a maintenance strategy to be developed to ensure adequate performance of the levee system. It is recommended that this strategy comprises development of an operations manual in combination with regular inspections and maintenance.

Regular yearly inspections are recommended to be carried out by experienced personnel and cover all associated works and identification of any issues that may affect the serviceability or performance of the levee system (burrow holes, trees, scouring, piping, seepage, weed build up etc.). Particular attention should be paid to drainage structures, valves etc. and associated earthworks, to ensure the system is functioning correctly. The documented results from the annual inspection can then be used as an input for a maintenance program, which will facilitate timely levee maintenance.

Maintenance operations to be undertaken include repair of batter slumps, revegetation, mowing, and removal of trees, filling of burrow holes and depressions and crest maintenance. Particular monitoring and maintenance of the surface protection system immediately following construction is particularly important, at a time when a thick vegetative mattress has not fully established. Replanting and other associated maintenance will likely be required following the initial wet season. Crest maintenance is recommended to maintain design levee height and to prevent development of ruts and potholes, which avoids water ponding and potential development of pipes.

Grazing of vegetation on the levees or diversions is not recommended as this will impact on the coverage of vegetation.

It is recommended that during exposure of the works to a flood event (and particularly during first exposure), the Principal arrange for the following monitoring inspections to be conducted by appropriately qualified personnel:

- 1. Daily inspections of town side foundations and batters to detect potential failure mechanisms (for example seepage, boiling, piping and heaving)
- 2. Inspection of downstream culvert inlets/ outlets to detect potential failure mechanisms (scouring, seepage, boiling, piping and heaving)

It is recommended that relevant details are recorded and photographed for use in the development of remediation and/ or maintenance plans.

Following receding of the flood event, a detailed inspection and condition assessment is recommended to be undertaken by appropriately qualified personnel.

Remediation of scoured areas will be required to be undertaken by the Principal, particularly if flooding occurs prior to full establishment of vegetation.

#### 1.2 **TURF MAINTENANCE**

The Contractor shall maintain all grassed areas for the whole of the Contract Period and the Defects Liability Period.

Such maintenance shall include:

- Irrigation
- Mowing to encourage a spreading habit of growth. Generally grass shall be kept at a height of less than 150 mm
- Such practices or work as are necessary to eliminate pests, disease or weed infestations
- Fertilising

Levee grasses should be inspected for healthy growth, and treated for any deficiency in topsoil or chemical imbalance. (This assumes that sufficiently regular watering for enhancing growth has been maintained).

All water used for watering of seed as appropriate shall be obtained from approved sources and be free of chemicals or compounds toxic to plant growth.

#### 2.1 GENERAL

- All materials, equipment, installation and workmanship included in the works of this Contract, shall be tested and inspected to prove compliance with the Contract requirements. An approved alternative to testing to the specification and relevant codes will be the furnishing of a certificate of compliance with the specification or code.
- 2) Tests and inspections, unless otherwise specified or accepted shall be in accordance with the following order of precedence:
  - a) This specification
  - b) Relevant TMR standard
  - c) Relevant Australian Standards
  - d) If no AS standards are applicable, then those of the British Standards Institution shall be used).
- 3) The Contractor shall at all times give to the Principals Representative a minimum of one day's prior notice of all tests and inspections.
- 4) Specific test requirements for the various items of work and equipment are covered in the relevant sections of this Specification and design drawings.
- 5) At all times when tests are in progress the Contractor shall have at least one suitably qualified and NATA or equivalent approved representative present.
- 6) All test results shall be suitably signed by a suitably qualified and NATA or equivalent approved representative, and submitted to the Principals Representative within 5 days of the completion of the testing.

#### 2.2 EQUIPMENT FOR TESTING

- The Contractor shall carry out all tests nominated in the Specification and the design drawings and all labour, materials, temporary works and equipment necessary for the tests shall be provided by and at the expense of the Contractor. It will also be the Contractor's responsibility to make any necessary temporary connections in order properly to carry out all specified testing procedures.
- NATA or other approved authority shall certify measuring and metering equipment. The Contractor at his expense shall provide all electrical power, compressed air or chemicals required for testing purposes.

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## **1 GENERAL**

The Contractor shall develop and implement a Project Environmental Management Plan (PEMP) which shall comply with the Environmental Management Systems Guidelines as outlined in the Preliminaries and the following sections of this Specification. It is noted that this may include development of a Review of Environmental Factors (REF) for the project site to ensure all relevant legislative and other requirements are identified by the construction team.

#### 1.1 **IMPLEMENTATION**

The Principals Representative is responsible for:

- Ensuring that the Contractor's PEMP meets the requirements of the Contract Documents.
- Auditing the Contractor's implementation of the PEMP and for issuing corrective action requests to the Construction Contractor as appropriate.

The Construction Contractor is responsible for preparation of a PEMP, which states the Contractor's policy and methods of implementation for:

- Compliance with all relevant local, state and Commonwealth environmental legislation, guidelines, permits and licences and industry codes of practice.
- Design of temporary environmental measures to mitigate any unfavourable environmental impacts.
- Construction, operation, maintenance and monitoring of all environmental measures during construction and the relevant defects liability period.
- Reporting and correcting non-conformances during construction and the relevant defects liability period.
- Correcting matters raised by the Principals Representative on corrective action requests.

The PEMP must be submitted by the Contractor and approved by the Principals Representative at least seven (7) days prior to the commencement of construction works.

The Principals Representative will be responsible for periodic monitoring and auditing, reporting the results and issuing Corrective Action Requests (CAR).

CAR will specify the non-conformance and require the person/organisation responsible to state the corrective action being taken and its time of completion and, in addition will require a statement of preventive actions to ensure that similar non-conformances do not occur.

#### 1.2 **REPORTING**

An Environmental Management File (EMF) is to be maintained by the Construction Contractor and should contain all documentation pertaining to environmental management of the project. The EMF should take the form of a traditional correspondence file and/or are readily retrievable, upgradeable and suitably protected computer file. The Construction Contractor shall undertake weekly inspections of the construction area to identify non-conformance with construction phase actions. A report providing details of these inspections is to be maintained on the EMF.

The Contractor shall prepare a monthly report including a description of monitoring activities, non-conformance notices and corrective action notices (both completed and pending). A monthly report shall be made available to Principals Representative for review and comment.

During construction, the Construction Contractor's Environmental Representative will make daily visual inspections of the site and any discharges from the site.

The Construction Contractor and any Subcontractors shall report any non-conformance with the PEMP to the Principals Representative. Reporting shall be done immediately after the non-conformance has occurred.

The Construction Contractor is responsible for carrying out preventative action and corrective action resulting from a non-conformance.

#### 1.3 ABORIGINAL ISSUE MONITORING

The Contractor shall develop the Project Environmental Management Plan (PEMP) and detail the provisions and requirements for Aboriginal Project monitoring during construction within this document.

The provision of all works necessary for monitoring for Aboriginal significance at the site is the responsibility of the Contractor.

#### 1.4 TRAINING, AWARENESS, COMPETENCE

All activities on the project which have a potential to cause environmental harm will be identified and personnel competent to carry them out without causing environmental harm will be selected or, alternatively, others will be given specific training and assessed for competency.

All staff will be trained and assessed in environmental responsibilities.

It will also be emphasized that compliance with the PEMP does not remove the responsibility of compliance with the law.

#### 1.5 **SAMPLING, ANALYSIS, MEASUREMENTS**

The Manuals published by the QLD Environment Protection Authority will be utilised where they apply.

Laboratories will be selected for their certification by NATA or other accrediting body. Samples will be taken and measurements made by persons with the necessary training and competence.

Control and calibration of measuring equipment will be as set out in the equipment manuals, the EPA manuals or Australian Standards as appropriate. Records of calibrations will be kept.

#### 1.6 STANDARDS, LEGISLATION, GUIDELINES

Compliance with the PEMP shall be made a condition of acceptance of the contract to work on the site by any Contractor or Subcontractor.

The Construction Contractor will plan and execute the construction to prevent or minimise environmental harm and in accordance with best practice environmental management as required by the relevant Queensland Legislation.

The following minimum standards apply to monitoring and auditing of performance. Subject to development of the CEMP by the contractor, additional standards may be required to be adopted.

Item	Regulation
Water	Australian Water Quality Guidelines for Fresh and Marine Waters – ANZECC. Approved Methods for the Sampling and Analysis of Water Pollutants in QLD
Soils	ANZECC/NHMRC – Guidelines for the Assessment and Management of Contaminated Sites.
Air	AS3580 Methods of Sampling and Analysis of Ambient Air.
Noise and Vibration	<ul> <li>QLD EPA Noise Manual</li> <li>AS1055.1 and AS1055.2 Acoustics - Description and Management of Environmental Noise.</li> <li>AS2436 Guide to Noise Control on Construction, Maintenance and Demolition Sites.</li> <li>AS2659.1 Guide to the Use of Sound Measuring Equipment.</li> <li>AS2659 Sound Level Meters.</li> </ul>
Flora and Fauna	Commonwealth Environment Protection and Biodiversity Conservation Act 1999 Nature Conservation Regulation 2006
Cultural Heritage	Aboriginal Cultural Heritage Act 2003
Contaminated Land	Contaminated Land Management Act 1997 Contaminated Land Management Regulations 1998
Dangerous Goods	<ul> <li>Dangerous Goods Act 1975</li> <li>Dangerous Goods (General) Regulation 1999</li> <li>Environmentally Hazardous Chemicals Act 1985</li> <li>Environmentally Hazardous Chemicals Regulation 1994</li> <li>AS1216 Classification, Hazard Identification and Information Systems for Dangerous Goods.</li> <li>AS1678 Emergency Procedure Guides - Transport</li> <li>AS1940 Storage and Handling of Flammable and Combustible Liquids.</li> <li>AS2508 Safe Storage and Handling Information Cards for Hazardous Materials.</li> <li>AS2809 Road Tank Vehicles for Dangerous Goods.</li> <li>AS2931 Selection and Use of Emergency Procedure Guides for Transport of Dangerous Goods.</li> </ul>

Item	Regulation
Soil Erosion and Sediment Control	<ul> <li>BCC guidelines for erosion and sedimentation control</li> <li>`Urban Erosion Sediment Control' (Department of Conservation and Land Management, 1992).</li> <li>Soil Erosion and Sediment Control – Engineering Guidelines for Construction Sites - Institution of Engineers 1996.</li> <li>Best Practice erosion and Sediment Control (IECA) guidelines 2008</li> </ul>

#### 1.7 **INTERPRETATION**

- Principal means the Maranoa Regional Council
- Principals Representative means the person authorized by the Principal, to act with the full authority of the Principal. For the purposes of this Specification, the Principals Representative is Ross Drabble of Maranoa Regional Council.
- Defects Liability Period means the period during which the Construction Contractor is responsible for making good defects in a part or the whole of any works. The Defects Liability Period shall also include routine maintenance items.
- The contractor means 'Ostwald Brothers'
- Routine Maintenance comprises:
  - Cleaning signs.
  - Litter collection.
  - Repair of Collateral Road Damage.
  - Repair Eroded Areas.
  - Maintenance of revegetated areas (i.e. watering, weed control etc.)
  - o Maintenance of temporary fencing
  - Maintenance of environmental mitigation devices
- Project Environmental Management Plan (PEMP) means a detailed program of works and measures documented and carried out by the Construction Contractor in accordance with the relevant provisions of the Contract and relevant government and legislative requirements.

#### 1.8 **PEMP ELEMENTS**

The following elements are considered the minimum requirements for the PEMP.

# 1.8.1 Erosion Control, Stormwater & River Water Quality Management & Monitoring

The Contractor shall determine the control measures appropriate to the site of the works. Prior to the commencement of the project works, the Contractor shall prepare an Erosion and Sediment Control Plan in consultation with the Maranoa Regional Council and the Department of Environment and Resource Management for the project.

Management measures shall incorporate the following items:

• Local stormwater runoff from the undisturbed area upstream of the site should be diverted away from the construction site and discharged to the existing open drain via erosion stabilised locations.

- Filtering devices, silt and sediment traps and other devices shall be utilised to prevent sediment, turbidity and other pollutants which result from the works being discharged into the Balonne River and Bungil Creek.
- Minimise the alteration of the existing drainage patterns in undisturbed areas.
- Drainage from construction sites shall be passed through sediment traps in the stormwater drainage. Examples of sediment traps include hay bales, geo-textile fabric filter fences and silt traps. Construction of temporary sediment fences and catch drains to be undertaken in accordance with standard guidelines.
- The extent of disturbance shall be the minimum required for construction activity or that provided in the permit, whichever is the lesser. This can be achieved by construction planning and undertaking the works in stages followed by prompt revegetation and rehabilitation of completed levee sections and the disturbed areas under the old levee sections.
- Identify and delineate no-go or limited access areas.
- Revegetation of completed earth levees, borrow areas and all other disturbed areas should be undertaken immediately after completion of construction.
- The completed earth levees shall be vegetated with a hardy, erosion resistant grass as specified. The vegetation of the batters will include techniques appropriate to earth embankments.
- Topsoil stockpiles shall be located on flat ground at least 5 metres away from areas subject to run-off and away from established flowpaths (eg. drains, gutters, etc.). The height of the stockpiles shall not exceed 2 metres, unless stockpiles are suitably protected from wind erosion. The Contractor shall protect temporary topsoil spoil stockpiles with diversion drains, silt fences and straw bales to prevent sediment loss.
- The Contractor shall be required to clean any machinery which has operated or travelled through an area where declared plants are present in accordance with the guidelines under the Land Protection Act 2002 and to collect this wash down water to allow silt and seeds to settle. The silt and seeds will be buried under at least 1 metre cover. Any declared plants, which emerge in the settling basin, will be destroyed.
- Tracking of sediment from the construction site via construction equipment onto the road shall be minimised. The Contractor shall be required to clean any machinery in a designated washdown area to prevent tracking of sediment off site.
- Washout of concrete trucks and cleaning of equipment and/or vehicles used during construction shall not be undertaken in locations that permit flow of untreated wastewater directly to the open drainage system.
- Stormwater from the construction site shall be collected and treated via sediment control structures prior to discharge off site.
- The design of any works by the Contractor shall minimise obstruction to normal and flood flows in the creeks and shall provide for protection against scour by floods.
- Stores of oils, paints, fuel, etc. will be contained in accordance with AS1940 so that no contaminants pollute land or waterways.
- At completion of construction the area shall be cleared of potentially polluting materials.
- Inspections of the construction area and periodic observations of work practices shall be undertaken daily to identify any potential problems.
- A Water Quality Monitoring Program shall be undertaken throughout construction that ensures that any water discharged into Bungil Creek or the existing drainage system during the construction phase is of equal or better quality than that flowing in the river at the time. Sampling of local stormwater during and immediately after the event should be undertaken before it discharges from the site into Bungil Creek.

#### 1.8.2 Traffic and Air Quality

The Contractor is responsible for ensuring that construction traffic and activities result in minimum impact on air quality.

The Contractor shall utilise best management practices and polices to minimise dust from construction including:

- Comply with the relevant Queensland Act.
- The contractor shall ensure that dust in the atmosphere complies with Occupational Health and Safety requirements.
- Maintain a water truck on site to wet down working areas and haul routes as required.
- Access roads and earthworks, including spoil heaps and stockpiles of sand, shall be watered or covered as required to minimise dust emissions. The use of waste oil for dust suppression will be strictly prohibited.
- Ensure that all vehicles and equipment are fitted with appropriate exhaust control measures and are adequately maintained in line with manufacturer's requirements.
- Enforce the covering of trucks transporting earth and fill materials where dust could be generated.
- Sealed public roads where trucks leave the construction site are to be checked daily for mud/dust and cleaned as necessary.
- Where practical, a stable surface will be provided for main haul routes in the construction area and maintained to minimise dust. When necessary, the Contractor shall water these surfaces on Sundays and public holidays as required by weather conditions.
- Appropriate speed limits over unsurfaced roads shall be established and enforced.
- Burning of any waste arising from construction activities is prohibited.

#### **1.8.3** Noise and Vibration Management

The contractors shall ensure that noise and vibration on the site are within acceptable limits as set out in the relevant QLD legislation and guidelines.

The contractor shall utilize best practices and policies to minimize noise from construction including:

- All construction plant is to be fitted with current best practice noise control and attenuation devices and maintained and operated to ensure that noise emissions are minimized.
- Permission is to be obtained from the Principals Representative for any work outside the standard hours and the conditions in the permit will be observed.
- Blasting is not expected to be required at the site.
- Noise and vibration levels are to be measured by the Construction Contractor, as required by the Principals Representative.
- Occupational Health and Safety requirements for noise and vibration are to be met and appropriate Codes of Practice followed.
- Ensure vehicles and construction equipment is maintained within service guidelines to minimise noise emissions from malfunctioning equipment.

#### **1.8.4 Flora and Fauna Management**

The Contractor shall ensure that construction activities are carried out in a manner that minimises the impact on flora and fauna.

The Contractor shall undertake activities including:

- Implementation of the revegetation principles as described in the Specification.
- Minimise area disturbed by construction activity and use temporary fencing to delineate construction areas from protected areas. The extent of disturbance will be the minimum required for the construction activity or that prescribed in the permit whichever is the lesser.
- Contractor shall advise the Principals Representative of any areas the Contractor proposes to clear and to mark the boundaries of clearing.
- Minimising the destruction of flora and interference with fauna.
- Declared plants and exotic flora and fauna shall not be introduced to the site and any that are accidentally introduced will be destroyed, using approved non residual herbicides applied by a licensed operator. Use herbicides as necessary (ie Round -Up) as necessary to control weed growth on site.
- Protected species of plants shall not be removed or interfered with, without the approval of the Department of National Parks, Recreation, Sport and Racing.
- The Specification will require the Contractor to advise the Principals Representative of any areas the Contractor proposes to clear and to mark the boundaries of clearing.
- The Contractor will be required to clean any machinery which has operated or travelled through an area where declared plants are present in accordance with the guidelines under the Land Protection Act and to collect this wash down water to allow silt and seeds to settle.
- The Construction Contractor will be required to prevent injury to, or death of, any native fauna on the construction site and to transport any injured fauna to an animal hospital or refuge.

#### 1.8.5 Dangerous Goods and Waste Management

- Any on site washrooms and toilets shall be provided with waste disposal.
- Waste oil to be collected in drums and transported off site for recycling where possible.
- Petroleum products to be stored, handled, separated and signed as required by the AS1940 Storage and Handling of Flammable and Combustible Liquids.
- Refuelling shall be carried out over contained area to prevent contamination of land from spillage.
- Waste oils shall be collected and transported to recycle depots or designated disposal sites. Major maintenance and repairs shall be carried out offsite.
- Dangerous goods shall be stored and handled on bunded impervious floors and separated and signed as required by the appropriate Australian Standards.
- The Contractor shall prepare an Emergency Response Plan and train employees in the use of equipment, chemicals, and protective clothing and the application of the Emergency Response Plan.
- Any spills of dangerous goods shall be rendered harmless and collected for treatment and disposal at a designated site. This includes cleaning materials, absorbents and contaminated soils.

#### **1.8.6 Road Transport and Sourcing of Materials**

- The Construction Contractor shall provide a Traffic Management Strategy and comply with the Traffic Management Plan in the project Specification.
- When suitable alternatives are available, the soil materials and manufactured goods causing minimum adverse environmental effects shall be assessed and used in construction.
- Permission shall be obtained from the relevant Traffic Authority and Council for transportation of any loads exceeding dimension and weight limits and the Construction Contractor complies with the conditions of the permits.

- The Contractor shall coordinate transport to ensure minimum damage to public and other access roads along the haulage routes.
- The Contractor shall utilise materials sourced locally wherever possible to limit the use of public roads for long distance hauling of bulk construction materials.
- The Contractor shall ensure that the transportation of dangerous goods is in accordance with the regulations and the relevant codes and standards published by Standards Australia.
- The Contractor shall ensure that a regular program of street cleaning within the main road transport routes and associated works areas is undertaken.
- Washout of concrete trucks and cleaning of equipment and/or vehicles used during the road construction shall not be undertaken in locations that permit flow of untreated wastewater directly to the open drainage system.

#### **1.8.7** Restoration and Rehabilitation of Disturbed Areas

- The Contractor shall restore all disturbed areas that result from construction activities. The disturbed areas shall be restored and returned to similar configurations and vegetation conditions to those that existed prior to the commencement of construction.
- The Contractor shall remove all structures used in the construction work that are not a permanent feature from the site together with all wastes.
- The Contractor shall repair all damage to existing roads, drainage and other infrastructure, as may have occurred during the construction work.
- Local drainage systems shall be constructed as per the Drawings and disturbed areas outside the works shall be reinstated to minimise erosion and to avoid changes in the overland flow patterns.
- The Contractor is responsible for rehabilitation maintenance until this responsibility is taken over, by prior arrangement with the Principal, or after the specified Defects Liability Period has expired.

#### 1.8.8 Aboriginal And Historical Archaeology And Monitoring

#### *i* Contractors Requirements

The contractor shall identify all requirements relating to Aboriginal and Historical Archaeology and Monitoring and implement all measures required to satisfy the relevant Authorities.

# **APPENDIX A – SPECIFICATION TABLES**

Appendix A Specification Tables			
Table No	Details		
Table A1	Material Classification Table		
Table A2	Placement and Compaction requirements for Earthworks		
Table A3	Testing and Acceptance of Earthworks Materials		
Table A4	Acceptable tolerances for Earthworks Levels and Position		
Table A5	Testing and acceptance criteria for Placement and Compaction of Earthworks		
Table A6	Schedule of Hold Points		

#### Table A1: Material Classification Table

MATERIAL CLASSIFICATION TABLE									
Classification	ation TYPE A TYPE B Topsoil General Fill		'PE B eral Fill	TYPE C Structural fill		TYPE D Rip rap			
Particle Size Grading Requirements (% passing by mass)	Sieve Size (mm)	Criteria (1)	Sieve Size (mm)	Criteria (1)	Sieve Size (mm)	Criteria (1)	Sieve Size (mm)	Criteria (1)	
		N/A	75 0.075	100 20%	25 0.075	100 20%	150mm min o 500mm maxi dimensio	limension mum m	
Min Plasticity Index	N/A		10 (2)		10 (2)	10 (2)		N/A	
Min/ Max Liquid Limit	N/A		20/65 (2)		20/65 (2)		N/A		
Emerson Class Number (ECN)	N/A		-		>3		N/A		
Acceptable material classification	N/A		SC, CL, CI, CH		SC, CL, CI, CH		N/A		
Other		Min UCS=10 MPa Max wet dry strength variation = 20%							

(1) Criteria = % passing nominated sieve by mass

(2) Refer to table below for acceptable range of values as shown on Atterberg plot

Table A2: Placement and compaction requirements for earthworks

	TEST METHOD	ACCEPTANCE CRITERIA			
TEST		TYPE A TOPSOIL	TYPE B GENERAL FILL	TYPE C STRUCTURAL FILL	TYPE D RIP RAP
Maximum Compacted Layer thickness (mm)	Inspection	N/A	300	300	(3)
Moisture Content	AS 1289.2.1.1 AS 1289.2.1.4	N/A	OMC(1)+-2%	OMC(1)+-2%	
Compaction	AS 1289.5.1.1 AS 1289.5.8.1	N/A	Min 98% (Standard)	Min 98% (Standard)	

- 1. As determined by AS1289.5.1.1 and AS1289.5.4.1
- 2. Proposed maximum lift to be submitted to DSGR for review and approval, depending upon type of machinery proposed for use
- 3. Layer thickness a function of max particle dimension (to be determined on site)

Table A3a: Testing and acceptance requirements for earthworks materials

CLAUSE REF	FILL TYPE	PROPERTY	TEST PROCEDURE	MIN TEST FREQUENCY	ACCEPTANCE LIMITS
Clause 3	TYPE A TOPSOIL	To Be Advised			
	TYPE B EARTH FILL	Particle Size Distribution	AS 1289.3.6.1	Min 1 test per 1000m3. Min 1 test per lot. (1)	Refer to Table A1 for all Acceptance limits for Properties noted in this Table
		Plasticity Index	AS1289.3.1.2 AS1289.3.2.1 AS1289.3.3.1	Min 1 test per 1000m3. Min 1 test per lot. (1)	
	TYPE C STRUCTURAL FILL	Particle Size Distribution	AS 1289.3.6.1	Min 1 test per 1000m3. Min 1 test per lot. (1)	
		Plasticity Index	AS1289.3.1.2 AS1289.3.2.1 AS1289.3.3.1	Min 1 test per 1000m3. Min 1 test per lot. (1)	
		Emerson Class Number	AS 1289.3.8.1	Min 1 test per 1000m3. Min 1 test per lot. (1)	
	TYPE D RIP RAP	Particle Size Distribution	AS 1289.3.6.1	1 test per material (quarried material)	
		Min Is50	TO BE DISCUSSED	1 test per material (quarried material)	
		Wet dry Strength variation	Q205C	1 test per material (quarried material)	

1. Lot size in layered fill placement can be greater than the volume listed above provided the number of tests per volume is achieved and subject to the entire fill from the lot being placed in a single day and being placed in a single layer.

Table A3b: Testing and acceptance requirements for ground surface treatments of foundations.

CLAUSE REF	FOUNDATION AREA	FOUNDATION MATERIAL	PROPERTY	TEST PROCEDURE	MIN TEST FREQUENCY	ACCEPTANCE LIMITS	PROCEDURE
Cl 3.8, 4.2 4.3, 5.0 & 6.0	Foundation Floor	Clay	Moisture content	Proof Roll	Single sufficient pass of trench width	No visible deformation or springing	Following approval scarify surface to >50mm depth and moisture condition prior to placement of fill material
		Sand	Moisture content	Proof Roll	Single sufficient pass of trench width	No visible deformation or springing	Following approval scarify surface to >50mm depth and moisture condition prior to placement of fill material

Table A4: Summary: Acceptable Tolerances for Earthworks Levels and Positions

EARTHWORKS LEVELS AND POSITIONS- ACCEPTABLE TOLERANCES			
POSITION	ACCEPTABLE TOLERANCE		
Foundation surface prior to filling	-100mm/ +0mm		
Levee finished surface	-0 mm / +100 mm		
All earthworks for roads and around culverts/ structures	As per TMR specifications		

Table A5: Acceptance Criteria for Earthworks Placement and Compaction

CLAUSE REF	FILL TYPE	PROPERTY	TEST PROCEDURE	MIN TEST AND REPORTING FREQUENCY	ACCEPTANCE LIMITS
	TYPE A TOPSOIL				
	<b>TYPE B</b> GENERAL FILL	Maximum Dry Density	_	Min 1 test per 400m3. (1)	
		Field Density	Refer Table A2	Min 1 test per 400m3. (1)	
		Moisture content		Min 1 test per 400m3. (1)	
		Layer thickness	Measurement (2)		
	TYPE C STRUCTURAL	Maximum Dry Density		Min 1 test per 400m3. (1)	
	FILL	Field Density	Refer Table A2	Min 1 test per 400m3. (1)	
		Moisture content		Min 1 test per 400m3. (1)	
		Layer thickness	Measurement (2)		
		Emerson Class	Refer Table A2	Min 1 test per 400m3. (1)	
	TYPE D RIP RAP	To be determined			

(1) Lot size in layered fill placement can be greater than the volume listed above provided the number of tests per volume is achieved and subject to the entire fill from the lot being placed in a single day and being placed in a single layer.

(2) Measurement with respect to items with level tolerance in accordance with Table A4. Contractor Quality Team shall sign off conformance of individual layer thicknesses.

Table A6: Schedule of Hold Points – Part B: Engineering Specification for Levee Earthworks Structures and Drainage

CLAUSE	HOLD POINT				
REF	PROCESS HELD	SUBMISSION DETAILS	RELEASE DETAILS		
Clause 1.3	Mixing of materials	Notification to the DSGR 5 working days before commencement of treating material.	Approval or rejection by DSR/DSGR		
Clause 1.5	Alternative material to be used to specification	Submittal to the DSR/DSGR details of an alternative to rock rip rap	Approval or rejection by DSR/DSGR		
Clause 2.10	Examining of material in borrow area	Notification to DSGR that excavation is proposed at a new Borrow Location, site investigation to be undertaken by DSGR.	Approval or rejection by DSGR.		
Clause 3.1	Inspection of foundation	Notification to DSGR of new excavation	Inspection of foundation by DSGR prior to initial filling. Witness of proof rolling.		
Clause 3.1	Groundwater and/or Seepage found	Notification to DSGR if groundwater and/or seepage is observed in foundation	Inspection by DSGR		
Clause 3.2 and 3.4	Presentation of levee and culvert foundation	Notification to DSGR that excavation to foundation level has been attained.	Inspection of foundation by DSGR prior to initial filling. Witness of Proof rolling. Selection of treatment (if required) and direction of removal of unsuitable material and/or further action as required. If unsuitable is removed, DSGR shall inspect stripped foundation prior to further filling operations.		
Clause 3.3	Replacement of each lot of unsuitable material	Notification to DSGR that unsuitable material has been removed as directed.	Inspection by DSGR Further removal of unsuitable may by directed prior to authorising release of Hold Point.		

CLAUSE	HOLD POINT				
REF	PROCESS HELD	SUBMISSION DETAILS	RELEASE DETAILS		
Clause 4.1	Delivery of imported material	Submittal to the DSGR details of imported fill and reason for use on the project.	Approval or rejection by DSGR		
Clause 5.0	Material conformance	Submittal to DSGR of conformity of each lot of Material placed, with relevant test and survey reports	Approval or rejection of placed material as ongoing responsibility of Contactor Quality Team followed by submission of results to DSGR		
Clause 5.0	Testing and acceptance of Earthworks	Submittal of all quality/ verification records and tests. Submittal to be issued as packages (per design lot). Submittal to include description of origin/ location of lot.	Approval or rejection by Principal		

Table A7: Schedule of Hold Points – Part C: Engineering Specification for Concrete and Structures

CLAUSE	HOLD POINT				
REF	PROCESS HELD	SUBMISSION DETAILS	RELEASE DETAILS		
1.1	Selection of appropriate concrete for cast in situ structures	Submittal to DSR details of concrete mix proportions and, details of materials and their sources	Approval or rejection		
1.7	Inspection of reinforcement for cast in situ structures	Notification to DSR that reinforcing requires inspection prior to concreting	Inspection by DSR of formwork and reinforcement		

**APPENDIX C – GEOTECHNICAL FACTUAL REPORT** 



# ROMA FLOOD MITIGATION PROJECT

# **Geotechnical Investigation Factual Report**

June 2014



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### **1** INTRODUCTION

#### 1.1 Background

The Roma township is located in the Bungil Creek floodplain and has an extensive history of flooding, with a number of major flood events in the recent years resulting in significant damage to private property, public infrastructure and utilities. Three significant large flood events have occurred between March 2010 and February 2012, with the February 2012 flood being the largest flood event ever recorded in Roma and resulting in significant damage to over 500 premises.

SMEC Australia Pty Ltd. (SMEC) has been commissioned by Ostwald Brothers Pty Ltd (OB) to prepare the detailed design of the Stage 1 Roma Flood Levee, for Maranoa Regional Council (MRC).

SMEC has carried out a geotechnical investigation to provide ground information for the detailed design of the proposed earthworks and structures and for potential nearby borrow areas to identify suitable materials for the levee earthworks.

For the purpose of design works and field investigation, the levee alignment has been divided in four separable sections (i.e. Section A to Section D). This report revision presents the factual data collected for Section A to Section D and available information for the borrow areas undertaken to this date.

#### **1.2** Report Scope and Structure

The scope of works covered in this report includes:

- A summary of the regional geology of the site;
- Details and results of the field investigation for Section A to Section D;
- Investigated potential borrow areas;
- Pavement test pits;
- Compiling of all test pit logs and boreholes logs;
- Compiling of all laboratory test results.

Explanatory notes and commonly used abbreviations specific to geotechnical logs of boreholes and excavations are presented in Appendix A.

An overall plan showing the locations of geotechnical investigations and borrow pits is presented in Appendix B.



# **2** FIELD INVESTIGATIONS

#### 2.1 Objectives

Test locations for this investigation were chosen to allow a geotechnical assessment of the ground conditions below proposed earthworks, structures, and in borrow areas. Final locations were also dependent on site access restrictions regarding property boundaries and land ownership.

The objectives of the investigation were:

- Identification of key potential geotechnical risks to the project;
- Characterisation of subsurface conditions;
- Provision of foundation parameters for detailed design of the proposed earthworks and structures;
- Identification and assessment of the suitability of materials to be excavated along the proposed alignment and potential borrow areas for possible use in levee construction;
- Identification of design constraints and potential issues relating to dispersive soils and erodibility;
- Provision of permeability/seepage soil parameters of foundation materials beneath and within the proposed levee structures.

#### 2.2 Previous Investigations

Previous ground information has been provided in GHD Report - Roma Flood Mitigation Project – Geotechnical Factual Report, dated March 2013.

This report includes the findings of 13 boreholes (BH1 to BH13) drilled to approximately 5.5m depth. These boreholes are generally located between residential Roma and Bungil Creek, however, not specifically along the currently proposed levee alignment.

#### 2.3 Investigation Activities

The investigation aimed to examine the subsurface ground conditions beneath the levee foundations. It should be noted at this point that the shape and levee type changed at several points during the investigation. The final shape of the levee is a 1:4 earth bund with a three (3m) wide crest from the start to finish however due to community preferences the scope changed in certain areas to allow for possible alternative alignments and alternative structures.

The investigation scope, with test locations presented in Appendix B and summarised in Table 1, generally comprised of:

- Test pits to a depth of three metres at approximately 200m centres (TP3 to TP32) and Dynamic Cone Penetrometer tests (DCP1 to DCP110) at approximately 50m centres along the proposed levee alignment footprint (including a DCP test adjacent to each test pit);
- 4 boreholes (BH14 to BH17) drilled to depths of between 9 and 12 metres and installation of standpipe piezometers to provide deeper ground information along the levee alignment;
- 3 further boreholes (BH21 to BH23) drilled to a depth of approximately 10m along one section of the levee alignment where a vertical wall structure was proposed in lieu of the earth bund structure over a length of approximately 100m. Boreholes were located at 50m spacing along the proposed structure.
- Installation of standpipe piezometers in each of the 4 levee alignment boreholes (BH14 to BH17) and in one at the concrete wall boreholes at BH22;



- 5 in-situ falling head and/or rising head permeability tests in each of the standpipe piezometers;
- 9 test pits to a depth of 6m at proposed borrow areas within the present Roma landfill site;
- Additional test pits to a depth of 3m at proposed borrow areas adjacent to the levee alignment on private properties.

The bulk of the fieldwork was carried out over ten days from the 24<sup>th</sup> of September to 3<sup>rd</sup> of October 2013 under the supervision of experienced geotechnical professionals from SMEC. The investigations were carried out in general accordance with SMEC environmental, health and safety management procedures and AS1726-1993 "Geotechnical Site Investigations".

Due to access constraints at the time of investigation, access was not possible from CH00 to CH1350 adjacent the Roma airport site and through properties adjacent to the Carnarvon Highway. Additionally, various alignment test pits were relocated due to property access constraints.

The works carried out during this period comprised:

- 106 test pits excavated both along the proposed levee alignment and at potential borrow areas:
  - 23 test pits (TP03-TP06, TP8-TP17, TP19, TP20, TP22-TP28) along the proposed levee alignment;
  - 15 test pits (TPB1-TPB9, TPB66-TPB69 and TPB74-TPB75) throughout the proposed landfill borrow areas and;
  - 32 test pits (TPB10-TPB16, TPB29-TPB41 and TPB58-TPB69) at the locations of proposed borrow areas along the levee alignment;
  - 14 test pits at Barron Borrow Area (TPB17-TPB28 and TPB54-TPB55);
  - 3 test pits at Racecourse Horse Pool (TPB42-TPB44);
  - 9 test pits at Potential Borrow Area at Peak's property (TPB45-TPB53);
  - 4 test pits at the Proposed western diversion channel (TPB70-TPB73) and;
  - 6 pavement test pits at proposed tie in points of the road crossings over the levee.
- 10 boreholes drilled to approximately 10m depth at the selected locations along proposed levee alignment:
  - 4 alignment boreholes where there was an absence of existing deeper ground information from the previous investigation (GHD March 2013);
  - 3 boreholes at 50m chainages at the location of one of the proposed wall sections south of McPhie Street at approximate CH2900 to CH3000 and;
  - 3 boreholes at 50m chainages at the location of one of the proposed walls at approximate CH750 to CH850.
- 97 Dynamic Cone Penetrometer tests (DCP) were carried out from CH1350 to CH5150 by South Queensland Soils, Roma;
- Installation of standpipe piezometers in six boreholes and;
- In situ permeability testing carried out in the six standpipe piezometers.

Test pit and borehole locations are summarised in Table 1.


Table 1 – Test Pit and Borehole Locations

Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD			
		Test Pits – Le	vee Alignment					
TP03/DCP01	Alignment Section D	00	677064.8	7063662.4	304.2			
TP04/DCP05	Alignment Section D	200	677171.5	7063350.4	302.7			
TP05/DCP09	Alignment Section D	400	677220.55	7063350.1	302.5			
TP06/DCP13	Alignment Section D	600	677409.5	7063286.0	302.6			
TP08/DCP20	Alignment Section D	1000	677725.1	7063278.6	302.50			
TP09/DCP25	Alignment Section D	1200	677796.3	7063175.1	303.82			
TP10/DCP29	Alignment Section D	1400	677879.3	7062988	302.32			
TP11/DCP33	Alignment Section C	1600	678076.6	7062915	301.05			
TP12/DCP33	Alignment Section C	1800	678268.2	7062884	301.47			
TP13	Alignment Section C	2100	678563.4	7062930.9	301.2			
TP14	Alignment Section C	2280	678742.6	7062959.5	301.4			
TP15/DCP49	Alignment Section C	2400	678861.5	7062956	301.27			
TP16/DCP53	Alignment Section B/C	2600	679049.6	7062890	300.39			
TP17/DCP57	Alignment Section B	2800	679101.6	7062709	300.92			
TP19	Alignment Section B	3180	679083.2	7062342.1	300.0			
TP20/DCP69	Alignment Section B	3400	679132.7	7062128	299.89			
TP22/DCP77	Alignment Section A	3800	679218	7061737	299.88			
TP23/DCP81	Alignment Section A	4000	679270.7	7061545	299.04			
TP24/DCP85	Alignment Section A	4200	679241.1	7061348	299.05			
TP25/DCP89	Alignment Section A	4400	679192.7	7061154	297.45			
TP26/DCP93	Alignment Section A	4600	679136.1	7060962	297.49			
TP27/DCP97	Alignment Section A	4800	679109.6	7060765	297.28			



Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD		
TP28/DCP101	Alignment Section A	5000	679153.3	7060573	297.12		
TP29/DCP105	Alignment Section D	940	677652.1	7063324	301.155		
TP30/DCP106	Alignment Section D	980	677660.29	7063278	303.002		
TP31/DCP107	Alignment Section D	1020	677666.82	7063239	303.53		
TP32/DCP109	Alignment Section D	1060	677695.82	7063207	303.49		
	Test Pit	s – Proposed	Landfill Borrow	Areas			
TPB01	Landfill – North Side	-	679427.8	7062423	300.23		
TPB02	Landfill – North Side	-	679897.1	7062392	313.20		
TPB03	Landfill – North Side	-	680286.9	7062332	318.61		
TPB04	Landfill – North Side	-	679659.9	7062295	299.44		
TPB05	Landfill – North Side	-	680056.4	7062234	313.86		
TPB06	Landfill – Northwest Corner	-	679739.1	7062195	302.40		
TPB07	Landfill – Northeast Corner	-	680240.8	7062095	312.44		
TPB08	Landfill – Northwest Corner	-	679823.5	7062074	305.71		
TPB09	Landfill – Southwest Corner	-	679840.5	7061655	301.81		
	Test	: Pits – Alignn	nent Borrow Area	as			
TPB10	Alignment Section C	1630	678031.1	7062797.3	300.60		
TPB11	Alignment Section C	1500	678060.4	7062993.2	301.20		
TPB12	Alignment Section C	1500	677950.6	7062930.9	301.30		
TPB13	Alignment Section A	4600	679251.7	7060870.0	297.80		
TPB14	Alignment Section A	4350	679259.9	7061191.0	297.90		
TPB15	Alignment Section B	3500	679417.5	7062078.0	299.60		



Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD		
TPB16	Alignment Section B	2050	678560.8	7063094.1	301.7		
Test Pits -	Proposed Borrow A	Area ID 5 (Bar	ron Borrow) – Cl	uster of TP approx	x. Ch 2600		
TPB17	East of Section B	2600	679700.6	7062784.2	302.5		
TPB18	East of Section B	2600	679698.3	7062762.7	302.1		
TPB19	East of Section B	2600	679696.8	7062735.5	301.9		
TPB20	East of Section B	2600	679734.9	7062785.5	302.6		
TPB21	East of Section B	2600	679735.4	7062760.8	302.6		
TPB22	East of Section B	2600	679728.8	7062726	302.6		
TPB23	East of Section B	2600	679767.7	7062789.9	303.3		
TPB24	East of Section B	2600	679768.1	7062751.4	303.2		
TPB25	East of Section B	2600	679771.7	7062714.6	303.6		
TPB26	East of Section B	2600	679798.2	7062789.9	304.1		
TPB27	East of Section B	2600	679811.8	7062756.5	303.6		
TPB28	East of Section B	2600	679819.1	7062730.7	303.7		
	Test Pits - Pro	oposed Borro	ow Area – Various	s Locations			
TPB29	West of Section C	1530	679819.1	7062730.7	303.7		
TPB30	West of Section C	1530	677940.0	7062760.7	300.9		
TPB31	West of Section C	1530	677934.0	7062789.3	300.9		
TPB33	West of Section D	1410	677967.6	7062790.2	299.5		
TPB34	East of Section C	1505	677895.5	7062940.2	301.9		
TPB36	East of Section C	1525	678052.1	7063041.4	301.2		
TPB37	East of Section C	1525	678070.2	7063016.1	301.2		
TPB38	East of Section B	2275	678911.988	7063046.991	301.333		
TPB39	East of Section B	2275	678897.169	7063121.187	301.489		
TPB40	East of Section B	2275	678902.121	7063096.366	301.499		
TPB41	East of Section B	2275	678906.986	7063072.247	301.397		
	Test Pits - Pro	posed Borrov	w Area – Racecou	Irse Borrow			
TPB42	East of Section B	3675	678092.404	7062356.942	303.080		
TPB43	East of Section B	3675	678131.573	7062359.779	302.999		
TPB44	East of Section B	3675	678170.32	7062362	302.824		
	Test Pits - P	roposed Bori	row Area – Peaks	Property			
TPB45	NE of Section B	2600	679283.399	7062806.446	299.838		



Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD									
TPB46	NE of Section B	2600	679196.899	7062801.784	299.971									
TPB47	NE of Section B	2600	679171.985	7062926.63	300.372									
TPB48	NE of Section B	2600	679221.757	7062935.324	300.559									
TPB49	NE of Section B	2600	679307.623	7062925.097	300.782									
TPB50	NE of Section B	2600	679309.279	7062844.696	300.342									
TPB51	NE of Section B	2600	679313.119	7063062.371	300.921									
TPB52	NE of Section B	2600	679268.267	7063070.392	300.991									
TPB53	NE of Section B	2600	679181.816	7063069.064	300.758									
	Test Pits –	Proposed Ext	ension at Baron's	Borrow										
TPB54         East of TPB27         2700         679891.042         7062766.769         304.989														
TPB55         East of TPB27         2700         679964.332         7062771.437         305.7														
	Test Pits - Prop	osed Dam Ext	ension at Campb	ells Property										
TPB56	Western Side	Not Applicable	678569.264	7061469.252	298.517									
TPB57	Eastern Side	Not Applicable	678641.666	7061486.153	298.765									
	Test Pits - Pro	oposed Dam E	Extension Lavelle	's Property										
TPB58	North of Bassett Lane	3600	678917.392	7061849.195	298.797									
TPB59	North of Bassett Lane	3600	678917.468	7061887.564	298.78									
TPB60	North of Bassett Lane	3600	678959.925	7061893.249	298.944									
TPB61	North of Bassett Lane	3600	678970.062	7061831.483	299.039									
	Test Pits – P	roposed Borro	ow Area Hulbert'	s Property										
TPB62	East of Section B	3300	679235.975	7062244.498	299.269									
TPB63	East of Section B	3300	679314.094	7062260.951	299.452									
TPB64         East of Section B         3300         679184.522         7062229.417         300.269														
	Test Pit – Proposed Borrow Area Heilbron's Property													
TPB65	East of Section B	3200	679231.746	7062310.146	300.554									
Test	: Pits – Proposed Bo	orrow Area La	ndfill SW Cnr - Ex	ctension of TPB9	Site									



Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD
TPB66	East of Bungil Creek	3900	679841.289	7061714.238	302.427
TPB67	East of Bungil Creek	3900	679896.057	7061718.663	303.157
TPB68	East of Bungil Creek	3900	679885.762	7061686.003	302.712
TPB69	East of Bungil Creek	3900	679877.674	7061648.173	301.614
	Test Pits – Pro	posed Borrov	w Area at Wester	n Diversion	
TPB70	South of Bowen Street	-	679252.688	7059313.809	295.972
TPB71	South of Bowen St	-	679283.227	7059258.564	295.799
TPB72	South of Bowen St	-	679260.679	7059216.999	296.914
TPB73	South of Bowen St	-	679261.146	7059185.803	296.529
		Borrow Pit La	andfill NW End		
TPB74	East of Bungil Creek	3100	679810.652	7062123.589	305.628
TPB75	East of Bungil Creek	3100	679817.657	7062179.472	306.867
		Pavemen	nt Test Pits		
TPX01	Carnarvon Hwy – Western Shoulder	650	677483.897	7063387.895	302.769
ТРХО2	Carnarvon Hwy – Eastern Shoulder	650	677557.02	7063047.447	303.242
TPX04	McPhie St Crossing (Eastern End)	2840	679176.353	7062624.455	301.109
ТРХО6	Miscamble St Crossing – Eastern Side	4750	679179.382	7060810.796	297.863
TPX08	Miscamble St Crossing – Eastern End	4750	679275.378	7060809.143	298.195
ТРХО9	Miscamble St Crossing – Western End	4750	679040.312	7060807.014	297.833
	E	Boreholes - Le	evee Alignment		
BH14*	Alignment Section C	1450	677931	7062952	301.49



Test Pit /DCP	Location	Chainage (Approx.)	Easting (m)	Northing (m)	RL (m) AHD
BH15*	Alignment Section C	2150	678537.8	7062896	301.15
BH16*	Alignment Section B	3500	679125.9	7062017	299.78
BH17*	Alignment Section A	4200	679210.8	7061371	299.00
BH18*	Alignment Section D - Concrete Wall	750	677526.818	7063351.668	302.420
BH19	Section D - Concrete Wall	810	677560.166	7063357.903	301.476
BH20	Section D - Concrete Wall	850	677590.388	7063360.726	301.207
BH21	Section B - Concrete Wall	2910	679097.4	7062596	300.66
BH22*	Section B - Concrete Wall	2950	679105.6	7062557	300.48
BH23	Section B - Concrete Wall	3000	679112	7062507	300.43

\*Standpipe piezometer installed and falling head, in situ permeability test carried out on 3<sup>rd</sup> October 2013. Notes: TP18 was replaced with BH23 (wall borehole).

TP21 not excavated due to property access constraints at time of investigation.



# **3** SITE DESCRIPTION AND GEOLOGY

## 3.1 General

Roma is located in the Maranoa Shire within southern Queensland. The majority of the town is located on the flood plain to the western side of the Bungil Creek. At this location the creek flows in a general direction North to South with several meanders, especially to the southern side of Roma.

Aerial photograph interpretation suggests that the creek has been migrating towards the east over time. These changes have left several abandoned channels; the most evident abandoned channel is located south of the Warrego Highway. These abandoned channels may become preferential flow paths when the Bungil Creek overflows.

Additionally, smaller gullies have been observed to run within the township of Roma and towards the Bungil creek. These gullies run seasonally, generally in south-easterly direction.

Cretaceous age deeply weathered rocks outcrops at both sides of the Bungil creek alluvial plains, in the Hospital Hill, towards the west and along the eastern margin of the Bungil creek. The landforms produced by these rocks are generally gentle rolling slopes interrupted by creeks and gullies.

## 3.2 Regional Geology

The flood levee is proposed to be located within the Quaternary alluvium of the Bungil creek floodplains. The Quaternary alluvium comprises clay, silt and sands that have been transported by the creek. These suggest that lateral and vertical variability is expected.

Underlying the quaternary sediments, rocks from Duncaster Member of the Wallumbilla Formation are present. These rocks generally comprise deeply weathered carbonaceous age mudstone, siltstone and some glauconitic and calcareous shelly fossils of Cretaceous age.

North of Roma and further upstream, the Bungil Creek flows through rocks from the Bungil Formation. The Bungil Formation mainly comprises Glauconitic, labile to Quartzose, siltstone and mudstone of Lower Cretaceous age. Bungil Formation (Ky): is part of the Blythesdale Group and underlies the Doncaster Member. Figure 1 is an extract from the geological map of the project area.





Figure 1 – Geological Map of Roma, QLD area

# 3.3 Groundwater

Groundwater was not observed within any of the test pits excavated to approximately three metres depth along the alignment area and to a maximum depth of six metres in the potential landfill borrow areas.

Ground water was encountered in BH14, BH17, BH 18, BH21, BH22 and BH23 during drilling. Standing water levels were measured in the standpipe piezometers installed in BH14, BH15, BH16, BH17, and BH21 prior to carrying out the falling head permeability tests on 3<sup>rd</sup> October 2013. Table 2 below summarises ground water levels encountered during drilling and standing water level measured before in-situ permeability testing.

Table 2 – Summary of Groundwater Levels

вн	Estimated water level encountered during drilling (mBGL)	Date	Standing water level measured in piezometer (mBGL)	Date
BH14	7.45	02/10/13	7.93	03/10/13
BH15	Not observed	02/10/13	6.52	03/10/13
BH16	4.20	30/09/13	4.48	03/10/13
BH 17	5.70	30/09/13	6.09	03/10/13
BH18	8.9	6/11/13	8.9	8/11/13



вн	Estimated water level encountered during drilling (mBGL)	Date	Standing water level measured in piezometer (mBGL)	Date
BH19	Not observed	7/11/13	-	-
BH20	Not observed	7/11/13	-	-
BH21	7.10	01/10/13	Not measured	-
BH22	7.25	01/10/13	5.90	03/10/13
BH23	7.00	01/10/13	Not measured	-
BH07*	4.90	06/02/13	Not measured	-
BH10*	4.50	05/02/13	Not measured	-
BH13*	4.00	14/02/13	Not measured	-

\* Reported in previous investigation undertaken by GHD, March 2013.



# **4** SUBSURFACE CONDITIONS

The exploratory test pit and borehole logs provide a record of the subsurface conditions encountered during the investigation and are presented in Appendix C. Photographs showing the stratigraphy layers in the test pits and the pit arisings are provided in Appendix D. Photographs of samples retrieved from SPTs during drilling boreholes are also included in Appendix D.



# 5 IN-SITU TESTING

## 5.1 DCP Testing

In-situ testing consisted of Dynamic Cone Penetration (DCP) tests carried out along the proposed levee alignment at 50m intervals from CH1350 to CH5150. DCP tests were carried out by South Queensland Soils. Results are presented in Appendix E.

# 5.2 SPT Testing

Standard Penetration Tests (SPT) were carried out during the drilling of each of the boreholes. Retrieved SPT samples were logged and photographed by experienced geotechnical personnel. SPT results are presented on the boreholes logs in Appendix C along with photographs in Appendix D.

# 5.3 Falling/Rising Head Permeability Testing

Falling Head and/or Raising Head Permeability tests were carried out in four of the levee alignment boreholes, (i.e. BH14, BH15, BH16 and BH17) and in one of the three concrete wall boreholes (BH22).

Details of the standpipe piezometers installed in the five boreholes are included in Appendix C.

Results of the permeability tests carried out are summarised in Table 3, below:

The Rising head tests were carried out for the soil around the standing ground water table while the Falling Head test was carried out for the soil profile immediately above the ground water table. Assuming isotropic permeability in horizontal and vertical direction, (i.e.  $k_v = k_h$ ) analysis of the field results has produced a coefficient of permeability ranging from  $5.81 \times 10^{-6}$  to  $3.58 \times 10^{-7}$  m/s.

ВН	Test conducted	Date	Material type	Material response zone	Permeability, k (m/s)
	Rising head	03/10/13	Gravelly SAND	8.09-8.5	3.18x10 <sup>-6</sup>
BH14	Falling Head	03/10/13	Sandy CLAY/ Gravelly SAND	6.82-8.5	4.12x10 <sup>-6 (*)</sup>
BH15	Falling Head	03/10/13	SAND	4.71-6.8	2.33x10 <sup>-6</sup>
	Rising head	03/10/13	XW Mudstone (weathered to Silty CLAY)	4.84-5.00	3.58x10 <sup>-7</sup>
BH16	Falling Head	03/10/13	Clayey SAND/SAND/ XW Mudstone (weathered to Silty CLAY)	1.65-5.00	1.57x10 <sup>-6 (*)</sup>
BH 17	Falling Head	03/10/13	Clayey SAND/Silty SAND	4.76-7.0	3.72x10 <sup>-6 (*)</sup>
DUDD	Rising head	03/10/13	Clayey SAND	6.6-8.2	5.81x10 <sup>-6</sup>
БП22	Falling Head	03/10/13	Clayey SAND/Silty SAND	1.2-8.2	1.32x10 <sup>-6 (*)</sup>

Table 3 – Summary of in-situ Permeability Testing

\*) Denotes average permeability



### 6.1 General

A suite of laboratory testing was undertaken on soil samples recovered during the field investigation. Testing was carried out by NATA accredited laboratories located in Roma, Chinchilla, Toowoomba and Brisbane.

Laboratory testing comprised:

- 295 x Field Moisture Content;
- 313 x Particle Size Distribution;
- 53x Hydrometer;
- 294 x Atterberg Limits;
- 235 x Linear Shrinkage;
- 175 x Emerson Class Dispersion;
- 40 x Shrink Swell Index;
- 96 x Standard Compaction;
- 1 x Maximum/Minimum Density;
- 73 x Falling Head Permeability and
- 10 CBR (10 day soak)
- 3 Organic Content testing

The results of the testing is summarised in Table 4. Test certificates are provided in Appendix E.



## Table 4 – Results of Laboratory Testing Summarised

Location ID	Depth	Material Type		Atterberg Limits         Sieve Analysis % Passing           LL         PL %         PI %         LS %         9.5         4.75         2.36         0.6         0.425         0.075         0.002							Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content			
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
								Levee	Alignment F	Foundation	Section A										
TP22	0.3-0.5	Silty SAND	-	-	-	-	100	100	100	97	94	23	9.3	3.0	-	1.88	11.0	-	1.00E-08	-	
	0.7-1.0	Clayey SAND	29	20	9	4.5	100	100	100	99	96	33	20.7	7.9	-	1.82	12.0	-	2.00E-08	-	
	2.5-2.7	Clayey SAND	-	-	-	-	86	84	80	61	54	21	-	-	-	-	-	-	-	-	
	2.8-3.0	Sandy CLAY	-	-	-	-	93	91	87	72	64	28	-	16.5	-	-	-	-	-	-	
TP23	0.3-0.6	Clayey SAND	23	13	10	5.5	100	100	100	100	98	44	15.3	5.5	3	1.87	11.5	-	1.00E-10	-	
	0.7-0.9	Silty SAND	19	NP	NP	1.5	100	100	100	99	97	33	12.1	3.6	3	1.91	10.5	-	1.00E-09	-	
	0.9-1.2	Silty SAND	-	-	-	-	100	100	100	99	96	28	8.8	-	-	1.86	10.5	-	4.00E-10	-	
	1.4-1.6	Clayey SAND	31	19	12	6.5	100	100	100	100	98	44	25.3	8.8	2	1.80	14.0	-	2.00E-12	-	
	2.0-2.3	Silty SAND	-	-	-	-	100	100	100	100	97	33	-	-	-	-	-	-	-	-	
BH17	3.0-3.45	Clayey SAND	22	13	9	4	100	100	100	100	99	30	-	-	-	-	-	-	-	-	<u> </u>
	5.5-5.95	Clayey SAND	27	18	9	6.5	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
	7.0-7.45	Silty SAND	28	19	9	4	100	99	97	69	55	18	-								
TP24	0.4-0.6	Silty SAND	-	-	-	-	100	100	100	100	99	39	16.4	5.5	3	1.87	11.5	-	3.00E-10	-	
	1.1-1.3	Silty SAND	-	-	-	-	100	100	100	100	99	33	14.3	5.2	-	1.87	12.5	-	3.00E-12	-	
TDOF	2.3-2.6	Silty SAND	-	-	-	-	100	100	100	100	99	32	-	-	-	-	-	-	-	-	
1P25	0.2-0.4	SIITY CLAY	30	15	15	7.5	100	100	100	100	100	70	23.0	10.2	3	1.08	14.5	1.2	3.00E-08	-	
	1.5-1.7	Sandy CLAY	29	11	18	8.5	100	100	100	99	98	23	22	10.3	2	1.80	12.5	-	5.00E-12	-	
	1.6-2.0	Sandy CLAT	-	-	-	-	100	100	100	90	97	49	-	-	-	-	-	-	-	-	
	2.4-2.7	Silty SAND	_		_	_	100	100	100	100	98	40	24.1	11.8	_	1 79	13.0	_	5 00F-11	-	
TP26	1 5-1 8	Silty CLAY	39	16	23	12 5	100	100	100	99	98	72	-	11.0	1	1.75	15.5	21	6.00E-10	_	
TP27	0.4-0.6	Silty CLAY	42	17	25	12.5	100	100	100	99	99	80	33.7	12.2	2	1.69	14.5	2.4	2.00F-11	-	
TP28	1.5-1.7	Sandy CLAY	34	15	19	10	100	100	100	100	99	63	24.8	11.1	-	1.74	14.5	-	2.00E-11	-	
	2.0-2.3	Silty CLAY	-	-	-	-	100	100	100	100	100	68	22.8	13.3	2	1.75	14.5	-	3.00E-09	-	
	3.2-3.4	Silty SAND	-	-	-	-	100	100	100	100	100	43	-	12.1	-	-	-	-	-	-	
	1 1	•				I		Levee	Alignment I	Foundation	Section B			I	1				<u> </u>		
TP17	0.4-0.6	Sandy CLAY	40	15	25	11.0	100	100	100	99	98	47	-	12.5	3	1.70	13.0	1.4	2.00E-09	-	
	0.8-1.0	Silty CLAY	37	13	24	11.5							-	10.7	4	1.77	14.0	1.4	6.00E-10	-	
	1.4-1.6	Sandy CLAY	30	14	16	8.0	100	100	100	99	99	59	-	8.4	-	-	-	-	-	-	
	2.2-2.4	Silty SAND	25	NP	NP	4.5	100	100	100	100	100	31	16.6	6.3	-	-	-	-	-	-	
TP18*															-	-	-	-	-	-	
BH22	5.5-5.95	Clayey SAND	28	18	10	6	100	99	97	73	55	17	-	-	-	-	-	-	-	-	
	7.0-7.45	Clayey SAND	34	21	13	6.5	100	100	99	94	90	42	-	-	-	-	-	-	-	-	
BH23	7.0-7.45	Silty SAND	25	20	5	3.0	100	100	100	97	90	27	-	-	-	-	-	-	-	-	
TP19	0.3-0.6	Silty SAND	-	-			100	100	100	98	94	14	-	2.3	-	1.81	11.0	-	4.00E-06	-	<u> </u>
	1.5-1.8	Silty SAND	-	-			100	100	100	98	94	12	-	1.7	-	1.73	13.0	-	5.00E-06	-	<u> </u>
	2.3-2.8	Silty SAND	-	-			100	100	100	98	94	12	-	1.7	-	1.73	13.5	-	5.00E-06	-	ļ
TP20	0.4-0.6	Silty SAND	-	-			100	100	100	99	93	23	-	2.4	-	-	-	-	-	-	<b></b>
	1.4-1.6	Silty SAND	22	NP	NP	0.0	100	100	100	99	95	22	-	4.5	5	1.88	12.5	-	2.00E-06	-	<b></b>
	2.8-3.0	Silty SAND	-	-	-	-	100	100	100	96	86	23	-	-	-	-	-	-	-	-	<b></b>
BH16	1.0-1.45	Silty SAND	20	11	9	4	100	100	100	100	98	38	-	-	-	-	-	-	-	-	
	2.0-2.45	Silty SAND	23	NP -	NP	0.0	100	100	100	100	99	26	-	-	-	-	-	-	-	-	
	3.0-3.45	Clayey SAND	18	5	13	3	100	100	100	99	96	27	-	-	-	-	-	-	-	-	



Location ID	Depth	Material Type		Atter	berg Limits	;			Sieve /	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
TD31*	4.0-4.25	Clayey SAND	54	28	26	13	100	99	99	93	86	40	-				-				
1921								Levee	Alignment I	Foundation	Section C										+
TP10	0.3-0.6	Silty CLAY											-	10.8	2	1.65	17	_	4.00F-10	_	-
	0.7-1.0	Silty CLAY	43	19	24	14.0							-	11.6	4	1.73	16	-	4.00E-10	-	
	2.2-2.5	Sandy CLAY	34	16	18	9.0	100	100	100	99	99	52	-	8	-	-	-	-		-	
	2.7-3.0	Clayey SAND	29	21	8	5.5	100	100	100	100	99	39	-	9	-	-	-	-		-	
BH14	2.0-2.45	Sandy Clay	27	17	10	6.0	100	100	100	100	99	51	-	-	-	-	-	-		-	
	4.0-5.93	Sandy Clay	39	19	20	5.5	100	100	97	94	94	54	-	-	-	-	-	-		-	
	8.5-8.95	Gravelly SAND	-	-	-	-	98	93	86	32	16	2	-	-	-	-	-	-		-	
TP11	0.2-0.4	Silty CLAY	-	-	-	-	100	100	100	100	99	72	37.6	8.6	3	1.7	15	1.7	5.00E-11	-	
	0.8-1.0	Silty CLAY	30	18	12	11.5	-	-	-	-	-	-	-	8.6	-	-	-	-		-	
	1.2-1.5	Silty CLAY	31	16	15	9.0	-	-	-	-	-	-	-	8	-	1.79	13	-	4.00E-09	-	
	2.0-2.3	Silty CLAY	45	19	26	13.0	-	-	-	-	-	-	-	13.7	-	1.66	16	-	5.00E-09	-	
TP12	0.1-0.3	Silty SAND					100	100	100	100	99	41	-	2.3	-	-	-	-	-	-	
	0.3-0.6	Silty CLAY	42	15	27	12.0	-	-	-	-	-	-	-	10.7	3	1.67	18	-	6.00E-10	-	
	1.0-1.3	Silty CLAY	46	15	31	15.5	-	-	-	-	-	-	-	12.9	-	-	-	-		-	
	2.6-2.8	Clayey SAND	25	17	8	3.5	100	100	100	100	99	34	16.1	5.8	-	1.85	13.5	-	2.00E-09	-	
	2.9-3.2	Silty SAND					100	100	100	97	94	33	-		-		-	-		-	
TP13	0.4-0.6	Silty CLAY	28	14	14	9.0	-	-	-	-	-	-		9.9	2	1.72	15	2.0	1.00E-10	-	
	0.9-1.1	Silty CLAY	40	17	23	13.5	-	-	-	-	-	-		9.8	-		-	-		-	
704.4	1.3-1.5	Sandy CLAY	31	14	17	9.0	100	100	100	99	99	61	26.6	7.9	-	1.79	12	-	3.00E-09	-	
1914	0.3-0.5	SIITY SAND	14	NP 17	10 10	2.0	100	100	100	100	99	44	16.8	4.6	-	1.88	11	0.5	1.00E-07	-	
	1.2-1.5		35	1/	18	11.0 F F	100	100	100	100	00	27	10.0	8.8 7.2	-	1.74	14	-	6.00E-10	-	
	2.3-2.0	Clayey SAIND	24 40	20	4	5.5 12 E	100	100	100	100	98	57	18.8	1.2	-	-	-	-		-	
1612	0.5-0.5	Silty CLAV	40	10	24	12.5	-	-	-	-	-	-	-	12 7	-	- 1 70	17	-	2 OOE 10	-	+
	1.4-1.0	Silty CLAT	20	1/	15	11 5	-	-	-	-	-	-	-	23.7 22	-	1.70	17	-	2.001-10	-	
	2 2-2 4		25	14	12	60	100	100	100	100	98	37	18.8	6.9	_	1.86	12 5		6.00F-08		
TP16	0.2-0.4	Sandy CLAY	20	10	18	12.0	100	100	100	100	90	60	16.8	5.4	2	1.00	14	1 9	5.00E-08		
1110	1 3-1 5	Silty CLAY	37	14	23	11.0	100	100	100	100	55	00	10.0	10.8	-	1.70	14 5	-	7 00F-10	-	
	1.0 1.0	0												2010		2.7.2	2.10			-	1
									Alignment F	Foundation	Section D							1	1 1		
TD02	0204	Sandy CLAV	45	1 Г	20	16	100	100	100	06		71	47	17.2	2	1.64	17.0	25	1 005 10		
1903	0.2-0.4	Sandy CLAY	45 E2	15	30	10	100	100	100	90	94	/1	47	12.3	2	1.04	17.0	2.5	1.00E-10	-	+
	0.4-0.8	Sandy CLAY	20	14	39	12 5	100	100	100	90	90	60	-	15.4	Ζ	-	-	-	-	-	
	1.5-1.0	Sandy CLAY	59	13	20	13.5	100	100	100	99	90 00	70	-	17.0	-	-	-	-	-	-	+
TD04	2.3-2.7	Sandy CLAT	55	15	40	18.0	100	100	100	99	96	75	- 62	17.9	- 2	- 1 60	20.5	36	- 1.00E_11	-	
11.04	1 0-1 3	Silty CLAT	71	22	40	16.0	100	100	90	95	95	90	-	20.7	-	-	-		-	_	+
TP05	0.4-0.6	Silty CLAY	56	15	41	17.5	100	100	100	99	98	85	_	15 3	_	1 58	21.0	_	_	_	+
1105	0.8-1 0	Silty CLAY	56	15	41	19.0	100	100	100	99	98	84	-	16.2	-	-	-	-	-	_	+
	1.2-1.5	Silty CLAY	65	18	47	19.0	100	100	100	96	96	91	65	20.8	-	_	_	-	1.00F-10	-	1
	1.7-2.0	Silty CLAY	65	15	50	17.0	100	100	99	99	98	96	-	21.1	-	_	-	-	-	-	+
	2.9-3.1	Sandy CLAY	30	11	19	8.5	100	100	100	100	100	92	-		-	_	-	-	-	-	1
TP06	0.3-0.5	Sandy CLAY	44	12	32	16.5	100	100	99	98	98	78	-	13.2	2	1.65	16.5	-	-	-	1
	0.9-1.2	Sandy CLAY	37	11	26	13.0	100	100	99	98	98	68	-	11.7	-	-	-	-	-	-	1
L		· · · · · · · · · · · · · · · · · · ·	_			·		1			1	1	1			·			- 1		- <b>1</b>



Location ID	Depth	Material Type		Atter	berg Limits				Sieve /	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
	1.8-2.0	Sandy CLAY	28	10	18	8.0	99	99	99	99	97	52	-	8.5	-	-	-	-	-	-	
	2.7-3.0	Silty SAND					100	100	100	99	96	31	-	4.5	-	-	-	-	-		-
BH18	1.0-1.45	Sandy CLAY	49	17	32	15	100	100	100	98	97	66	-	-	-	-	-	-	-	-	-
	2.0-2.45	Clayey SAND	27	13	13	5	100	100	100	97	92	32	-	-	-	-	-	-	-	-	-
	3.0-3.45	Sandy CLAY	34	13	22	9.5	100	100	100	98	96	58	-	-	-	-	-	-	-	-	-
	4.1-4.45	Clayey SAND	29	15	14	6	100	100	100	99	96	44	-	-	-	-	-	-	-	-	-
BH19	1.0-1.45	Sandy CLAY	42	17	25	12.5	100	100	100	92	85	52	-	-	-	-	-	-	-	-	-
	2.0-2.45	Sandy CLAY	33	16	17	9	100	100	100	99	97	55	-	-	-	-	-	-	-	-	-
	3.0-3.45	Silty SAND	23	16	7	1.5	100	100	100	94	86	22	-	-	-	-	-	-	-	-	-
	4.1-4.45	Silty SAND	-	-	-	-	99	96	91	52	41	21	-	-	-	-	-	-	-	-	-
	5.5-5.95	Silty SAND	-	-	-	-	96	94	90	55	34	9	-	-	-	-	-	-	-	-	-
BH20	1.0-1.45	Sandy CLAY	-	-	-	-	100	100	100	99	98	59	-	-	-	-	-	-	-	-	-
	2.0-2.45	Sandy CLAY	28	13	15	8	100	100	100	97	96	52	-	-	-	-	-	-	-	-	-
	4.0-4.44	Clayey SAND	-	-	-	-	93	90	83	55	48	33	-	-	-	-	-	-	-	-	-
TP08	0.3-0.6	Sandy CLAY	41	12	29	14.0	100	100	100	99	98	64	40	8.7	1	1.71	14.5	-	2.00E-10	-	-
	1.2-1.5	Sandy CLAY	37	11	26	12.0	100	100	100	100	99	67	-	10.4	1	-	-	-	-	-	-
	2.0-2.3	Clayey SAND	24	14	10	6.5	100	100	100	99	99	44	-	8.4	-	-	-	-	-	-	-
TP09	0.2-0.4	Silty SAND	20	18	2	0.5	100	100	100	99	97	18	-	1.6	3	1.77	11.0	-	-	-	-
	0.4-0.7	Clayey SAND	22	12	10	5.0	100	100	100	99	97	47	35	7.4	-	1.82	13.0	-	5.00E-10	-	-
	0.7-1.0	Clayey SAND	24	14	10	6.0	100	100	100	100	98	32	21	7.6	-	1.81	14.0	-	1.00E-09	-	-
	1.5-1.8	Sandy CLAY	29	11	18	9.5	100	100	100	100	99	80	-	12.8	-	-	-	-	-	-	-
TP29	0.3-0.5	Sandy CLAY	43	15	28	15.0	100	100	100	99	97	58	-	14.4	2	-	-	-	-	-	-
	0.5-0.8	Sandy CLAY	41	13	28	13.5	100	100	100	99	98	62	-	11.9	2	-	-	-	-	-	-
	2.5-2.7	Silty SAND	26	15	11	6.0	100	100	100	93	85	34	-	6.0	-	-	-	-	-	-	-
TP30	0.5-0.8	Sandy CLAY	41	13	28	14.0	100	100	100	99	98	64	-	10.6	2	-	-	-	-	-	-
	0.8-1.0	Sandy CLAY	44	14	30	15.0	100	100	100	99	98	66	-	13.6	2	-	-	-	-	-	-
	2.7-2.9	Silty SAND	21	16	5	2.0	100	100	100	100	99	28	-	4.8	-	-	-	-	-	-	-
TP31	2.0-2.3	Sandy CLAY	35	14	21	11.5	100	100	100	99	97	76	-	12.8	-	-	-	-	-	-	-
TP32	0.3-0.5	Clayey SAND	32	10	22	10.5	100	100	99	97	95	48	-	9.6	2	-	-	-	-	-	-
	0.7-1.0	Clayey SAND	33	11	22	10.0	100	99	98	97	95	42	-	9.5	2	-	-	-	-	-	-
	1.8-2.1	Clayey SAND	27	12	15	7.0	100	100	99	98	96	37	-	9.2	-	-	-	-	-	-	-
	·	•						Pro	posed Land	Fill Borrow	Areas										
TDB1	06-09	Silty SAND	_	_	_	_	100	100	100	100	00	30	_	10	_	_	_	_	_		
IFDI	1216		-	-	-	-	100	100	100	100	100	15	-	4.9	-	-	-	-	-	-	-
тррр	0710	SAND Silty CLAV	- 56	- 10	- 20	- 10.0	100	100	100	100	100	20	-	5.2 12.7	-	-	-	-	-	-	-
IFDZ	1216	Silty CLAT	50	10	20	19.0 10 E	100	100	90 100	90	93	00	-	10 5	2 2	-	-	-	-	-	-
	0.2.0.6	Silty CLAT	65	25	30	21.0	100	100	100	99 07	90	95 70	-	19.5	2 4	-	-	-	-	-	-
IPDS	0.3-0.0	Silty CLAY	60	22	45	21.U 10 E	100	100	90	0/ 70	00 77	70 60	57	10.0	4	- 1 50	- 21 E	-	-	-	-
	0.0-1.0	Siltstone/Mudstone	00	22	40	10.3	100	55	05	13	//	09	22	10.0	5	00.1	21.3	-	1.006-10	-	
	27-20		60	21	20	17.0	۵۵	۵۵	۵۵	۵۶	۵7	05	_	10.2	2		_	_		_	-
TDR/	0 6-0 0	Silty CLAT	62	<u>۲</u> 12	11	12.0	100	100	02	0/	رد دە	22	55	20 /	2	1 55	24.0	21	4 00F-10	-	-
1104	1 0-0.9		72	25	52	22 5	01	E3	/1	<u>у</u> ч //1	<i>JZ</i> //1	10	20	20.4	<u>ר</u>	1.55	24.0	5.1	2 00E-10	-	-
	2.2-2.2		10	25	55	22.3	100	100	100	100	41	40		23.0	<u>۲</u>	1.51	23.0	_	2.001-10	-	-
TDRE	0.6-0.0	Silty CLAV	-	- วว	-	- 17 ⊑	100	100	100	05	99	40 QQ	_	17 0	-	- 1 57	- 22 0	_	- 1 00E, 10	_	-
TDDC	0.0-0.9		51	10	45	1/.5	100	22	50 07	95 05	20	71	-	10 1	2	1.57	25.0	-	3 00E-10	-	-
IFDO	0.4-0.0	Mudetono	51	24	52 26	14.U 1 E E	100	50 100	100	100	95 00	00	-	10.9 10.9	2	1.30	20.3 26 F	-	5.00E-10	-	
	0.5-1.2	ividustone	00	24	50	10.0	100	100	100	100	33	00	-	23.0	۷	1.47	20.5	-	2.00E-10	-	

Location ID	Depth	Material Type		Atter	berg Limits				Sieve	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
		– Silty CLAY																			
TPB7	0.3-0.6	Silty CLAY	58	19	39	19.0	99	86	64	64	64	62	41	16.2	4	1.61	18.5	4.1	1.00E-10	-	-
	1.8-2.1	Clayey Gravel	60	22	38	18.0	98	68	51	48	46	41	22	18.8	2	1.49	22.0	-	7.00E-09	-	-
TPB8	0.5-0.8	Clayey Gravel	47	16	31	16.0	94	77	60	57	55	37	31	13.7	4	1.76	16.5	-	1.00E-10	-	-
	0.8-1.1	Sandy CLAY	46	14	32	15.5	100	99	98	94	91	57	-	12.9	2	-	-	-	-	-	-
	1.4-1.7	Clayey SAND	35	14	21	8.0	91	82	70	66	63	30	22	10.7	3	1.72	14.5	-	6.00E-11	-	-
	2.2-2.5	Siltstone/Mudstone - GRAVEL	45	17	28	13.0	53	29	10	10	10	7	-	17.3	2	-	-	-	-	-	-
TPB9	0.7-1.0	Silty CLAY	54	18	36	17.5	100	100	99	93	92	74	-	15.5	2	1.73	17.0	-	2.00E-10	-	-
	1.2-1.5	Silty CLAY	62	22	40	16.0	100	100	99	98	97	85	-	18.6	2	-	-	-	-	-	-
	2.5-3.0	Clayey GRAVEL	50	13	37	15.5	86	58	37	37	37	36	18	16.8	2	1.65	20.0	-	1.00E-10	-	-
	3.0-3.2	Gravelly CLAY	41	15	26	11.5	99	90	67	67	66	46	32	18.7	2	-	-	-	-	-	-
	4.3-5.0	Silty SAND	-	-	-	-	99	97	95	83	76	9	-	7.8	-	-	-	-	-	-	-
	5.0-5.5	Silty SAND	-	-	-	-	78	71	64	39	31	11	-	9.5	-	-	-	-	-	-	-
								Pr	oposed Lev	ee Alignme	nt Borrow A	Areas									
TPB10	0.3-0.6	Silty CLAY	46	16	30	13.5	100	100	100	99	99	83	45.6	12.5	2	1.67	16	-	1.00E-10	-	-
	1.9-2.2	Silty CLAY	56	15	41	13.0	100	100	100	100	99	87	-	20.0	2	-	-	-		-	-
	2.6-2.9	Sandy CLAY	28	15	13	10.5	100	100	100	99	98	70	36.5	12.6	1	1.74	15	-	9.00E-10	-	-
TPB11	0.3-0.6	Silty CLAY					100	100	100	99	99	72	43.6	10	4	-	-	-	-	-	-
	1.3-1.6	Sandy CLAY	26	14	12	8.5	100	100	99	98	97	53	24.1	8.3	2	-	-	-	-	-	-
TPB12	0.2-0.4	Silty CLAY	49	15	34	14.5	100	100	100	99	98	70	39.8	14.4	3	1.64	17.5	2.3	2.00E-10	-	-
	0.4-0.7	Silty CLAY	72	19	53	16.0							45.1	14.3	4	1.72	15.5		2.00E-10	-	-
	2.7-3.0	Sandy CLAY	32	14	18	9.0	100	100	100	100	99	60	30.0	10.5	2	1.77	13.5		3.00E-09	-	-
TPB13	0.6-1.0	Silty CLAY	36	14	22	13.5	100	100	100	99	98	58	-	11	3	1.65	16.5	2.4	-	-	-
	1.2-1.5	Silty CLAY	33	16	17	8.5	100	100	100	99	99	80	31	12.7	4	1.64	15.5	-	4.00E-10	-	-
	2.4-2.6	Sandy CLAY	27	16	11	7.0	100	100	100	100	100	55	19.7	7	3	1.81	12.5	-	9.00E-09	-	-
	3.0-3.3	Silty SAND					100	100	100	100	99	52	-	8.4	-	-	-	-	-	-	-
TPB14	0.3-0.6	Sandy SILT	25	16	9	5.5	100	100	100	100	100	58	18.7	4.6	3	1.79	12.5	0.8	2.00E-08	-	-
	0.6-0.9	Silty CLAY	31	14	17	4.5	100	100	100	100	99	53	22.7	6.2	3	1.75	12.0	1.0	2.00E-04	-	-
	1.2-1.5	Sandy CLAY	34	15	19	9.5	100	100	100	100	99	59	31.2	9.6	3	1.75	13	1.5	3.00E-10	-	-
	2.0-2.3	Sandy SILT	30	17	13	5.5	100	100	100	100	99	49	24.8	8.6	3	1.79	14.5	1.5	9.00E-11	-	-
	2.8-3.0	Silty SAND					100	100	100	100	99	29	-	6.7	-	-	-	-	-	-	-
ТРВ15	0.4-0.7	Silty CLAY	36	16	20	7.5	100	100	100	100	100	73	24	8	5	1.68	13.5	1.0	3.00E-09	-	-
	0.7-1.1	Silty SAND	22	NP	NP	0.0	100	100	100	100	99	39	-	4.9	3	1.85	12.5	-	6.00E-10	-	-
	1.3-1./	Sandy CLAY	28	14	14	8.0	100	100	100	99	98	65	-	ð./	3 2	1.//	14.0	-	5.00E-10	-	-
ILRID	0.3-0.0	Sanuy Siity CLAY	24	11	10	0.5	100	100	100	100	100	58	22	10.2	5	1.72	14.U	0.8	-	-	-
	0.7-1.0	Sandy CLAY	29 77	11	10	0.5 7 E	100	100	100	100	100		-	10.2	-	1.03	19.5 12 F	-	-	-	-
	1.2-1.5		21	11	10	7.5	100	100	100	100	100	22	-	9.Z	-	1./1	12.5	-	-	-	-
	2.0-2.3	Silty SAND					100	100	100	100	00	29	-	5.1	-	-	-	-		-	-
	<i>∠.1</i> -J.U	Sity SAND	1			<u> </u>	100	P	roposed Bo	prrow ID 5 (I	Barron Borr	ow)		5.7	-	-	_	_	-	-	
TDD47	0400		62	24	20	10 5	100	100	00		00	FC		10	c	4 5 2	10.0				1
IPRT/	0.4-0.8	SIITY CLAY	63	24	39	10.5	100	100	99	92	88	50	-	Τρ	б	1.52	19.0	-	-	-	-
	0011		20	20	10	12.0	100	07	05	70	60	22		15 6	Л	1 55	<u>, , , , , , , , , , , , , , , , , , , </u>		5 00E 10		-
	1216	- Cidyey SAIND	20 15	2U 1E	0T 0	11.0	100	97 00	95	/0 20	09 E0	55 24	-	11.0	4	1.55	22.0	-	J.00E-10	-	+
	1.3-1.0		43	12	50	11.0	100	22	94	00	59	24	-	14.3	-	-	-	-	-	-	

Location ID	Depth	Material Type		Atteri	berg Limits	;			Sieve	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
		- Clayey SAND																			
		XW Sandstone/																			
	2.1-2.3	- Clayey SAND	34	17	17	7.0	100	98	87	62	53	17	-	14.9	-	-	-	-	-	-	-
TPB18	0.3-0.7												-		-	-	-	-	-	-	-
	0.9-1.1	Silty CLAY	35	16	19	11.5	100	100	100	98	96	78	-	13.6	5	1.62	18.9	1.7	-	-	-
	2.3-2.6	Sandy CLAY	39	16	23	13.0	100	100	99	90	85	53	-	12.7	2	1.73	16.7	1.6	-	-	-
TPB19	0.3-0.6	Clayey SAND	23	12	11	9.5	100	100	99	94	90	46	-	6.5	3			-	-	-	-
	0.8-1.1	Clayey SAND	28	14	14	4.5	82	73	66	49	42	11	-	5.5	5			-	-	-	-
	4245	Mudstone/	26	10	47	10 F	00	07	00	62	50	22		46.5		4 70	17.0		4 005 00		-
	1.2-1.5	- Clayey SAND	36	19	17	10.5	99	97	90	63	53	23	-	16.5	4	1.73	17.9	-	4.00E-08	-	
	2022	Mudstone/	44	20	24	11.0	100	100	00	77	70	27		14.6		1 67	10 C		1 005 09		
TPR20	2.0-2.5		44 27	20	12	65	100	100	99	02	70	57	-	12.7	-	1.07	19.0	-	1.002-06	-	
TPB20	0.3-0.3		27	14	12	7.5	100	100	90	69	57	43	_	15.7		-				_	
11 021	1 3-1 7	Sandy CLAY	36	12	23	10.0	100	100	90	92	88	51	_	9.5	5	1.83	13.3	15		_	
	2 2-2 5	Sandy CLAT	38	14	23	10.0	100	100	99	94	91	52	_	12.3	-	1.84	12.5	2.6	_	-	
TPB22	0.4-0.7	Clavey SAND	25	9	16	6.0	100	100	98	90	85	40	-	59	-	-	-	-	-	-	
11 022	0.7-1.0	Sandy CLAY	41	16	25	12.0	100	100	99	94	91	61	-	93	5	_	-	_	-	_	
TPB24	1.7-2.0	Clavey SAND	29	11	18	8.0	99	96	92	82	76	37	-	10.2	-	-	-	_	-	-	
	2.2-2.5	Clayey SAND	31	13	18	7.5	71	65	57	36	30	11	-	5.1	-	-	-	-	-	-	
TPB25	0.6-1.0	Sandy CLAY	43	18	25	12.5	97	97	96	92	89	62	-	11.1	5	1.75	16.4	2.0	-	-	
	1.9-2.2	Sandy CLAY	41	14	27	12.0	100	100	98	92	89	58	-	9.7	4	-	-		-	-	
	2.2-2.5	Sandy CLAY	36	13	23	11.5	100	100	97	91	88	54	-	11.4	-	-	-		-	-	
TPB26	0.3-0.6	Silty CLAY	66	21	45	18.5	100	100	100	97	96	88	-	14.6	-	1.52	23.0	4.4	-	-	
		Mudstone/																			
	0.9-1.2	- Silty CLAY	62	27	35	17.5	100	100	99	96	95	84	-	17.8	6	1.54	23.8	-	2.00E-09	-	
TPB27	0.5-0.8	Sandy CLAY	40	17	23	11.5	100	100	98	86	84	50	-	9.3	-	1.78	15.4	2.4	-	-	
	1.5-2.0	Clayey SAND	35	14	21	11.0	100	100	99	95	91	48	-	10.0	-	1.81	11.9	-	-	-	
		Siltstone/Mudstone																			
	2.4-2.7	- Clayey SAND	41	15	26	9.5	87	79	63	52	45	17	-	20.0	-	1.74	17.3	-	2.00E-08	-	
TPB28	0.6-1.0	Sandy CLAY	51	18	33	16.5	100	100	98	92	89	69	-	14.0	-	-	-	-	-	-	
	1.7-2.0	Silty CLAY	62	17	45	19.5	100	100	100	97	95	81	-	20.8	-	-	-	-	-	-	
									Borrow I	D 8 – McNa	mara Dam	1									
TPB31	0.7-1.0	Silty CLAY	58	18	40	20	100	100	100	100	100	91	-	24.9	3	1.64	18.5	-	-	-	-
	1.7-2.0	Sandy CLAY	38	11	27	14	100	100	100	100	99	66	-	14.6	3	-	-	-	-	-	-
	3.0-3.3	Sandy CLAY	38	12	26	13.5	100	100	99	97	94	66	39	13.4	3	1.69	16.0	-	1.00E-10	-	-
				•				Borrow	B12 – McN	amara Prev	iously Prop	osed Dam				•					
TCOOT	0206	Sandy CLAV	20	10	25	11	100	100	100	00		65		<i>с 1</i>	ъ	1 67	14.0				
18833	0.3-0.0	Sandy CLAY	38 26	13	25 1E	14 7 c	100	100	100	99	98 00	05	-	0.4 <i>C 1</i>	3 1	1.07	14.U	-	-	-	
	1.0-1.3	Sanuy CLAY	20	11	12	7.5	100	100	100	33	90	45	-	0.4	2	1.03	11.5	-	-		<u> </u>
								Borrow ID	7 (TPB34, T	PB36 & TPE	337) – McNa	amara Dam	1								
TPB34	0.4-0.6	Silty CLAY	59	16	43	18.5	100	100	100	99	99	85	-	12.4	-	1.62	19.5	-	-	-	
	0.6-0.9	Sandy CLAY	30	12	18	9.5	100	100	100	99	98	60	-	9.2	-	1.79	14.0				
	2.0-2.4	Sandy CLAY	30	11	19	10	100	100	100	99	98	62	-	8.8	-	1.75	15.0				
	3.0-3.4	Sandy CLAY	29	11	18	9.5	98	98	98	98	97	62	-	11.0	-	1.75	14.5				
TPB36	0.2-0.4	Silty CLAY	45	16	29	16.5	100	100	100	100	99	76	-	10.7	2	1.62	12.5	2.6			

Location ID	Depth	Material Type		Atter	berg Limits				Sieve /	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
	0.4-0.6	Sandy CLAY	40	12	28	14	100	99	98	97	96	67	-	10.7	2	1.73	16.5				
	1.0-1.3	Sandy CLAY	27	12	15	7.5	100	100	100	99	98	52	-	7.7	2	-	-				
	2.2-2.4	Sandy CLAY	36	13	23	12.5	100	99	99	98	97	69	-	11.5	2	1.71	16.5				
	3.6-3.9	Sandy CLAY	30	13	17	9.0	100	99	99	98	98	74	-	10.6	2	1.76	15.5				
TPB37	0.3-0.5	Silty CLAY	50	14	36	17.5	100	100	100	100	99	82	-	11.2	-	1.65	17.0	3.3			
	0.6-0.8	Sandy CLAY	28	11	17	9.0	100	100	100	99	99	57	-	7.5	-	1.80	13.0				
	1.6-1.9	Sandy CLAY	31	11	20	11.0	100	100	100	99	97	66	-	10.6	-	1.65	14.0				
	2.7-3.0	Sandy CLAY	35	11	24	13.0	100	100	99	98	97	74	-	9.8	-	1.75	11.5				
	3.5-3.7	Sandy CLAY	34	13	21	12	100	100	100	99	98	71	-	10.3	-	1.73	13.5				
									33 McPh	ie Street Bo	orrow Area										
TPB38	1.3-1.5	Sandy CLAY	29	10	19	11.0	100	100	100	99	99	64	-	12.1	2		-	-	-	-	
	2.0-2.3	Sandy CLAY	24	14	10	6.0	100	100	100	100	100	52	-	11.3	2		-	-	-	-	
	4.0-4.3	Silty SAND	24	16	8	5.0	100	100	99	99	98	44	-	11.3	2		-	-	-	-	
TPB39	1.2-1.5	Silty SAND	21	12	9	4.0	100	100	100	99	97	49	-	13.5	2		-	-	-	-	
	2.2-2.5	Clayey SAND	27	11	16	8.0	100	100	100	99	98	44	-	7.9	2		-	-	-	-	
TPB40	1.3-1.6	Clayey SAND	21	11	10	5	100	100	100	99	98	42	-	6.7	2		-	-	-	-	
	2.0-2.5	Sandy CLAY	21	12	9	5.5	100	100	100	100	99	57	-	9.6	2		-	-	-	-	
TPB41	1.3-1.5	Silty SAND	16	13	3	1.0	100	100	100	99	97	38	-	3.8	3		-	-	-	-	
	2.0-2.3	Sandy CLAY	31	12	19	10.5	100	100	100	100	99	66	-	11.4	2						
									Race C	ourse Borro	ow Areas										
TPB42	0.7-1.0	Sandy CLAY	30	11	19	8.5	100	100	100	99	97	54	-	11.7	2						
	1.2-1.5	Sandy CLAY	30	11	19	8.0	100	100	100	100	99	57	-	10.2	2						
	1.8-2.1	Silty SAND	23	19	4	2.0	100	100	100	100	99	37	-	11.2	3						
	2.5-2.8	Sandy CLAY	33	12	21	9.0	100	100	100	100	100	-	-	16.6	2						
TPB43	0.7-1.0	Sandy CLAY	31	11	20	8.0	99	97	97	96	94	56	-	9.5	1						
	1.5-1.8	Silty CLAY	42	13	29	15.0	100	100	100	99	99	82	-	15.0	2						
	2.5-2.8	Silty CLAY	43	15	28	15.5	100	100	100	99	98	78	-	14.5	2						
TPB44	0.3-0.5	Clayey SAND	28	11	17	8.0	100	100	100	99	96	44	-								
	1.2-1.5	Sandy CLAY	32	12	20	10.0	100	100	100	99	99	59	-	10.0	2						
	2.4-2.7	Sandy CLAY	34	14	20	10.5	100	100	100	100	99	62	-	10.4	2						
									Borrov	w Pit Peaks	Property										
TPB45	0.5-1.0	Sandy CLAY	40	14	26	13.0	100	100	100	99	99	81	-	9.6	3			1.8			Pass
	2.2-2.5	Sandy CLAY	36	12	24	10.5	100	100	100	100	100	71	-	16.3	3						
	2.8-3.2	Sandy CLAY	34	13	21	10.5	100	100	100	100	99	66	-	16.7	1						
TPB46	0.3-0.7	, Sandy CLAY	26	11	15	8.0	100	100	100	99	99	65	-	5.1	3						
-	0.8-1.2	, Sandy CLAY	30	12	18	9.0	100	100	100	100	99	58	-	8.1	2						
	1.4-1.8	Clayey SAND	29	12	17	7.5	100	100	100	100	99	47	-	-	2						
TPB47	0.5-0.8	Silty CLAY	29	12	17	9.0	100	100	100	100	99	71	-	7.8	3						
	1.8-2.2	Clayey SAND	28	12	16	8.0	100	100	100	99	97	40	-	-	3						
	2.4-2.7	Clayey SAND	29	11	18	8.0	100	100	100	99	99	49	-	8.3	3						
TPB48	0.5-0.8	Sandy CLAY	23	17	6	3.5	100	100	100	100	100	55	-	5.0	3						
	1.3-1.7	Sandy CLAY	30	12	18	10	100	100	100	100	99	58	-	6.6	2						
	1.9-2.3	Clayey SAND	26	11	15	7.0	100	100	100	100	99	38	-	6.3	2						
TPB49	0.3-0.6	Sandy CLAY	31	12	19	9.0	100	100	100	100	100	77	-	6.8	3						
	0.7-1.0	Clayey SAND	26	15	11	6.5	100	100	100	100	100	49	-	6.5	2						

Location ID	Depth	Material Type		Atter	berg Limits				Sieve	Analysis % F	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
	2.4-2.6	Silty SAND	25	16	9	6.0	100	100	100	100	99	36	-	6.2	2						
TPB50	0.7-1.0	Sandy CLAY	24	12	12	7.0	100	100	100	100	100	55	-	5.4	2						
	1.2-1.6	Sandy CLAY	32	13	19	10.5	100	100	100	99	99	70	-	8.9	2			1.0			Pass
	2.0-2.4	Sandy CLAY	32	12	20	11.0	100	100	100	99	99	69	-	8.6	2						
	2.4-2.8	Sandy CLAY	27	12	15	7.0	100	100	100	100	99	53	-	7.1	3						
TPB51	1.3-1.6	Clayey SAND	26	12	14	6.5	100	100	100	100	98	39	-	12.2	3			2.0			Pass
	2.6-3.0	Sandy CLAY	41	14	27	12.0	100	100	100	100	100	68	-	7.6	3						
TPB52	0.9-1.2	Sandy CLAY	25	11	14	7.0	100	100	100	100	99	66	-	6.1	2						
	2.1-2.4	Clayey SAND	25	14	11	6.5	100	100	100	99	97	36	-	7.4	3						
ТРВ53	1.3-1.7	Sandy CLAY	33	13	20	11.0	100	100	100	100	99	55	-	10.4	3						
	2.4-2.8	Sandy CLAY	30	12	18	8.5	100	100	100	100	100	52	-	8.4	3						
								Pron	osed Dam F	xtension C	amnhell's P	roperty									
TDDCC	0507		20	10	20	12 5	100	100	100					0.0	2			1 5			
TPB50	0.5-0.7		30	10	20	12.5	100	100	100	99	97	61	-	9.9	3			1.5			
	1.0-1.3		22 28	11	17	9.5	100	100	100	99	97	47 20	-	7.0	2						
	2 5-2 7	Sandy CLAV	20	10	22	0.0 Q 5	100	100	100	99	95	55	-	0.0 Q /	2						
TPB57	0.4-0.6		24	10	13	7.0	100	100	100	98	93	37	_	63	3						
11 037	0.7-1.0	Clavey SAND	37	12	25	12.0	100	100	100	98	92	44	_	8.6	3						
								Pro	posed Dam	Extension I	Lavelle's Pro	operty	1			1	1		11		
TPB58	0.5-0.8	Sandy CLAY	51	12	39	15.0	100	97	97	96	96	82	_	19.8	2						
	0.9-1.2	Sandy CLAY	33	12	21	12.0	100	100	100	97	95	61	-	9.0	3						
	2.8-3.1	Sandy CLAY	35	10	25	12.0	100	100	99	97	95	66	-	9.2	2						
	4.5-4.8	Sandy CLAY	52	15	37	17.0	100	100	100	99	99	81	-	22.5	3						
TPB59	0.2-0.3	Sandy CLAY	28	12	16	9.0	99	99	99	98	97	75	-	8.9	2			1.0			
	0.4-0.7	Sandy CLAY	40	11	29	14.0	100	100	100	99	99	79	-	12.6	3						
	1.0-1.3	Silty CLAY	45	13	32	16.0	100	100	99	98	98	79		13.6	2						
	1.7-2.0	Sandy CLAY	37	13	24	11.0	100	99	99	99	98	58	-	12.2	2						
	2.1-2.4	Sandy CLAY	33	12	21	10.0	100	99	99	99	99	56	-	10.9	2						
TPB60	0.2-0.4	Sandy CLAY	21	13	8	4.5	100	100	100	99	97	60	-	4.0	3						
	0.5-0.8	Sandy CLAY	35	11	24	13.0	100	100	100	99	98	67	-	9.5	2						
	0.9-1.2	Sandy CLAY	35	10	25	9.5	100	100	100	99	99	54	-	7.2	3						
TDDC4	1.7-2.0	Sandy CLAY	37	12	25	12.0	100	98	97	96	96	67	-	10.0	3						
16801	0.4-0.7	Sanay CLAY	28	11	1/	9.5	100	100	100	99	9/	59	-	6.9 7 0	2						
	0.9-1.2	Sandy CLAY	20	12	21	10.0	100	100	100	98	97	55 72	-	7.0	1						
	2.0-2.3	Salluy CLAT	55	12	21	12.3	100	100	Borrow			/3		5.1	Z						<u> </u>
TDDCO				10	4 🗖	7.0	100	100	100		s rioperty	24		F 1	n	1					1
	0.5-0.8	Clayey SAND	2/	12	21	7.U 12.0	100	100	100	100	90	54 67	-	5.1 10.4	5 2						
1804	0.0-0.9 1 2_1 ⊑	Sandy CLAT	0C 20	11	24 01	12.0	100	100	100	100	99	54 51	-	10.4 גע	2						
	1.2-1.3	Januy CLAT	52	11	21	9.0	100	100	100	33	33	54	-	0.5	ر ا						
			1		<u> </u>	<u> </u>	<u> </u>	<u> </u>	Borrow	Pit Waster	Diversion	1	1	1	<u> </u>	1	1	I	I		1
TDD 70	0400		40	14	20	12.0	100	100	100	100		07		0.2		1		1.0			1
15810	0.4-0.6		40	11	29	12.0	100	100	100	100	99	<u>ک/</u>	-	9.3	-	-	-	1.9	-	-	-
	0.7-1.0	SIITY CLAY	אט	19	49	19.0	100	100	100	100	99	81	-	9.4	-	-	-	1./	-	-	-

Location ID	Depth	Material Type		Atter	berg Limits				Sieve /	Analysis %	Passing			Moisture Content	Emerson Class Number	Maximum Dry Density or Max/Min	Optimum Moisture Content	Shrink Swell Index	Falling Head Permeability	CBR (10days soak)	Organic Content
	(m)		LL %	PL %	PI %	LS %	9.5 (mm)	4.75 (mm)	2.36 (mm)	0.6 (mm)	0.425 (mm)	0.075 (mm)	0.002 (mm)	(%)		(t/m3)	(%)	(%)	k (m/s)	(%)	
	2.2-2.4	Sandy CLAY	42	11	31	12.5	100	100	100	99	99	72	-	13.6	-	-	-	-	-	-	-
TPB72	1.2-1.5	Sandy CLAY	40	13	27	13.5	100	100	100	99	99	73	-	13.9	3	-	-	2.4	-	-	-
	1.4-1.8	Sandy CLAY	40	13	27	14.0	100	100	98	96	96	60	-	13.3	3	-	-	-	-	-	-
	2.4-2.8	Sandy CLAY	33	11	22	9.0	100	100	100	99	99	56	-	13.1	3	-	-	-	-	-	-
TPB73	0.5-1.0	Sandy CLAY	42	13	29	18.0	100	100	99	99	98	77	-	13.3	6	-	-	2.1	-	-	-
	1.8-2.2	Sandy CLAY	44	17	27	14.0	100	100	100	100	99	80	-	16.6	3	-	-	2.1	-	-	-
									Pa	vement Tes	st Pits										
TPX1	0.22-0.3	Gravelly SAND	31	22	9	4	80	70	66	62	60	35	-	10.4	-			-			-
	0.3-0.45	Silty CLAY	42	13	29	15	98	96	96	95	95	69	-	5.1	3	1.69	17.3	-	6.00E-11	2.5	-
	0.45-0.6	Sandy CLAY	37	12	25	13.5	100	100	100	99	97	56	-	11.8	3	1.72	16.3	-	1.00E-11	2.5	-
	0.6-0.9	Sandy CLAY	38	11	27	13.5	100	100	100	99	97	61	-	7.7	3	1.73	17.1	-	-	2.5	-
TPX2	0.03-0.2	Sandy GRAVEL	16	10	6	3.5	63	45	32	23	20	8	-	3.3	-			-	-		-
	0.33-0.55	Silty CLAY	30	11	19	10.0	100	100	99	98	97	50	-	2.8	-	1.79	15.1	-	-	4.5	-
	0.55-0.8	Silty SAND	17	12	5	3.0	100	100	99	98	97	46	-	4.1	-	1.75	14.0	-	-	3	-
	0.015-																				
TPX4	0.22	Sandy GRAVEL	25	18	7	4.5	81	60	43	32	30	16	-	6.0	-		-	-	-		-
	0.22-0.5	Silty SAND	17	14	3	1.5	100	100	100	99	97	32	-	1.0	-	1.91	-	-	-	9	-
TPX6	0.03-0.18	Sandy GRAVEL	20	16	4	2.0	74	59	44	35	33	11	-	4.1	-	-	-	-	-		-
	0.18-0.4	Gravelly Silt	22	17	5	3.5	85	75	64	55	51	21	-	5.2	-	-	-	-	-		-
	0.4-0.5	Clayey SAND	22	10	12	5.0	94	90	85	66	59	24	-	6.0	-	-	-	-	-		-
	0.5-0.8	Silty CLAY	39	14	25	13.5	100	100	100	99	99	87	-	6.2	-	1.64	-	-	-	1.5	-
TPX8	0.03-0.2	Sandy GRAVEL	28	24	4	1.5	37	22	14	7	7	2	-	1.4	-	-	-	-	-		-
	0.2-0.3	Clayey SAND	30	12	18	8.5	93	87	74	72	72	39	-	11.0	-	-	-	-	-		-
	0.3-0.6	Sandy CLAY	32	11	21	9.5	98	97	96	92	91	52	-	3.7	-	1.80		-	-	4	-
	0.015-																				
TPX9	0.18	Sandy GRAVEL	28	14	14	7.5	69	54	42	24	22	12	-	8.0	-	-	-	-	-		-
	0.18-0.5	Silty CLAY	41	13	28	15.0	100	100	100	99	99	81	-	13.2	-	1.69	-	-	-	1.0	-
	0.7-0.9	Silty CLAY	37	11	26	14.0	100	100	99	98	98	76	-	10.9	-	-	-	-	-	1.5	-



## **GEOTECHNICAL LOG OF BOREHOLE AND EXCAVATION**

(These notes explain the terms and abbreviations used on the log sheets)

### GENERAL

Information obtained from drilling investigations is recorded on log sheets. The "*Geotechnical Log of Borehole*" presents data from drilling operations where a core barrel has not been used to recover material and information is based on a combination of regular sampling and in-situ testing. The "*Geotechnical Log of Excavation*" presents data obtained on the subsurface profile from observations of excavations, either natural or man made.

The heading of the log sheets contains information on client and project identification, hole or pit identification, location and elevation. Details of the drilling contractor, equipment, drilling or excavation dates, and of the personnel responsible for the preparation of log, are given at the bottom of the sheet. The main section of the log contains information on drilling or excavation methods and conditions, material substance description, details of in-situ tests and additional observations, presented as a series of columns plotted with reference to length in metres below the ground surface.

As far as is practicable the data contained on the log sheets is factual. Some interpretation is inevitable in the assessment of conditions between samples and of the origin of the materials. Material description and classification is generally based on AS1726-1993 Geotechnical Site Investigations.

### **DRILLING OR EXCAVATION METHOD**

- B Rubber tyred backhoe
- H Hand Excavation
- EE Existing excavation
- EX Excavator
- HA Hand auger
- AS Auger screwing
- AV Auger drilling with V-bit
- ATC Auger drilling with TC bit
- WR Washbore drilling with roller bit
- WD Washbore drilling with drag or blade
- RC Reverse circulation
- RAB Rotary Air Blast (Percussion)

### SUPPORT

The measures used to support the hole or excavation during logging and the extent of those measures are described in the support column

### WATER

- $\blacksquare$  Groundwater level with date.
- Drilling fluid loss at level marked.
- Groundwater inflow at the level marked.

### **CLASSIFICATION SYMBOL**

Standard symbol in accordance with the Unified Soil Classification System.

### MATERIAL DESCRIPTION

Term	Grain Size (mm)
Boulders	>200
Cobbles	60 - 200
Gravel	2 - 60
Sand	0.06 - 2
Silt	0.002 - 0.06
Clay	<0.002

### DESCRIPTIVE TERMS FOR MATERIAL PROPORTIONS

Coarse G	rained Soils	Fine Gra	ained Soils
% Fines	Term	% Coarse	Term
≤ 5	trace	≤ 15	trace
> 5 ≤ 12	with some	> 15 ≤ 30	with some
> 12	Prefix silty /	> 30	Prefix sandy /
	clayey		gravelly

## MOISTURE

- D Dry
- M Moist no free water on remoulding
- W Wet free water on remoulding

### DENSITY

(Non-cohesive soil only)

Symbol	Term	SPT	Density Index
		N Value	
VL	very loose	0 - 4	< 15%
L	loose	4 - 10	15 - 35%
MD	medium dense	10 - 30	35 - 65%
D	dense	30 - 50	65 - 85%
VD	very dense	>50	> 85%

### CONSISTENCY

(Cohesive soil only)

Symbol	Term	Cone	Undrained
		Resistance	Shear Strength
VS	very soft	0-180 kPa	< 25 kPa
S	soft	180-375 kPa	25 – 50 kPa
F	firm	375-750 kPa	50 – 100 kPa
St	stiff	750-1500 kPa	100 – 200 kPa

Symbol	Term	Cone	Undrained
		Resistance	Shear Strength
VSt	very stiff	1500-3000 kPa	200 – 400 kPa
Н	hard	>3000 kPa	> 400 kPa
Fr	friable	n/a	

### SAMPLE

Sections sampled bounded by lines across column

- DS Disturbed sample
- BS Bulk disturbed sample
- S Standard penetration test sample
- U50 Undisturbed sample (50 mm diameter)
- U75 Undisturbed sample (75 mm diameter)

### TEST TYPE

- S Standard penetration test
- V Vane shear
- PP Pocket penetrometer
- P Pressuremeter
- W Permeability
- MC Field moisture content
- LL Liquid limit
- PL Plastic limit
- PI Plasticity index
- LS Linear shrinkage
- UC Unconfined compression

### **OTHER OBSERVATIONS**

Information on the structure of the material and an assessment of the inferred origin of the material.

RESIDUAL SOIL: Soil essentially in-situ and derived from weathering and degradation of the underlying bedrock

ALLUVIUM: Soil transported by river or stream processes and deposited remote from the parent material.

COLLUVIUM: Soil transported down slope by a process of creep and/or landslide activity.

BEDROCK: In-situ rock (excludes transported boulders or boulders underlain by soil)

TALUS: Rock fragments deposited from rock fall landslides

FILL: Material placed by human activities

# APPENDIX B SITE LOCATION PLANS

Please refer to appendix A of the main design report for the relevant drawings.

# APPENDIX C TEST PIT, BOREHOLE AND DCP LOGS

PRO.	JECT ATIOI	. <u>.</u>   N :	Roma Roma	Flood N	<i>l</i> itigation	1		EXCA	VATION - GEOLOGICA CLIENT : Ostwald Bros F FEATURE :	Pty Ltd			F S	<b>PIT  </b> ILE / HEE	<b>NO</b> : JOB NC T:1 C	<b>TP01</b> 0 : 30031182 0F 1
POSI		і: NT 1	YPE	: CAT	CRS222	25			SURFACE ELEV METHOD : Bacl	ATION :						
DATE	E EX	CAV	ATED	: 17/5/	13				LOGGED BY : 0	CM				(	CHECK	KED BY : JSM
EXC/	AVAT		DIME	NSION	S : 0.60	) m Wll	DE		N 4 A T							
z	2	Di		NG av ທ	7 0		z		MAT	ERIAL		X				
	T	SUPPORT	GROUND WATE LEVELS	SAMPLES {	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC SYMBOL	So	MATERIAL DESCRIPTION bil Type, Colour, Plasticity or Particle Characte Secondary and Minor Components	ristic	MOISTURE	CONSISTENC RELATIVE DENSITY	5 1(	원 0152	0	STRUCTURE & Other Observations
ÍĪ	ĪĪ				- 0.0		СН	Silty CLAY	Y: Very stiff, high plasticity, dark grey, dry to slig	ghtly moist, grass,		VSt			ALL	UVIUM
				0.20m B	0.5			becoming	hard, trace of rootlets to 0.4m	^					<sup>&gt;&gt;X</sup> 0.20	): HP In-situ >600 kPa -
				0.80m			СН					т				-
				<u>1.40m</u> B	1.5		сі	1.40m Sandy CL/ parts, fine sandy silt i 1.60m gradually s	AY: Very stiff, medium plasticity, light brown, sl grained sands, some medium grains, high silt in parts, dry to moist shading light brown only	ightly brown in content, grading	W - Q	VSt		×	1.60	- ): HP In-situ =350 kPa
				1.80m			СІ									
				1.90m				1.90m								
				B 2.20m	2.0		SC	2.20m	AND: Dense, fine to medium grained, light brow fines, grading silty sand in parts	n, varying low	M - D	۵				-
900				<u>2.50m</u> B	- - 2.5 - -			2.70m	ense, tine to medium grained, light brown, vary lent, some clayey/silty sand lenses/bands, easie	ng trace to some er digging	W - Q	٩				
%2014 12:56 8.2.8		_		3.00m	- 3.0		· SP	3.10m								-
LEVEE.GPJ < <drawingrile>&gt; 12/0</drawingrile>								EXCAVAT No ground Test pit ba Test pit loc stage, May	TION TP01 TERMINATED AT 3.10 m d water encountered. ackfilled on completion. ccated in Borrow Area 1 as proposed as possib ay 2013.	le site at tender						-
	PH	ютос	GRAPHS	<u> </u>	4.0											
ī 2	NC	DTES			J YES				[			MPOL		<b>T</b> -	ONOLO	
ANNE LIBKAKY - COLOUR.GEB LOG IE L B B B B B L B B B B B F	THOD Natu Exis Bacl Bullo Ripp PPORT	iral Ex ting E thoe E dozer ter F pering	xposure xcavati Blade Blade	e ion V		D Oct., 7 o Oct., 7 evel on [ ater inflo	o Resi 3 Wate Date sh w low	stance er nown	SAMPLES & FIELD TESTS         U50       Undisturbed Sample         50 mm diameter         D       Disturbed Sample         B       Bulk Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       Plate Bearing Test	CLASSIFICAT SOIL DE Based Classifica D - Dry M - Moist W - Wet	ION SY SCRIP on Unif ation Sy	INBOL: FION ied istem	5 &		CONSIS RELATIV /S /St /St /L /D /D	I ENCY/ /E DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See E detail & bas	Explai Is of a sis of	nator Ibbre desc	y Note viatior ription	es for is s.					SMEC AUSTRALIA							

PR( LO(		CT : ON :	Roma Roma	Flood N	Mitigatio	n		E	(CA)	VA	TIC	<b>ON -</b> CLIEN <sup>-</sup> FEATU	GE T JRE	OLO : Ostwa	GIC	AL I s Pty Lt	L <b>OG</b>				<b>PIT</b> FILE SHEE	<b>NC</b> / JO ET :	<b>)</b> : <b>TP03</b> B NO : 30031182 : 1 OF 1	
PO	SITIC	)N :	E: 677	7064.85	0, N: 70	63662.	440 (	55 MG/	A94)				5	SURFAC		VATIC	N : 304	1.184 (.	AHD)					
EQ				: KUB(		cavator	· 4T						<u> </u>		D:Exo DBV:	cavato	r					СН		
EX				NSION	S : 0.6	) m WI	DE						L		. 10	CIVI						CII	ILCRED BT . JOIN	
	-	D	RILLIN	IG		-									MAT	TERIA	L							
	ENETRATION	UPPORT	OUND WATER LEVELS	AMPLES & ELD TESTS	-EVATION (RL) EPTH (m)	BRAPHIC LOG	<b>SSIFICATION</b>		Sc	oil Typ	pe, Co Sec	MATERI olour, Pla xondary a	IAL DES asticity o and Min	SCRIPTIC or Particle or Compo	ON e Charact onents	teristic		AOISTURE CONDITION	INSISTENCY RELATIVE		DCP		STRUCTURE & Other Observation	ons
۳ı	е и ш т Г Т Т	: 0	80	SE		*////	5 C		0.11 01 41	<u> </u>					4.4.6.			2 0	0 1 1 1	5	1015	20		
					304.0		СІ	0.30m	Silty CLA of fine gra 0.1m	Y: ver ained	ry stiff sand	f to hard, I, grass a	nd rootl	m plasticity lets to 0.2r	y, dark bro m, becom	rown to g ning haro	grey, trace d below	۵	VSt to I				ALLOVIOM	
					0.5		СІ	0.40m	light brow powdery o higher mo	n to b calcid oisture	brown le noc e coni	i, trace of dules >15 tent	f fine gra 5 <u>mm dia</u>	ameter	d in parts	s with so 	me white							-
					303.5		CI	0.80m	Sandy CL		hard,	low to me	edium p	blasticity, li	ight browr	n and lig	ht grey						0.60: PP > 500 kPa	
					1.0-		СІ		brown mo	ottled,	, som	e fine to r	medium	n grained s	sand									-
					- 303.0			1.20m 	becoming veneer ale	g partl long fi	ly fiss issure	ured, sor ed surface	me dark es	grey to bl	lack mottl	ling and	staining/	v						
	1.5 - CL S																	D to N					1.50: PP > 500 kPa	_
					2.0			1.80m	grey to lig staining, f	 ght bro highei	own, i r silt c	mottled o content	occasion	 nally grey t	 to black m	mottles a	and							-
					- 302.0		СІ																	
					2.5																			-
					301.5	//////		2.70m     	EXCAVA <sup>-</sup> No ground Test pit ba	TION d wat ackfill	TP03 ter en led or	3 TERMIN counteren n complet	NATED d. tion.	AT 2.70 m	n									
					3.0																			-
					3.5-																			_
					300.5																			
					4.0																			
		NOTES	GRAPHS		] YES		[		NO															
M N E B B R S T	ETHO Na Ex H Ba Bu Rij JPPOI Tir	D atural E: isting E ickhoe I Ildozer pper RT nbering	kposure xcavati Bucket Blade	e on V		TION 	lo Res 3 Wat Date s ow flow	istance er hown		<b>S</b> U D В М H V Р	AMP J50 ) , , , , , , , , , , , , , , , , , ,	LES & Fl - Undis 50 mr - Distur - Bulk [ - Moistu - Hand - Vane R-Rei - Plate	IELD TI sturbed m diame rbed Sa Disturbe ure Cor Penetr Shear; moudeo Bearing	ESTS Sample eter ample ed Sample ntent rometer (L ; P-Peak, d (uncorre g Test	e JCS kPa) ected kPa	CL N D N V a)	ASSIFICA SOIL D Base Classifi IOISTURE 0 - Dry 1 - Mois V - Wet	ATION S DESCRIP id on Uni ication S	YMBO PTION ified ystem	LS &		COI REL VS S F St VSt H VL L MD D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium DC - Dense - Very Dens	ense e
See deta	Expl ails of asis c	anator abbre of desc	y Note viation ription	es for Is s.						S	SM	IEC /	AUS	STR/	ALIA								SM	IEC

EXCAVATION - GEOLOGICAL LOG       PIT NO :       TP04         PROJECT : Roma Flood Mitigation       CLIENT :       Ostwald Bros Pty Ltd       FILE / JOB NO :       30031182         LOCATION : Roma       FEATURE :       SHEET :       1 OF 1										NO : TP04 JOB NO : 30031182 T : 1 OF 1		
POS		N :	E: 677	7170.18	5, N: 70	63555.	370 (! 4T	55 MGA94) SURFACE ELEVATION : 30	2.540 (/	AHD)		
DAT	TE EX	CAV	ATED	: 29/10	/13	Javalui	41	LOGGED BY : CM			(	CHECKED BY : JSM
EXC	CAVA	TION	DIME	NSION	S : 0.60	) m Wll	DE					
	z			iG ∞Ω	z o		S	MATERIAL		λ		
L K	E PENETRATIC H	SUPPORT	GROUND WATI LEVELS	SAMPLES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	요 5 10152	STRUCTURE & Other Observations
					- 302.5		СІ	Silty CLAY: very stiff, medium plasticity, dark grey brown , traces to som- fine grained sand in parts, grass and rootlets to 0.2m, becoming hard below 0.1m		VSt		ALLUVIUM
					- 5.0 305  -		сі 	0.40m becoming light brown to brown, traces of fine to medium grained sand in parts 0.80m becoming light brown to slight light grey brown, some white powdery	×			-
					- 1.0 - 3:2 - 6 - -		СІ	calcide nodules >15mm diameter				-
					1.5 0.0 		сı	1.50m moisture content, becoming moist in parts		Т		
					- 300 		СІ	light grey brown to light brown, some dark grey to black staining/ veneer along fissures, occasional glass like fine gravel sized angular crystalline inclusions, occasional while powdery nodules to 20mm diameter, blocky higher moisture content in parts	Σ			
005.20					2.5 - 0.000 - - -		 CI	2.60m occasional rock like structure in parts				
					- 0.5 - 20 - 70 - 70 - 70			3.10m EXCAVATION TP04 TERMINATED AT 3.10 m No ground water encountered. Test pit backfilled on completion.				
יייי ארר שישיאר איי איי איי איי איי איי איי איי איי												-
Since a				<u> </u>	4.0-							
		OTES	жарна	°	YES		[		ATION S	MBOL:	S& 0	CONSISTENCY/
	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering			0 Oct., 7 evel on I rater inflo	o Resi 3 Wate Date sl w low	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) WS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	DESCRIP ed on Uni fication Sy E st t	TION fied /stem	F F S \	RELATIVE DENSITY           /S         - Very Soft           S         - Soft           -         - Firm           St         - Stiff           /St         - Very Stiff           +         - Hard           /L         - Very Loose           -         - Loose           VID         - Medium Dense           0         - Dense           /D         - Very Dense		
See deta & ba	Expla ails of a asis of	nator abbre desc	y Note viation	es for Is s.				SMEC AUSTRALIA				

PROJECT : Roma Flood Mitigation LOCATION : Roma								EXCAVATION - GEOLOGICAL LOG					<b>PIT</b> FILE SHEI	PIT NO : TP05 FILE / JOB NO : 30031182 SHEET : 1 OF 1		
POS		N :	E: 677	206.22	9, N: 70	63350.	609 (5	55 MGA94)		SURFACE ELEV	ATION : 302.4	150 ( <i>A</i>	AHD)			
DAT	ΕΕ)	EN I KCAV		: KUB	)/13	cavator	41			OGGED BY : C	avator CM				СН	ECKED BY : JSM
EXC	AVA	TION	DIME	NSION	S : 0.60	) m WI	DE				-				-	
		DI	RILLIN	G			7			MATI	ERIAL					
VE E DENIETDATION	E PENETRATION SUPPORT SROUPDAT SROUPDATE SRAMPLES & SAMPLES & CEVELS ELEVATION (RL) DEPTH (m)			GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	MATERIAL DE bil Type, Colour, Plasticity Secondary and Mir	SCRIPTION or Particle Characte or Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	요 5 1015	20	STRUCTURE & Other Observations		
					- 0.0			Silty CLA	Y: very stiff, medium plasti and, grass and rootlets to	city, dark grey brown, 0.15m	, traces of fine					ALLUVIUM
					- - - 0 0 0 0 0 0 0 0 0 0 0		СІ — —	0.40m becoming fine grain	medium to high plasticity, ed sand in parts, high mois	shading dark grey to sture content	brown, traces of					- 0.30: PP > 500 kPa - -
					- - - 5: 1.0 —		сі-сн	0.80m light brow occasiona	n, occasonal white powde al glass like fine gravel size	ry calcide nodules >1 ed crystals inclusions	5mm diameter,					0.70: PP > 500 kPa - - -
					- - - 0.1.5 -			1.20m	ght brown, light grey, brow blackened, some dark grey urfaces, moist.	m and orange brown, v to black staining/ ve	motteled, neer along					1.20: Easy digging 
					- - - - - - - - -		CI-CH									
					- - 00 2.5 — -		— — Сі-СН	2.50m some fine	grained sand content							- - - -
					-		SM	Silty SAN some clay	D: dense to very dense, fir / fines in parts	ne grained, yellow bro	own/ light brown,					-
					- 		CL	Sandy CL grained sa	AY: hard, low plasticity, ye and with high silt content,	ellow brown and light some clayey sand in p	grey brown, fine pockets					2.90: Slower digging
							5	No ground Test pit ba	TION TP05 TERMINATED d water encountered. ackfilled on completion.	AT 3.10 m						- - - - - - - - - - - - - - - - 
	F	PHOTO	RAPHS		_ <sub>00</sub> 4.0 — ] YES		[									
ME BH B R SUI	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     10 Oct., 73 Watter       T     Timbering			lo Resi 3 Wate Date sh w low	stance er iown	SAMPLES & FIELD T U50 - Undisturbed 50 mm diam D - Disturbed S: B - Bulk Disturb MC - Moisture Co HP - Hand Penet VS - Vane Shear R-Remoude PBT - Plate Bearin	ESTS Sample eter ample ed Sample ntent rometer (UCS kPa) P-Peak, d (uncorrected kPa) g Test	CLASSIFICAT SOIL DE: Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unif ation Sy	TION FION ied stem	S &	CON REL VS S F St VSt H VL L MD D VD	ASISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense			
See l detai & bas	Explais of sis o	anator abbre f desc	y Note viation riptions	s for s s.					SMEC AU	STRALIA						

EXCAVATION - GEOLOGICAL LOG       PIT NO :       TP06         PROJECT : Roma Flood Mitigation       CLIENT : Ostwald Bros Pty Ltd       FILE / JOB NO : 30031182         LOCATION : Roma       FEATURE :       SHEET : 1 OF 1										0 : <b>TP06</b> B NO : 30031182 1 OF 1				
POS	ITIC	N :	E: 677	412.95	9, N: 70	63284.	916 (	55 MGA94)	SURFACE ELEVATION : 30	2.500 (	AHD)			
EQU DAT	E EX	EN I KCAV		: KUB0 : 27/10	)1A EX //13	cavato	·41		LOGGED BY : CM				CHE	ECKED BY : JSM
EXC		TION	DIME	NSION	S : 0.6	0 m WI	DE						-	
		D	RILLIN	G			7		MATERIAL		<u> </u>			
VE E DENETRATION	E PENETRATION SUPPORT SROUND WATEF LEVELS SAMPLES & SAMPLES & FIELD TESTE (RL) DE PTH (m) CRAPHIC CAG				GRAPHIC LOG	CLASSIFICATIO SYMBOL	S	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	요 5 1015	20	STRUCTURE & Other Observations	
					302.5		сі сі	Silty CLA grained s 0.20m 0.30m becreasin becoming powdery	AY: very stiff, medium plasticity, dark grey brown, traces of fine sand, grass and rootlets to 0.15m	 	VSt to H			ALLUVIUM
					0-0.5 - 301 - 9 - 1.0 - - - - - - - - - - - - - - - - - - -		СН	0.80m Sandy Cl sand	CLAY: hard, medium plasticity, yellow and brown, fine grained		т			
				сı	1.50m       increasing fine sand and silt content, lower moisture content         1.80m       Sandy CLAY: very stiff, fine to medium grained sand, yellowy brown/ light grey brown, varying clay fines content									
					0. 2.5 <sup>-</sup> 0. 0.5 <sup>-</sup> 0.00 0.		CI SM	2.50m Silty SAN brown/ gi	ND decreasing silt and clay fines content, shading lighter yellow rey brown, lower moisture content		D to VD			- - - - - - - - - - - - - - 
B C C C C C C C C C C C C C C C C C C C						3.00m EXCAVA No grour Test pit b	ATION TP06 TERMINATED AT 3.00 m nd water encountered. backfilled on completion.					- 		
	r	рното	RADHO	-	ل <sub>ي 4.0</sub> –									
ME N BH R SUI	MOTES     NOTES       METHOD     N       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering			lo Res 3 Wat Date s bw flow	NO istance er hown	CLASSIFICATION SYMBOLS & SOIL DESCRIPTION         U50       - Undisturbed Sample       Based on Unified         50 mm diameter       D       Disturbed Sample         D       - Disturbed Sample       Based on Unified         Classification System       Classification System         MC       - Moisture Content         HP       - Hand Penetrometer (UCS kPa)         VS       - Vane Shear; P-Peak,         R-Remouded (uncorrected kPa)         PBT       - Plate Bearing Test				CON REL VS S F St VSt H VL L MD D VD	ISISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense			
See l detai & bas	Expl s of sis o	anator abbre f desc	Viations	s for s					SMEC AUSTRALIA					

PROJECT       : Roma Flood Mitigation         LOCATION       : Roma									AL LOG Pty Ltd			<b>P</b> Fil SH	<b>IT N</b> LE / J <sup>i</sup> HEET	<b>O</b> : OB NO: : 1 OF 1	<b>TP08</b> 30031182		
POSITIO	)N :	E: 677	725.09	B, N: 70	63278.	566 (	55 MGA94)			SURFACE ELEV	ATION : 302.4	494 (/	AHD)				
			: KUB(	JTA Exe 13	cavator	· 4T				METHOD : Exca	avator CM				C	HECKER	BY ISM
EXCAVA			NSIONS	5 : 0.6	0 m WI	DE					2141 						
	DF	RILLIN	G	1						MAT	ERIAL	1				1	
VE E PENETRATION H	PENETRATION SUPPORT SUPPORT SROUD WATER LEVELS SAMPLES & FIELD TESTS FIELD TESTS FIELD TESTS CRAPHIC DEPTH (m) CRAPHIC LASSFICATION SYMBOL				So	oil Type	MATERIAL D e, Colour, Plasticit Secondary and M	ESCRIPTION y or Particle Characte inor Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	2 1520	& O	STRUCTURE ther Observations		
				- 0.0		СІ	Silty CLA grey brow 0.15m 0.20m becoming	AY: very wn, trac	stiff to hard, medi es of fine to mediu  shading lighter an	um plasticity, grey brow Im grained sand, grass  d less greyish brown	wn/ slight dark s and rootlets to	٥	VSt to H			ALLUVI	UM
						СІ	brown to >20mm d	, slightly diamete	greyish brown, sc r	me white powdery cale			т				-
				- - - - - - - - - - - - - -		CI	1.20m light brow and stain	 wn to br ning with	own/ slight grey br	own, some dark grey t lodules	o black mottling	D to M	-				-
				9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.00		SC	2.00m Clayey SJ orange by possibly g 2.60m SAND: do SAND: of f	GAND: d prown, lc grading	ense to very dens w plasticity fines, clayey sand/ sand	e, fine to medium grain high silt content, varyin dy clay in parts	ned sand, yellow/ ng fines content	_	VD to D				-
				- - - - - - - - - - - - - - - -		SM	3.00m		P08 TERMINATE	D AT 3 00 m			٥				
				- 5        			No groun Test pit b	nd water backfille	r encountered. d on completion.								-
		RAPHS		- بن 4.0 WYES													
METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Buildozer Blade       R     Ripper       SUPPORT     T       T     Timbering		istance er nown	SAMPLES & FIELD TESTS     CLASSIFICATION SYMBOLS       be     U50 - Undisturbed Sample 50 mm diameter     Based on Unified Classification System       D - Disturbed Sample     Based on Unified       MC - Moisture Content     MOISTURE       HP - Hand Penetrometer (UCS kPa)     D - Dry       VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa)     W - Wet			S &	& CONSISTENCY/ RELATIVE DENSITY VS - Very Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loc L - Loose MD - Medium D - Dense VD - Very Der										
See Expl details of & basis o	anator abbre f desc	/ Notes /iations iptions	s for S					S	MEC AU	STRALIA							SMEC

EXCAVATION - GEOLOGICAL LOG         PIT           PROJECT : Roma Flood Mitigation         CLIENT : Ostwald Bros Pty Ltd         FILE           LOCATION : Roma         FEATURE :         SHE										<b>PIT NO</b> FILE / JO	D : <b>TP09</b> DB NO : 30031182	
POS EQU DATI	ITION IPME E EX(	N : I NT T CAVA	E: 677 YPE	796.27 : KUB : 5/11/	0, N: 70 OTA Exc 13	63175. cavator	149 (! 4T	5 MGA94) SURFACE ELEVATION : 303 METHOD : Excavator LOGGED BY : CM	CH	HECKED BY : JSM		
EXC	AVAT	TION	DIME	NSION	S : 0.60	) m WI	DE					
2	2	DF	RILLIN ≝	G ~~ w	7 0		z	MATERIAL		×		
VE E denetratio		SUPPORT	GROUND WATE LEVELS	SAMPLES 8 FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	요 5 101520	STRUCTURE & Other Observations
					- 0.0		SM	Silty SAND: medium density to dense, light brown to brown, traces of fine to medium grained sand, grass and rootlets to 0.15m, occasional tree roots to 60mm diameter	e D	MD to D		ALLUVIUM
					03.5		SM	0.30m				-
					0.5 -		sc	Clayey SAND: dense, fine to medium grained sand, light brown grey with some orange brown in parts, some silt content low to medium plasticity fines		٥		
							sc	0.70m becoming very dense, light brown to slightly light grey brown, increase in clay content tending clayey sand/ sandy clay in parts				-
					1.0 —		sc	1.00m increasing clay content with depth		Q		
					302.5			1.30m Sandy CLAY: hard, medium plasticity, light brown and slight orange and grey brown in parts, fine grained sand fissured in parts, black staining/ veneer along fissures	_			-
					1.5				D to M			
					302.0		CI					-
					-			2.20m		т		-
					- - 3012 2.5 -			increase in plasticity, higher moisture content				-
					-		CI					-
		_			- <u>30</u>			3.00m EXCAVATION TP09 TERMINATED AT 3.00 m				
					- - 2			Test pit backfilled on completion.				-
					0.5							_
												-
					 4.0							-
	PH NC	IUTOG DTES	KAPHS		] YES		[	<u>м</u> мо				
ME N E BH R SUF	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     ↓10 Oct., 73 Wate			TION 	lo Resi 3 Wate Date sl	tance SAMPLES & FIELD TESTS CLASSIFICATION SYMBOLS & SOIL DESCRIPTION 50 mm diameter D Disturbed Sample B Disturbed Sample B Bulk Disturbed Sample MC Moisture Content HP Hand Penetrometer (UCS kPa) own VS Vane Shear; P-Peak, R-Remouded (uncorrected kPa)				INSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense		
See I detail & bas	Explai s of a sis of	natory abbrev desci	V Note Viation	s for s	<b></b> ◀ w	ater out	low	SMEC AUSTRALIA			VD	- Very Dense

PRC LOC	PROJECT : Roma Flood Mitigation LOCATION : Roma CLIENT : Ostwald Bros Pty Ltd FEATURE :										F	PIT N FILE /	<b>NO</b> : JOB I T : 1	: <b>TP10</b> NO : 30031182 OF 1		
POS		N :	E: 67	7879.33 · CAT	1, N: 70 Excavat	62988.	084 (	55 MGA94) SURFACE ELEVATION : 302.	324 (A	AHD)						
DAT	TE EX	(CAV	ATED	: 2/10/	13	01 51		LOGGED BY : CM				(	CHE	CKED BY : JSM		
EXC	CAVA			NSION	S : 0.60	0 m WI	DE	MATERIAL								
	N		E E	_∞Ω	Z (	0	NO	MATERIAL		ک						
L VE	E PENETRATI H	SUPPOR	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIC (RL) DEPTH (n	GRAPHIC LOG	CLASSIFICAT SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components			5 1	요 0152	20	STRUCTURE & Other Observations		
					-		сн	Silty CLAY: hard, high plasticity, dark grey brown, less than trace of fine grained sand, grass rootlets to 0.25	٥	-		-	A	LLUVIUM		
				0.30m B 0.60m	- 3050		сн	shading brown to dark brown, slightly darker grey brown, trace to no fine grained sand, occasional fissuring						-		
				0.70m B 1.00m	 - - - 1.0 -		сн	9.70m		т				-		
								1.20m shading light brown/yellow brown, increasing moisture content, trace of fine grained sand	Q					-		
					- - - - 2.0		СН							-		
				2.20m B 2.50m	 0.0000000000000000000000000000000000		СІ	2.20m Sandy CLAY: hard, medium plasticity, yellow brown, fine grained sand	W - Q	т				-		
006.2.0 00.21 +				2.70m B 3.00m	   		sc	<ul> <li>2.70m</li> <li>Clayey SAND: very dense, fine grained, yellow brown, high clay fines content, grading clayey sand/sandy clay in parts.</li> <li>3.00m</li> </ul>	M - D	đ						
								EXCAVATION TFT0 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.						-		
	P	HOTOG	RAPHS	S	4.0		[	NO								
	METHOD       PENETRATION       SAMPLES & FIELD TESTS         N       Natural Exposure				stance SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	TION SY ESCRIP I on Unit ation Sy	<b>MBOL</b> TION ied rstem	S &	C F S S F S V H V L L M L	CONS RELA /S St /St / /L / /D /D	ISTENCY/ TIVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense					
deta & ba	Expla ails of asis of	anator abbre f desc	y Note viatior ription	es for ns s.				SMEC AUSTRALIA				•				

EXCAVATION - GEOLOGICAL LOG       PIT NO :       TH         PROJECT : Roma Flood Mitigation       CLIENT :       Ostwald Bros Pty Ltd       FILE / JOB NO :       SUC         LOCATION : Roma       FEATURE :       Ostwald Bros Pty Ltd       FILE / JOB NO :       SUC											O: TP11 OB NO : 30031182 : 1 OF 1		
P			E: 67	8076.58	9, N: 70	62915.	051 (	55 MGA94) SURFACE ELEVATION : 301.	050 (A	AHD)			
D	ATE	EXCA	ATEC	): 1/10/	Excava 13	101 31		LOGGED BY : CM				С	HECKED BY : JSM
E	(CA)	/ATIO			S : 0.6	0 m WI I	DE	ΜΑΤΕΡΙΑΙ					
	NO			~S 20 20 20	Z G	ι <sub>Ο</sub>	NOI		шz	∑ U			
ΛE	E F	асаан» н	GROUND WA	SAMPLES FIELD TES	ELEVATIC (RL) DEPTH (n	GRAPHIC	CLASSIFICAT SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURI	CONSISTEN RELATIVE DENSITY	5 10	1520	STRUCTURE & Other Observations
					- 0.0 -			Silty CLAY: hard, high plasticity, slightly dark grey brown, less than trace of fine grained sand, grass rootlets to 0.15, occasional tree roots to 15mm	0				ALLUVIUM
				0.20m B			СН	thick		-			-
				0.40m	-			0.40m					-
				0.50m	0.5-			becoming medium to high plasticity, light brown to brown, with white powdery calcite nodules <20mm diameter				Ľ	-
				в	300.5		СІ-СН					_	-
				0.70m				0.70m light brown, trace of fine grained sand, occasional, white powdery calcite	-			┯	-
				0.80m B				nodules <10mm diameter					-
				1.00m	- 1.0		сі-сн						_
					300.0								-
				1.20m B				1.20m            light brown, slight yellow brown, trace of fine grained sand	-				-
									Σ				-
				1.50m	1.5								_
					299.5		сі-сн						-
					-					-			-
					-								-
				2.00m	2.0			2.00m	_				2 00: slow diaging, change
				ľ	299.0			<20mm diameter and occasional shiny, coarse sand sized crystals					to 0.3m bucket
				2.30m	-								-
							СН						-
					2.5 -								_
					298.5			2.60m		-			-
000					-								-
56 8.2.9							СП						
14 12:5					3.0 —			3.00m	_ Σ				
2/03/20					298.0		СН	grained sand					-
e> 1								3.30m					
awingF					-			EXCAVATION TP11 TERMINATED AT 3.30 m No ground water encountered. Test it backfilled on completion					
J << D					3.5 —	-							-
/EE.GP					297.								-
OD LEV													
IA FLO					-	-							-
T ROM		PHOT	GRAPH	s	⊥ <sub>4.0</sub>	I	r						
EST PI		NOTE	5		J 1ES				TION SY	MBOI	S &	CC	DNSISTENCY/
Log T	METH		Typogur	_   F	PENETRA	TION		SAMPLES & FIELD TESTS SILLASSIFICA US0 - Undistructed Sample Base	SCRIP	TION		RE	S - Very Soft
JR.GLB	E Existing Excavation BH Backhoe Bucket D - Disturbed Sample						ation Sy	stem		S F	- Soft - Firm - Stiff		
COLOU	B Bulldozer Blade B - Bulk Disturbed Sample MOISTI R Ripper WATER MC - Moisture Content									St - Very Stiff - Hard			
ARY - (						0 Oct., 7 evel on I	3 Wate	er HP - Hand Penetrometer (UCS kPa) M - Moist Nown VS - Vane Shear; P-Peak, W Wet				VL L	- Very Loose - Loose
AE LIBR	T	Timberi	g			ater inflo	w low	R-Remouded (uncorrected kPa) PBT - Plate Bearing Test					- iviedium Dense - Dense D - Very Dense
RISBAN Se	e Ex	planat	ory Not	es for	•	ator oul							
B de &	tails basis	of abb of des	eviation cription	ns Is.				SMEC AUSTRALIA					SMEC

EXCAVATION - GEOLOGICA           PROJECT : Roma Flood Mitigation         CLIENT : Ostwald Bros F           LOCATION : Roma         FEATURE :	PIT NO:         TP12           Pty Ltd         FILE / JOB NO:         30031182           SHEET:         1 OF 1														
POSITION         : E: 678268.228, N: 7062884.459 (55 MGA94)         SURFACE ELEV.           EQUIPMENT TYPE         : CAT Excavator 3T         METHOD         : Excavator 3T           DATE EXCAVATED         : 1/10/13         LOGGED BY : C	ATION: 301.462 (AHD) avator CM CHECKED BY: JSM														
EXCAVATION DIMENSIONS : 0.60 m WIDE DRILLING MATH	ERIAL														
MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Character Secondary and Minor Components	rristic NORLILOR STRUCTURE NORLILOR OD WOLLION OD WOLLION OD STRUCTURE & Other Observations 5 101520														
0.10m     Silty SAND: medium dense to dense, fine grained, light b some clay fines, grass and rootlets to 0.2m       0.30m     SM       B     SM       0.30m     SM       B     SM       0.30m       B       Silty CLAY: hard, medium to high plasticity, light brown to occasional white powdery calcite nodules	orown to brown,														
0.60m															
1.00m     0     1.0       B     0     1.0       Image: State of the state of	ad, less frequent														
light brown slight grey brown, increasing moisture content	nt Σ .														
2.60m       2.60m         B       Cl	rains, yellow														
2.90m 2.90m 2.90m D Silty SAND: very dense, fine to medium grained, yellow b fines	brown, some clay														
3.20m     3.20m       State     State       State															
PHOTOGRAPHS YES NO															
METHOD       PENETRATION       SAMPLES & FIELD TESTS         N       Natural Exposure       > u u ± →       > u u ± →         E       Existing Excavation       BH       Backhoe Bucket       Bulldozer Blade         B       Bulldozer Blade       WATER       D       Disturbed Sample         SUPPORT       T       Timbering       10 Oct., 73 Water       Use on Date shown water inflow         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       PBT - Plate Bearing Test	CLASSIFICATION SYMBOLS & SOIL DESCRIPTION         CONSISTENCY/ RELATIVE DENSITY           Based on Unified Classification System         VS         - Very Soft           MOISTURE         VSt         - Very Stiff           D         - Dry         VSt         - Very Stiff           M         - Moist         VL         - Very Stiff           W         - Wet         - Dense         VL         - Loose           W         - Wet         - Dense         VD         - Very Dense														
See Explanatory Notes for details of abbreviations & basis of descriptions.	SMEC														
PR LO	EXCAVATION - GEOLOGICAL LOG       PIT         OJECT       : Roma Flood Mitigation       CLIENT       : Ostwald Bros Pty Ltd       FILE         CATION       : Roma       : E: 678562.752, N: 7062933.563 (55 MGA94)       SURFACE ELEVATION : 301.237 (AHD)       SURFACE ELEVATION : 301.237 (AHD)											T NC E / JC EET	D: <b>TP13</b> DB NO : 30031182 : 1 OF 1		
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PO	SITIC	N :	E: 67	8562.752	2, N: 70	62933.563 (	(55 MGA94)			ATION : 301.2	237 (A	HD)			
DA	UIPN TE E	IEIN I XCAV		: CAT	⊨xcavat I3	or 31			LOGGED BY : C	M				C⊦	IECKED BY : JSM
EX	CAVA		DIME	INSIONS	6 : 0.60	) m WIDE									
	Z	DI	≺ILLI!  ≝	vG ⊗ ຍ	z ~	, IS			MATE	RIAL		2			
VE	e penetratio F H	SUPPORT	GROUND WATE LEVELS	SAMPLES	ELEVATIOI (RL) DEPTH (m	GRAPHIC LOG CLASSIFICATIO SYMBOI		MATERIAL DE Soil Type, Colour, Plasticity Secondary and Mir	SCRIPTION or Particle Character nor Components	istic	MOISTURE	CONSISTENC RELATIVE DENSITY	වි 5 101	520	STRUCTURE & Other Observations
					- 0.0		Sandy C grains, g	CLAY: hard, low plasticity, b grass and rootlets to 0.15m	rown, fine grained san	d, some medium	۵				ALLUVIUM
				0.40m	301.0		0.40m				M - 0	т			
				В	0.5		Silty CL/ of fine g	AY: hard, high plasticity, slig rained sand	ghtly dark grey brown,	less than trace					_
				0.60m			0.80m				2				
				0.90m B	1.0		light bro <10mm	wn to brown, occasional tre diameter	e roots, white powder	y calcite nodules		т		Π	0.80: slow digging, change to 300mm bucket
				1.10m			1.20m								
				<u>1.30m</u> B	- 300.0		Sandy C	CLAY: hard, medium plastici	ity, light brown, fine gr	ained sand					
				1.50m	1.5		1.60mgrading	higher silt content, increasi	ng moisture content						
											W - 0	т			
						2.00m increasir	ng sand content, grading fir	 ne to medium grained		_				2.00: very slow digging	
					- 299.0		2.30m grading	sandy clay/clayey sand in p	 arts		_				
					-2.5		EXCAV/ No grou Test pit	ATION TP13 TERMINATED nd water encountered. backfilled on completion.	AT 2.50 m						
2:57 8.2.900					- 598.5										
12/03/2014					3.0										-
DrawingHIE>>					- 598.0										
NEE.GPJ <<					3.5										-
MA FLOUD LE					297.5										
		PHOTO NOTES	GRAPH	s	⊥ <sub>4.0</sub> —   YES	ļ	NO				1				I
ANE LIBRARY - COLOUK GLB LOG 1ES 1F	MOTES     YES       METHOD     Natural Exposure       Existing Excavation     Sublidozer Blade       R Ripper     No Resist       SUPPORT     Timbering						sistance ter shown	SAMPLES & FIELD 1         U50       -         Undisturbed         50 mm dian         D       -         Disturbed S         B       -         BUR Disturbed         MC       -         MOS -       Moisture Co         HP       -         Hand Penel       VS         VS       -         Vane Shear         R-Remoude         PBT       -	TESTS I Sample teter ample ed Sample ontent (UCS kPa) (; P-Peak, d (uncorrected kPa) ng Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIPT on Unif ation Sy	MBOLS FION ied stem	\$ &	CO REI VS S F St S F S VS H VL L D D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
MEC BKIN det & b	e Expl ails of asis o	anator abbre f desc	y Note viatior ription	es for ns is.				SMEC AU	STRALIA						

PRO LOC	JEC <sup>.</sup> ATIC	T: N:	Roma Roma	Flood N	vlitigatio	n		EXCA	٩V/	<b>TION - GE</b>	EOLOGICA : Ostwald Bros F	AL LOG Pty Ltd			<b>PI</b> FIL SH	<b>T N(</b> E / J( EET	<b>D</b> : <b>TP14</b> DB NO : 30031182 : 1 OF 1
POS		N :	E: 678	3742.62	7, N: 70	62959.	463 (	55 MGA94)				ATION: 301.4	400 (A	AHD)	0.1		
DAT	E EX	CAV		: 1/10/	Excava 13						LOGGED BY : C	CM				CH	HECKED BY : JSM
EXC	AVA	TION DI		INSION	S : 0.6	0 m WI	DE				MATI	FRIAI					
Ž	5	ح	TER	sTS STS	NO Ê	υ	NOIL .						₩ N	× ≣^C			
LVE E DENIETDAT	H F	SUPPOF	GROUND W	SAMPLES FIELD TES	ELEVATI (RL) DEPTH (	GRAPHI LOG	CLASSIFICA SYMBOI	s	Soil T	MATERIAL DE ype, Colour, Plasticity Secondary and Mi	SCRIPTION or Particle Character nor Components	ristic	MOISTUF	CONSISTE RELATIV DENSIT	5 10 <sup>-</sup>	1520	STRUCTURE & Other Observations
					-		CL	Sandy C grading	CLAY g claye	: hard, low plasticity, li y sand in parts, some	ght brown to brown, fi silt content, grass and	ne grained sand, d rootlets to 0.3m		т			ALLUVIUM -
				0.30m B 0.50m	0.5	/////     	sc	0.30m Silty SAI some sil	AND: c silt fine	lense to very dense, ç s, dry	grey brown, light browr	n/yellow brown,		D- VD			-
					-		CL	0.60m Sandy C grading	CLAY g claye	hard, low plasticity, li y sand in parts, some	ght brown to brown, fi silt content, grass and	ne grained sand, d rootlets to 0.3m	_				-
					- 9.00 1.0 –			0.90m varying s	sand	content, grading silty	clay some fine grained	d sand in parts		_		Π	-
	Ci <u>1.20m</u> B <u>becoming n</u> content, hig								ing me	dium plasticity, orang	e brown, higher fine g	rained sand	_				1.20: slow digging, change
	1.50m							content,	t, high	er moisture content			2	т			
								1.80m					- D				-
	sc					sc	shading nodules	g light s	brown/yellow brown, d	occasional white powd	lery calcite					1.80: easier digging - -	
	2.30m						2.20m Clayey S brown, s	SANE	): dense to very dense silt fines	e, fine grained, light br	own/yellow					-	
				2.60m	0. 667 2.5 —		sc						Μ	۵۷			
					-			2.70mslightly h	highe	r silt content				D			-
		_			- 5.8.5 - 0.6-7			3.00m EXCAVA No grout	/ATIO	N TP14 TERMINATE	0 AT 3.00 m						-
b					-	-		Test pit	L DACK	med on completion.							-
	8 8 3.5																-
																	-
																_	
	NOTES YES NO							NO NO				CLASSIFICAT		(MBOI 9	s& I	00	DNSISTENCY/
ME N BH R SUI	METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering METHOD PENETRATION Support 10 Oct., 73 Wate Level on Date si water inflow water outflow					lo Res 73 Wat Date s ow	istance er hown		SAMPLES & FIELD U50 - Undisturber 50 mm diar D - Disturbed S B - Bulk Disturf MC - Moisture C, HP - Hand Pene VS - Vane Shea R-Remoud PBT - Plate Beari	TESTS d Sample neter sample bed Sample ontent trometer (UCS kPa) r; P-Peak, ed (uncorrected kPa) na Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	on Unit ation Sy	TION fied /stem	ο α	CREVS FStSF L D V	ILATIVE DENSITY LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Dense - Dense - Very Dense	
See l detai & bas	T Timbering water inflow water outflow ee Explanatory Notes for tails of abbreviations basis of descriptions.									SMEC AU	STRALIA						

PRO.	JECT	Г:I N:I	Roma Roma	Flood N	Vitigatic	n		EXCA	VATION - GEOLOGICA CLIENT : Ostwald Bros FEATURE :	AL LOG Pty Ltd			<b>PIT</b> FILE SHE	<b>NC</b> :/JO	D : <b>TP15</b> B NO : 30031182 1 OF 1
POS	TION	N :	E: 678	3861.51	3, N: 70	62955.	674 (	55 MGA94)	SURFACE ELE	/ATION : 301.2	265 (/	AHD)			
EQU	IPME = FX			: CAT	Excava	tor 3T				avator CM				СН	
EXC		TION		NSION	S : 0.6	0 m W	DE		LOGGED BT					CII	
		DF	RILLIN	IG			1	1	MAT	ERIAL	1				
VE E PENETRATION	LΙ	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DFPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characte Secondary and Minor Components	eristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	වි 5 1015	520	STRUCTURE & Other Observations
					- 0.0 -			Sandy CL some me 10mm thio	LAY: hard, low to medium plasticity, brown, fine edium grains, grass and rootlets to 0.3m, occasi ick to 0.8m	e grained sand, ional tree roots to	۵	т			ALLUVIUM -
				0.30m B 0.50m	- 0.5 -		CL-CI	0.60m decreasin	ng sand content		M - D	H VSt-H			- - - -
					1.0 –		CL-CI	0.80m Silty CLA' grained s	Y: hard, low to medium plasticity, brown, trace sand	to some fine					- - - -
	1.40m B 1.5 - Cl 1.60m 1.5 - Cl 1.60m 1.5 - Cl 1.60m 1.5 - Cl 1.60m 1.5 - Cl 1.60m Cl 1.90m						 CI	1.30m becoming trace of fi	g medium plasticity, light grey brown and orang line grained sand, slightly higher moisture conte	e brown, mottled, ent	M - D	т			- - -
	1.70m B 1.90m 2.0 Cl 2.20m						CI	shading c change to 2.00m Sandy CL	orange brown, slightly higher moisture content, o 0.3m bucket LAY: hard, low plasticity yellow/light brown, fine	slow digging,	5				-
	2.0 2.0 2.0 CL 2.20m B B B B						CL	2.20m Clayey S/ light brow	AND: dense to very dense, fine grained, some vn/yellow brown	medium grains,	- _	T			-
				2.40m	2.5 -		sc	2.60m			Σ	Q			-
				2.80m B	298.5		CL-CI	Silty CLA	AY: hard, low to medium plasticity, light brown/ye ned sand, high silt content	ellow brown, some	- W	г			-
		_		3.00m	3.0 -			3.10m							
								No groun Test pit b	ackfilled on completion.						- - -
	297.5														-
PHOTOGRAPHS YES NO															
ME N B B R SUF	PHOTOGRAPHS NOTES YES METHOD N Natural Exposure Existing Excavation BH Backhoe Bucket 3 Bulldozer Blade R Ripper SUPPORT 「 Timbering YES PENETRATION Su u ⊥ ± → No Resident WATER 10 Oct., 73 Wate Level on Date shi water inflow water outflow						Io Res 3 Wat Date s ow flow	NO istance er hown	SAMPLES & FIELD TESTS         U50       -       Undisturbed Sample         50       mm diameter         D       -       Disturbed Sample         B       -       Bulk Disturbed Sample         MC       -       Moisture Content         HP       -       Hand Penetrometer (UCS kPa)         VS       -       Vane Shear; P-Peak, R-Remouded (uncorrected kPa         PBT       -       Plate Bearing Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	CION SY SCRIP on Uni ation Sy	/MBOLS TION fied /stem	5&	COI REL VS S F St VSt H VL L MD D VD	NSISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See E detail & bas	Timbering water inflow e Explanatory Notes for tails of abbreviations basis of descriptions.								SMEC AUSTRALIA	I					

File: 30031182 TP15 Page 1 OF 1

PROJ LOCA	ECT TIOI	: I N : I	Roma Roma	I Flood N	litigatio	n		E	XCA	VA	<b>TIC</b>	ON CLIE FEAT	- G	EOL : Ost	OGIC wald Bros	SAL s Pty	Ltd	G				<b>PI</b> FILI SHI	T N E / J EET	<b>O</b> : OB NO : 1 O	<b>TP16</b> : 3003118 F 1	82	
POSI EQUII DATE		I : NT T CAV	E: 679 YPE ATED	9049.636 : CAT   : 30/9/1	6, N: 70 Excavat 13	62889.8 or 3T	809 (	55 MC	GA94)					SURF METH LOGG	ACE ELE OD : Ex ED BY :	EVAT kcava CM	TON : ator	300.3	86 (A	AHD)			CI	HECK	ED BY :	JSM	
EXCA	VAI	ION DF	DIME	NSIONS	5 : 0.60	) m WI	DE								MA	TER	IAL										
VE E PENETRATION	L T	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL		So	ioil Ty	'pe, C Sec	MATE olour, condar	RIAL D Plastici y and M	ESCRIP ty or Part /linor Cor	TION icle Charac nponents	cteristi	ic		MOISTURE	CONSISTENCY RELATIVE	5	о ОС 101	520		STRUC & Other Ob:	TURE	s
				0.20m B 0.40m	- 0.0		сі	0.50m	Sandy CL and rootle decreasir	LAY: lets to	very s , 0.2m	stiff, me 1, occa 1, occa 1, occa	edium p sional tr	lasticity, l	prown, fine g to 10mm thi	graine ick to (	ed sand, y 0.4m	grass	۵	VSt				ALL	JAIAW		
					- - - - - - - - -		СІ	0.70m	Silty CLA sand	λY: ha	ırd, me	edium	plasticit	ty, brown,	trace to so	ome fin	ne graine	d									- - - - -
	1.30m         1.30m         becomparise           B         0         1.30m         becomparise           1.50m         1.5         CH         CH           1.60m         1.60m         becomparise         becomparise           B         0         1.60m         becomparise           B         0         1.60m         becomparise           1.80m         0         1.60m         becomparise           1.80m         0         1.90m         0								becoming parts becoming higher mo	g high g high g mec loistur	1 plast	 ticity, d	lark gre	y brown, t	trace to no f	fine gr	rained sa		M - D	т				1.30 to 30	: slow diggir )0mm bucke	ng, change et	e - - -
	1.80m 1.90m B 2.10m 2.10m CI-CH 1.90m 2.0 CI-CH 1.90m CI-CH 2.0 CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH CI-CH								light g	irey bi	rown												2.20		diaging	-	
	2.10m - 2.2 - CI-CH - CI-CH - 2.4 CI-CH 2.5 - CI-CH 2.5 - CI-CH 2.5 - CI-CH 2.7 - CI-CH 2.7 - CI-CH 2.7 - CI-CH 2.7 - CI-CH - C						2.40m 2.50m 2.70m	higher silf fine grain increasing Sandy CL medium g	It confined satisfies and sati	tent, s and id con hard, ed sar	ntent low pla	g light g  asticity, h silt co	rey browr  light grey ntent, gra	brown/yello	ow bro  ow bro d sand	own, trace	e of  to						2.20	. very slow (	aigging		
> 12/03/2014 12:57 8.2.900									EXCAVA No groun Test pit b	ATION nd wa backfil	TP16 ter en lled or	6 TERM counte n comp	MINATE ered. bletion.	ED AT 2.7	0 m												-
AVEC CD4 AVEC D4 AVEC																				- - -							
PHOTOGRAPHS YES NO														-													
NOTES       PENETRATION       SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS & SOIL DESCRIPTION       CON REL Based on Unified Classification System       CON REL         N       Natural Exposure E       Existing Excavation BH Backhoe Bucket B       PENETRATION       SAMPLES & FIELD TESTS       U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B       CLASSIFICATION SYMBOLS & Based on Unified Classification System       VS S         WATER       WATER       U50 - Undisturbed Sample B       MOISTURE VS       VS S         SUPPORT T       T       Timbering       D Oct., 73 Water Level on Date shown water inflow water outflow       PBT - Plate Bearing Test       MOIst       U       U											DNSIST ELATIV St 	ENCY/ E DENSITY - Ver - Soft - Firm - Stiff - Ver - Har - Ver - Loo - Met - Der - Ver - Ver	Y y Soft t n f y Stiff rd y Loose vse dium Den: nse y Dense	se													
See E details & basi	xplai s of a is of	hator bbre desci	y Note viatior ription	es for ns s.						ę	SM	IEC	; AL	JSTF	RALIA	\						_	_			SME	C

PROJ LOCA	PROJECT : Roma Flood Mitigation       CLIENT : Ostwald Bros Pty Ltd         OCATION : Roma       FEATURE :         POSITION : E: 679101.596, N: 7062708.730 (55 MGA94)       SURFACE ELEVATION : 300.920 (AHD)												<b>P</b> FI SI	<b>IT N</b> LE / J HEET	O: <b>TP17</b> IOB NO : 30031182 : : 1 OF 1	
POSI		:	E: 679	0101.596	3, N: 70	62708.	730 (	55 MGA94)		SURFACE EL	EVATION : 300	.920 (/	AHD)			
EQUI				: CAT I	Excavat	or 3T					· CM				C	
EXCA	VAT	ION	DIME	NSIONS	S : 0.60	0 m WI	DE			LOOGLD DI	. 0111				0	
		DF	RILLIN	IG	1					M	ATERIAL		1			
VE E PENETRATION	LI	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	So	MATE bil Type, Colour, ' Secondar	RIAL DESCRIPTION Plasticity or Particle Chara y and Minor Components	acteristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	වි ) 1520	STRUCTURE & Other Observations
					- 0.0			Sandy Cl sand, sor	AY: hard, mediun ne medium grains	m plasticity, light brown to t s, grass and rootlets to 0.3r	brown, fine grained m	۵				ALLUVIUM
				0.40m B 0.60m 0.80m	 - <u>-</u> - <u>-</u>   		Сі	0.70m becoming diameter		ne white powdery calcite no		_	т			
				B 1.00m	0. - 00 1.0 - - -		СІ					W - Q				1.20: slow digging, change to 300mm wide bucket
				1.40m B 1.60m	- 1.5 86 1.5 		ci	<u>1.40m</u> <u>becoming</u> grained s nodules,	Jow to medium p and, some mediu <10mm diameter	plasticity, shading light brow m grains, occasional white , some silt content	vn/yellow brown, fine powdery calcite					
				2.20m B 2.40m	- 2.0		CI	2.00m increasing 2.20m Silty SAN higher mo	g sand content, w	vithout white nodules	y sand in parts,	WQ	н			
12:57 8:2:900					~ 2.5		CL-CI	2.60m slightly hi 3.00m	gher moisture co	ntent		-				
DrawingFile>> าะเบงะบ								EXCAVA No groun Test pit b	TION TP17 TERM d water encounte ackfilled on comp	/IINATED AT 3.00 m red. Jletion.						
A FLOOD LEVEE. GP 3 3					~ 3.5 — - - - -											
	PHOTOGRAPHS NOTES     YES     NO       METHOD     YES     NO       METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering       See Explanatory Notes for								SAMPLES & U50 - Uni 50 D - Dis B - Bul MC - Mo HP - Ha VS - Vai R-F PBT - Pla	FIELD TESTS disturbed Sample mm diameter turbed Sample k Disturbed Sample isture Content nd Penetrometer (UCS kP ne Shear; P-Peak, Remouded (uncorrected ki te Bearing Test	Pa) CLASSIFICA SOIL D Baser Classific MOISTURE D - Dry M - Moist W - Wet	TION SY ESCRIP d on Unit cation Sy	/MBOLS TION fied /stem	S &	CC Rt VS F St VS F St VS F U U U	ONSISTENCY/ ELATIVE DENSITY S - Very Soft - Firm t - Stiff St - Very Stiff - Hard L - Very Loose - Loose D - Medium Dense D - Medium Dense D - Very Dense
details & basi	xplar of a s of (	bbrev bbrev desci	/ Note /iation iptions	S TOP S S.					SMEC	AUSTRALIA	4					SMEC

PROJECT : Roma Floo LOCATION : Roma	d Mitigatior	ı	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :		<b>PIT NC</b> FILE / JC SHEET :	D : <b>TP19</b> DB NO : 30031182 : 1 OF 1
POSITION : E: 679083.	168, N: 706	62342.102 (	5 MGA94) SURFACE ELEVATION : 300.	014 (AHD)		
EQUIPMENT TYPE : CA	1 Excavat	or 3 l			CH	
EXCAVATION DIMENSIO	NS : 0.60	) m WIDE				
DRILLING			MATERIAL			
VE F PENETRATION H SUPPORT GROUND WATER EVELS SAMPLES &	ELEVATION ELEVATION C (RL) DEPTH (m)	GRAPHIC LOG CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY	요 5 101520	STRUCTURE & Other Observations
0.30m	- 0.0	SM	Silty SAND: medium dense to dense, fine to medium grained, light brown to brown, grass and rootlets to 0.25m, trace of clay fines	Q - QW		ALLUVIUM
0.60m	0.5 - 5662 - 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 5660 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 56600 566000 566000 56600 56600 56600 56600 566000 566000 566000 566000 566000 566000 566000 566000 566000 5660000 566000 566000 5660000 5660000 5660000000000 566000000000000000000000000000000000	(1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999) (1999)	becoming dense, light brown, less clay fines content, slightly moist	M - M		-
1.50m B <u>1.80m</u>	1.5	1	1.50m becoming light brown slight yellow/orange brown			
2.30m B 2.80m	2.5 2.67 	1	2.30m	- W - Q		-
	- 562 0	SP	3.00m higher moisture content, slightly higher fines content      3.30m      EXCAVATION TP19 TERMINATED AT 3.30 m     No ground water encountered.     Test in backfilled on completion			-
	3.5 — 986 — - 5 - 5 - 1					-
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	VES PENETRA	TION TION No Res O Oct., 73 Wate evel on Date sl ater inflow ater outflow	SAMPLES & FIELD TESTS       CLASSIFICA'         stance       U50 - Undisturbed Sample       Basec         0 - Disturbed Sample       Classific         B - Bulk Disturbed Sample       MOISTURE         MC - Moisture Content       D - Dry         HP - Hand Penetrometer (UCS kPa)       W - Moist         VS - Vane Shear, P-Peak,       R-Remouded (uncorrected kPa)         PBT - Plate Bearing Test       W - Wet	TION SYMBOLS ESCRIPTION I on Unified ation System	& CO REI VS S F St VS F VS U U D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff : - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.			SMEC AUSTRALIA		•	SMEC

PF	OJEC	CT :	Roma	Flood N	<i>l</i> itigatior	n		EXCAVATION - GEOLOGICAL LOG			<b>P</b> Fl	<b>ILE /</b>	<b>10</b> : JOB N	<b>TP20</b> O : 30031182 OF 1
PC	DSITIC	ON :	E: 679	9132.69	4, N: 70	62127.9	988 (5	5 MGA94) SURFACE ELEVATION : 299.8	373 (4	AHD)	3		1	<u>.</u>
EC D/ E>	QUIPN ATE E	IENT XCAV	TYPE ATED DIME	: CAT : 30/9/ NSION	Excavat 13 S : 0.60	or 3T ) m WI	DE	METHOD : Excavator LOGGED BY : CM				(	CHEC	KED BY : JSM
Ĺ		D	RILLIN	IG				MATERIAL			1			
Æ	PENETRATION	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	음 2 0152	0	STRUCTURE & Other Observations
Í					- 0.0	1. 		Silty SAND: dense to very dense, fine to medium grained, light brown to brown, grass and rootlets to 0.2m					AL	LUVIUM
				0.40m B	- - - - - - - - - - - - - - - - - - -		SM	0.60m	۵	D - U				-
								some clay fines, shading light brown to orange brown	-					
					-	10000) 1000-1	SM		- P	D				
					0.000 66 1.0 -		- - - - -	0.90m Silty SAND: very dense, fine to medium grained, orange brown, varying clay content grading clayey sand/sandy clay in parts						-
				1.40m	- 3.5		sc							
				B 1.60m	1.5 — 		•		M - D	Q				-
					- 0 867 2.0		sc	1.80m decreasing clay content, some silt fines, high moisture content, shading yellow/orange brown						-
					- - - - - - - - - - - - - - - - - - -		SM	becoming dense to very dense, fine to medium grained, yellow brown, some clay fines         2.40m         grading fine to coarse grained	× ۵					_
006				2.80m	-		SM	2.60m becoming moist	×	D - VD				
57 8.2				В	97.0	1								
> 12/03/2014 12				3.00m				3.00m EXCAVATION TP20 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.						
J < <drawingfile></drawingfile>					- 9.96 3.5 —									-
LOOD LEVEE.GF					-									
					- 500 4.0									
2		PHOTO NOTES	GRAPHS		] YES			NO NO						
BANE LIBRARY - COLOUR.GLB Log TEST	METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket 3 Bulldozer Blade R Ripper SUPPORT T Timbering T Timbering METHOD PENETRATION Suresistance WATER Level on Date shown water inflow water outflow							stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	ION SY SCRIP on Unit ation Sy	MBOL TION ied stem	S &		CONSIS RELAT /S /St /St /L /D /D	STENCY/ IVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
SWEC BRIST de & I	e Exp tails of basis o	lanator f abbre of desc	y Note viatior ription	es for is s.				SMEC AUSTRALIA						

PRO. LOCA	JECT ATIOI	- :   N_:	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :				<b>PIT</b> FILE SHEE	NC / JO :	): <b>TP22</b> B NO : 30031182
POS		۱ : NT -	E: 679	9218.00	8, N: 70	61737.	183 (	55 MGA94) SURFACE ELEVATION : 299.8	875 (A	AHD)				
DATE	E EX			: 30/9/	⊏xcavat 13	UF 31		LOGGED BY : CM					СН	ECKED BY : JSM
EXC/	AVAT	ION	DIME	NSION	S : 0.60	) m WI	DE							
z	Ę	DF	RLLIN R	lG ∞γΩ	z ~		NC	MATERIAL		X				
VE E penetratio	H H	SUPPORT	GROUND WATE LEVELS	SAMPLES {	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENC RELATIVE DENSITY	5 1	ං වි 1015	20	STRUCTURE & Other Observations
				0.30m B	- 0.0 +		SM	Silty SAND: dense, fine to medium grained, light brown, grass and rootlets to 0.3m	0	0				ALLUVIUM
				0.70m	- 0.5	   	SM	some clay fines		D - DM				-
				B			sc	Clayey SAND: medium dense to dense, fine to medium grained, orange brown, grading low plasticity sandy clay in parts, some light brown fine to medium grained silty sand pockets		D - DM				
				1.00m	- 1.0		sc	becoming very dense	-					
							sc	1.50m shading yellow/orange brown		Q				-
	2.50m B 2.70m				- 		sc sc	2.20m         occasional light grey brown silty clay pockets/lenses, <100mm thick, slight increase in plasticity	_	-				
	B 2.70m 2.80m B 3.00m 3.00m 3.0							2.80m shading light brown, grey brown and brown grey, slightly lower plasticity, fine to coarse grained sand, trace of fine to coarse rounded to subrounded gravels 3.00m increasing moisture content with depth	×					
	SC 3.3						SC	3.30m EXCAVATION TP22 TERMINATED AT 3.30 m No ground water encountered. Test pit backfilled on completion. DCP 77 carried out adjacent test pit.						
	PH	HOTOC	GRAPHS	<u> </u>	- 			× №						
, ME BH B R SUF	NOTES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering T Timbering NO Resista WATER 10 Oct., 73 Water Level on Date show water inflow water outflow							stance SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test CLASSIFICAT SOIL DE Based Classific MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unif tion Sy	<b>'MBOL</b> TION ied /stem	.S &		COI REL VS S F St VSt H VL L MD D VD	VSISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Vary Stiff - Vary Loose - Loose - Medium Dense - Dense - Very Dense
See E detail & bas	Explai Is of a sis of	nator Ibbre desc	y Note viatior ription	es for Is S.				SMEC AUSTRALIA						SMEC

PROJECT LOCATION	「∶Roma F N∶Roma	lood Miti	gatior	ı		EXCA	VATION - GEC CLIENT : ( FEATURE :	DECORECT	<b>LLOG</b> Pty Ltd			<b>PIT</b> FILE SHEE	NO: JOB NO	<b>TP23</b> 5 : 30031182 F 1
POSITION	N : E: 6792	70.704, I	N: 706 cavat	61544.9	963 (5	55 MGA94)	SU	RFACE ELEV	ATION: 299.0	)38 (A	HD)			
DATE EX	CAVATED :	29/9/13	cavat				LO	GGED BY : C	M				CHECK	ED BY : JSM
EXCAVAT	TION DIMEN	SIONS :	0.60	) m WI	DE			MATE	RIAL					
FENETRATION	SUPPORT ROUND WATER	SAMPLES & IELD TESTS	DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	So	MATERIAL DESC oil Type, Colour, Plasticity or Secondary and Minor	RIPTION Particle Character Components	istic	MOISTURE	ONSISTENCY RELATIVE DENSITY	5 1015	20	STRUCTURE & Other Observations
<u>&gt; u u z</u>	0	<u>оп</u>	0.0 —	<u>])</u>	ō	Clayey S	AND: very dense, fine graned	sand, brown, gras	s and rootlets to		0		ALL	UVIUM
	<u>о.</u> В	30m			sc	0.5m, gra	ang iow plasticity sandy day	in parts		D - M	D - VD			-
	0. 0. 0. 0.	60m gom			sc	0.60m Silty SAN fines 0.90m	ID: dense to very dense, fine	grained, light brow	m, trace of clay					
	B 1.	20m	1.0		SM	shading li	ight grey brown/cream, trace o	f clay fines			D - VD			-
	1. B	40m 60m			sc	Clayey S yellow/ora	AND: dense to very dense, fin ange brown mottling, low to m	e grained, light gre edium plasticity fin	ey brown, some es	×	D - VD		1 70	- slow diaging, change
	2.00m B 2.30m 2.30m 2.30m						ID: very dense, fine grained, s gh silt content, some clay fine	ome medium grain s	is, yellow/orange				to 30	Jomm wide bucket
12:5/ 8.2.900		206 F	2.5		SM	2.50m grading fi content	ine to medium grained, high si	It content, varying		2	ΔV			-
		298.0	3.0			3.10m EXCAVA No groun Test pit b DCP 81 c	TION TP23 TERMINATED AT d water encountered. ackfilled on completion. carried out adjacent test pit.	3.10 m						
ROMA FLUOU LEVEE.GTJ -		295 F	4.0											
METHOD N Natu BH Back BH Back R Ripp SUPPORT T Timb	HOTOGRAPHS DTES ural Exposure titing Excavation khoe Bucket dozer Blade ber T bering	WA		TION → D Oct., 73 vel on E ater inflo	o Resi 3 Wate Date sh w low	NO stance	SAMPLES & FIELD TES         U50       Undisturbed Sa         D       Disturbed Sam         B       Bulk Disturbed         MC       Moisture Conte         HP       Hand Penetrom         VS       Vane Shear; P         R-Remouded (in         PBT       Plate Bearing T	TS imple ole Sample nt neter (UCS kPa) Peak, uncorrected kPa) est	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unif ation Sy	MBOLS FION led stem	5&	CONSIST RELATIV VS S F St VSt H VSt L MD D VD	<b>TENCY</b> / <b>Fe DENSITY</b> - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See Explai details of a & basis of	natory Notes abbreviations descriptions.	for					SMEC AUS	TRALIA						

PRC LOC	JECT	· : F N : F	Roma Roma	Flood N	litigatior	n		EXCA	VATIO	<b>DN - GE</b> CLIENT FEATURE	: Ostwald Bros F	AL LOG Pty Ltd			PI FII SH	<b>IT NO</b> LE / JO HEET	<b>D</b> : <b>TP24</b> DB NO : 300311 : 1 OF 1	<b>l</b> 182
POS FOI		NTT	:: 679 YPF	241.10	5, N: 70 Excavat	61347.8 or 3T	358 (5	55 MGA94)				ATION : 299.0	)52 (A	AHD)				
DAT	EEX	CAVA	TED	: 29/9/1	13						LOGGED BY : (	CM				Cł	IECKED BY	: JSM
EXC	:AVA1	TION I DR		NSIONS G	5 : 0.60	) m WI[ 	DE				MAT	ERIAL						
	PENETRATION	SUPPORT	ROUND WATER LEVELS	SAMPLES & IELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	So	bil Type, Co Seo	MATERIAL DE olour, Plasticity ondary and Mir	SCRIPTION or Particle Characte nor Components	ristic	MOISTURE	ONSISTENCY RELATIVE DENSITY	5 10	1520	STRU & Other Of	CTURE oservations
<u> </u>			U	<u>ог</u>	0.0 -		0	Silty SAN	D: medium	dense, fine gra	ained, brown, some c	lay content, grass		0			ALLUVIUM	
				0.40m B 0.60m 1.10m B	- 586 5.0 		SM	0.60m becoming	g dense, hig g dense to v	gher moisture c	ontent, shading light l	brown to brown	0 - M	DM				
				1.30m	1.5		SM SM	brown, sc 1.80m decreasir 2.00m becoming	, me clay fin	s content, light	brown/yellow brown	ce of clay fines		D - D				
<pre><drawingfile>&gt; 12/03/2014 12:57 8.2.900</drawingfile></pre>	2.00m 2.00m 2.00m 2.00m 2.00m 2.00m 2.00m 2.00m 2.00m 56 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1					3.50m	, co i co i ini				Ψ	Q				-		
Z B C C C C C C C C C C C C C C C C C C	3.5     3.5     Image: Constraint of the second se							EXCAVA No groun Test pit b DCP 85 c	TION TP24 d water end ackfilled on arried out a SAMPI	I TERMINATED countered. completion. adjacent test pil	AT 3.50 m : : : : : : : : : : : : :	CLASSIFICAT SOIL DE Based	ION SY SCRIPT on Unif	MBOLS FION ied	5&	CC RE VS	UNSISTENCY/ LATIVE DENSIT - Ve	- - - - - - - - - - - - - - - - - - -
ALEC BRISBANE LIBRARY - COLOUR.GLB © 20 20 30 30 1 2 2 3 2 2 3 2 3 2 3 2 3 2 3 2 3 2 3 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	THOD     PENETRATION       Natural Exposure     Sinther Excavation       Existing Excavation     Sinther Excavation       Bulldozer Blade     WATER       PPORT     10 Oct., 73 Water       Timbering     10 Oct., 73 Water       Explanatory Notes for     water outflow							9r Iown	D - B - MC - HP - VS - PBT -	50 mm dian 50 mm dian Disturbed S Bulk Disturb Hand Penet Vane Shear R-Remoude Plate Bearin	elter ample ed Sample ntent rometer (UCS kPa) ; P-Peak, id (uncorrected kPa) ig Test	Classifica MOISTURE D - Dry M - Moist W - Wet	ation Sy	stem		S F St VS H VL L D VD	- SC - Fiir - Stit - Ve - Het - Ve - Ve - Do - Do - Do - Ve	oft m fff ard yry Stiff ard yry Loose ose ose ose odium Dense ense ery Dense

PROSITION:         E-6779122:271.N:7051153.022 (55 MGA04)         SUPPACE ELEVATION:         27447 (AHD)           DUMPENT IVEF:         CATE VOID         CHECKUN TED:         299913         CHECKUN TED:         CHECKUN TED:         CHECKUN TED:         CHECKUN TED:         299913         CHECKUN TED:         CHECKUN TED:         CHECKED BY : SM         SM         SM         SM <td< th=""><th>PROJECT : Roma Flood Mi LOCATION : Roma</th><th>EXCAN</th><th>/ATION - GEOLOGICAL LO CLIENT : Ostwald Bros Pty Ltd FEATURE :</th><th>G</th><th></th><th>PIT NC FILE / JO SHEET :</th><th><b>D</b> : <b>TP25</b> DB NO : 30031182 : 1 OF 1</th></td<>	PROJECT : Roma Flood Mi LOCATION : Roma	EXCAN	/ATION - GEOLOGICAL LO CLIENT : Ostwald Bros Pty Ltd FEATURE :	G		PIT NC FILE / JO SHEET :	<b>D</b> : <b>TP25</b> DB NO : 30031182 : 1 OF 1
EXCAVITON DMERISIONS : 0.60 m WDE       MATERIAL         Image: Construction of the second	POSITION : E: 679192.721 EQUIPMENT TYPE : CAT E DATE EXCAVATED : 29/9/13	: 7061153.992 (55 MGA94) avator 3T	SURFACE ELEVATION : METHOD : Excavator LOGGED BY : CM	297.447 (A	AHD)	СН	IECKED BY : JSM
B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B     B <td>EXCAVATION DIMENSIONS DRILLING</td> <td>0.60 m WIDE</td> <td>MATERIAL</td> <td></td> <td></td> <td></td> <td></td>	EXCAVATION DIMENSIONS DRILLING	0.60 m WIDE	MATERIAL				
1       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0	E PENETRATION SUPPORT SECUND WATER LEVELS SAMPLES & SAMPLES &	DEPTH (m) GRAPHIC LOG LOG SYMBOL SYMBOL	MATERIAL DESCRIPTION il Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY 2	ලි 101520	STRUCTURE & Other Observations
1.50m       0       1.50m       0       1.50m       Sandy CLAY: hard, low to medium plasticity, grey brown, some yellow       1.50: easier digging         1.70m       8       Sandy CLAY: hard, low to medium plasticity, grey brown, some yellow       1.50: easier digging         1.80m       9       2.00m       9       2.00m       1.50: easier digging         2.00m       9       2.00m       9       2.00m       2.10m       2.10m         2.00m       9       2.0       CL       2.10m       2.10m       2.10m         2.00m       9       2.5       M.       M.       2.40m       2.10m         2.00m       9       2.5       M.       M.       2.40m       2.40m         8       9       2.5       M.       Sardy SULT: hard, low plasticity, yellow brown, fine grained sand, some day fines       2       x         1.30m       9       3.0       Sardy SULT: hard, low plasticity, yellow brown, fine grained sand, some day fines       2       x         2.40m       Sardy SULT: hard, low plasticity, yellow brown, fine grained sand, some day fines       2       x         3.00m       9       3.0       Sardy SULT: hard, low plasticity, yellow brown, fine grained sand, some day fines       2       x         3.00m       9<	<u>0.20m</u> B <u>0.40m</u>	CL-CI CL-CI CL-CI CL-CI CL-CI CL-CI CL-CI CL-CI CL-CI CL-CI Slight moi CL-CI Night moi CL-CI	Y: very stiff to hard, low to medium plasticity, dark brown gr no fine grained sand in parts, grass and rootlets to 0.15 sture	ey,	H - TSV		ALLUVIUM
2.40m       B       2.40m       Image: Constraint of the second constraints of the second	1.50m B 1.70m 1.80m B 2.00m	L.5	AY: hard, low to medium plasticity, grey brown, some yello ttles, fine grained sand low plasticity, light brown grey and yellow brown, mottled, content	w  ay	I		1.10: slow digging, change to 300mm wide bucket - - 1.50: easier digging - - - - - -
B       3.00m       9       3.01       Silty SAND: dense, fine grained, some medium grains, yellow brown, traces of clay fines         SM       SM       SM       SM       SM         3.00m       9       3.0       SM       SM         SM       SM       SM       SM       SM         SM       SA0m       SM       SM       SM         SA0m       SM       SA0m       SM       SM         SA0m       SM       SM       <	<u>2.40m</u> B 2.70m	2.40m 2.5 ML 2.70m 2.70m	.T: hard, low plasticity, yellow brown, fine grained sand, so	me 🚬 🗵	т		-
EXCAVATION TP25 TERMINATED AT 3.40 m No ground water encountered. Test pit backfilled on completion.	3.00m	- Silty SAN - Silty SAN traces of the second	D: dense, fine grained, some medium grains, yellow brown clay fines	×	۵		
DCP 89 carried out adjacent test pit.					-		
PHOTOGRAPHS NOTES       YES       NO         METHOD       SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS & SOIL DESCRIPTION       CONSISTENCY/ RELATIVE DENSITY         N       Natural Exposure       Sumption       Samples       Soil DESCRIPTION       Based on Unified Classification System       VS       - Very Soft         B       Based on Unified R       Soil DESCRIPTION       St       - Very Soft       S         WATER       In Oct., 73 Water       Based on Unified Doct., 73 Water       D       - Disturbed Sample MC       MOISTURE       VSt       - Very Soft S         SUPPORT       In Oct., 73 Water Level on Date shown water inflow       In Oct., 73 Water       VS       - Very Losse No       VSt       - Very Losse ND       V.       - Uery Losse ND       NO         PBT       Plate Bearing Test       PBT       Plate Bearing Test       VD       - Very Dense	PHOTOGRAPHS NOTES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering See Explanatory Notes for details of abbreviations	ES NO TRATION J ⊥ ⊥ T No Resistance I 0 Oct., 73 Water Level on Date shown water inflow water outflow	SAMPLES & FIELD TESTS       CLASS         U50       Undisturbed Sample       CI         50 mm diameter       CI         D       Disturbed Sample       CI         B       Bulk Disturbed Sample       MOIST         MC       Moisture Content       D         HP       Hand Penetrometer (UCS kPa)       M         VS       Vane Shear, P-Peak, R-Remouded (uncorrected kPa)       W         PBT       Plate Bearing Test       SMEC ALISTRALIA	IFICATION SY DIL DESCRIP1 Based on Unif assification Sy TURE Dry Moist Wet	THBOLS & TION ised istern	CO REI VS F St VSt H UD D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Firm - Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense

PF	ROJI	ECT TIO	- :   N :	Roma Roma	I Flood N	<i>l</i> itigatio	n		EXCA	VATION - GEOLOG CLIENT : Ostwald B FEATURE :	ICAL LOG Bros Pty Ltd			F	<b>PIT</b> File :	NC / JO =T :	D : <b>TP26</b> B NO : 30031182 1 OF 1
P(	DSIT	rion DMF	NT :	E: 679	9136.13 • CAT	7, N: 70 Excavat	60962. tor 3T	135 (	55 MGA94)	SURFACE E	ELEVATION : 297.	483 (/	AHD)				
D/	ATE	EX	CAV	ATED	: 29/9/	13				LOGGED BY	Y : CM					СН	ECKED BY : JSM
E)	(CA	VAT	TION DF	DIME	INSION:	S : 0.60	0 m WI	IDE			MATERIAL						
Æ	PENETRATION	-	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	MATERIAL DESCRIPTION ioil Type, Colour, Plasticity or Particle Cha Secondary and Minor Component	aracteristic ts	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 1	D 015	20	STRUCTURE & Other Observations
						- 0.0			Silty CLA sand, gra	AY: very stiff, high plasticity, dark brown gre ass and rootlets to 0.1m	ey, trace fine grained						ALLUVIUM
						-		Сн	0.30m				VSt				-
					0.50m	-		СН	0.50m								-
					В	297.0			shading o	dark grey brown, slightly higher moisture c	ontent,						>>
					0.80m			СН	0.80m	fine grained sand, slightly higher moisture		-					>> .
						- 0.0 2 5967 -						W - 0	т				1.00: slow digging, change to 300mm wide bucket
					1.50m	-		СН									
					B	- 0. 1.5 - 967 		HHHH									-
					1.80m				2.00m								-
						- 50	-		No groun Test pit b DCP 93 c	A low the diverse in the second secon							-
0						- 2.5  - -											- - -
/03/2014 12:57 8.2.90		4 3.0													-		
3PJ < <drawingfile>&gt; 1</drawingfile>	Q 3.5 −					- 	-										- - -
MA FLOOD LEVEE.(						-											-
		PH	HOTOC	GRAPH	s	4.0	•	. [	NO NO							1	1
BANE LIBRARY - COLOUR.GLB Log TESTF	METI 3 3 3 3 3 3 3 3 3 3 3 5 0 PF	PHOTOGRAPHS NOTES NO ETHOD Natural Exposure Existing Excavation Backhoe Bucket Bulldozer Blade Ripper JPPORT Timbering No Resistance WATER 10 Oct., 73 Water Level on Date shown water inflow water outflow						Vo Res 73 Wat Date s ow flow	er hown	SAMPLES & FIELD TESTS         U50       - Undisturbed Sample         50 mm diameter         D       - Disturbed Sample         B       - Bulk Disturbed Sample         MC       - Moisture Content         HP       - Hand Penetrometer (UCS)         VS       - Vane Shear, P-Peak, R-Remouded (uncorrected         PBT       - Plate Bearing Test	kPa) CLASSIFICA SOIL DI Basec Classific MOISTURE D - Dry M - Moist W - Wet	FION SY ESCRIP I on Unit ation Sy	/MBOL TION fied /stem	S &		COI REL VS S F St VSt H VL L MD D VD	NSISTENCY/ _ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
SMEC BRISI	e E: tails basi	Explanatory Notes for s of abbreviations sis of descriptions.								SMEC AUSTRAL	IA						

PR	OJEC <sup>.</sup> CATIC	T: N:	Roma Roma	I Flood N	litigatio	ı		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT</b> FILE SHE	<b>NO</b> / JOB I ET : 1	: <b>TP27</b> NO : 30031182 OF 1
PO EQ	SITIO UIPMI	N : ENT T	E: 67 FYPE	9109.563 : CAT	3, N: 70 Excavat	60765.3 or 3T	897 (	55 MGA94) SURFACE ELEVATION : 297. METHOD : Excavator	283 (/	AHD)			
EX	CAVA		DIME	NSIONS	6 : 0.60	) m WIE	DE					CHE	CRED BT . JSINI
	7	D	RILLIN ⊺∝	IG س	1_		z	MATERIAL		~			
VE	E PENETRATION	SUPPORT	GROUND WATE LEVELS	SAMPLES &	+ ELEVATION	GRAPHIC LOG	CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	년 5 1015	20	STRUCTURE & Other Observations
					0.76		CI	Silty CLAY: firm to stiff, medium plasticity, dark brown grey, grass and rootlets to 0.3m, trace of no fine grained sand in parts	0	F - St		A	LLUVIUM - -
				0.40m B			CI	0.50m		VSt			-
				0.60m	0.5 —          -		СІ	becoming hard, shading dark grey brown	_				
				1.40m	- 1.0        		CI	1.00m gradually grading slightly higher silt content, no sand content	_				
				B 1.60m	1.5		CI	slightly higher moisture content	M - D	т			-
					- 2:0 - 562 - 2.0 			2.00m	_				- - -
					- 0.5 - 562 - 2.5		СІ	2.60m					- - -
4 12:57 8.2.900								EXCAVATION TP27 TERMINATED AT 2.60 m No ground water encountered. Test pit backfilled on completion. DCP 97 carried out adjacent test pit.					-
awingFile>> 12/03/201					- 294.0								- - -
000 LEVEE.GPJ < <l< td=""><td></td><td></td><td></td><td></td><td>3.5 — - - - - - -</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></l<>					3.5 — - - - - - -								
					4.0								
t Log TEST PIT F			GRAPH		YES	TION		NO SAMPLES & FIELD TESTS U50 - Undisturbed Sample Based	FION SY ESCRIP	(MBOL: TION fied	S &	CONS RELA	SISTENCY/ TIVE DENSITY - Very Soft
Y-COLOUR.GLE	Exis H Bac Bull Rip	sting E ckhoe I Idozer	xcavat Bucket Blade	ion <b>v</b>	VATER	0 Oct. 73	Wate	stance 50 mm diameter Classific D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) M - Moist	ation Sy	vstem		S F St VSt H VL	- Soft - Firm - Stiff - Very Stiff - Hard - Very Loose
ISBANE LIBRAR	UPPOR Tim	RT Ibering		as for		ater outfle	ow	own VS - Vane Shear; P-Peak, W - Wet R-Remouded (uncorrected kPa) PBT - Plate Bearing Test				MD D VD	- Loose - Medium Dense - Dense - Very Dense
det & b	ails of asis of	abbre f desc	viation	15 15 5.				SMEC AUSTRALIA					SMEC

PR LO	OJEC CATIC	T : DN :	Roma Roma	I Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL CLIENT : Ostwald Bros Pty L FEATURE :	LOG Ltd			<b>PI</b> FIL SH	<b>T NC</b> E / JO EET :	D: <b>TP28</b> B NO : 30031182 1 OF 1
PC FC	SITIO	N :	E: 67	9153.30 • CAT	2, N: 70 Excavat	60572. or 3T	685 (	5 MGA94) SURFACE ELEVATIO METHOD · Excavato	ION : 297.12	21 ( <i>A</i>	(HD)			
DA	TE E	KCAV	ATED	: 29/9/	13			LOGGED BY : CM					СН	ECKED BY : JSM
EX	CAVA		DIME	NSION:	S : 0.6	0 m WI	DE	MATERIA	AL					
	PENETRATION	SUPPORT	ROUND WATER LEVELS	SAMPLES & IELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	;	MOISTURE	ONSISTENCY RELATIVE DENSITY	5 101	520	STRUCTURE & Other Observations
3	<u> </u>		0	ол Ш	0.0 -		0	Silty CLAY: very stiff to hard, medium to high plasticity, dark b	prown grey,		U T - T			ALLUVIUM
				0.30m B 0.50m	296.5 - 297.0		сі-сн	2.50m		5	~			
					- - - - - - - - - - -		сі-сн	1_00m		Ч - U	I			
				1.50m B 1.70m	- 1.5 - - 2:9 - 2:68 - 		сі — — сі-сн	Som     Sandy CLAY: hard, medium plasticity, brown, fine grained san     content     becoming low to medium plasticity, light brown, increase in fin     sand content, grading clayey sand in parts	nd, some silt	M - M	т			
				2.00m B 2.30m	- 2.0 -   		CL-CI	2.00m Silty CLAY: hard, low to medium plasticity, light brown with fin- sand varying high silt content, grading clayey/sandy silt in part	e grained rts	M - M	т			
7 8.2.900					2.5	× × × × × × × × × × × × × × × × × × ×	< < < ML	2.50m Sandy SILT/Silty SAND: very stiff, low plasticity, light brown, fi sand with clay fines varying silt and clay fines in parts	fine grained	M - M	VSt/D			
rawingFile>> 12/03/2014 12:5				3.20m B 3.40m			SM SM	Silty SAND: dense, fine grained, light brown, trace of clay fine grading toward fine to medium grained sand, less clay fines R40m EXCAVATION TP28 TERMINATED AT 3.40 m		M - D	۵			
MA FLOOD LEVEE.GPJ < <e< td=""><td></td><td></td><td></td><td></td><td>3.5</td><td></td><td></td><td>No ground water encountered. Test pit backfilled on completion.</td><td></td><td></td><td></td><td></td><td></td><td></td></e<>					3.5			No ground water encountered. Test pit backfilled on completion.						
	METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI METHOI ME	D tural Existing E ckhoe I lldozer oper RT nbering	kposure xcavat Blade	S For	↓ 4.0	TION 	lo Resi 3 Wate Date sl ow	NO SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	CLASSIFICATIO SOIL DES Based o Classificat MOISTURE D - Dry M - Moist W - Wet	ON SY CRIP on Unif	mBOLS FION ied stem	S &	COI REI VS S F St VSt H L L D D VD	NSISTENCY/ _ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
sMEC BR	ails of basis of	abbre f desc	viation	IS IS.				SMEC AUSTRALIA						SMEC

PR		:T : DN :	Roma Roma	a Flood N	<i>l</i> itigatio	n		EXCA	VATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	;		<b>PI</b> File She	<b>f nc</b> E / Jc Eet :	D: <b>TP29</b> B NO : 30031182 : 1 OF 1
PO FO	SITIO		E: 67	7652.100	0, N: 70	63324. cavator	000 (! 4T	55 MGA94)	SURFACE ELEVATION : 3	01.155 (/	AHD)			
DA	TE E	XCAV.	ATED	: 2/5/14	1	cavator	-		LOGGED BY : CM				CH	ECKED BY :
EX	CAVA				5 : 0.5	0 m WI	DE		ΜΑΤΕΡΙΔΙ					
	PENETRATION	SUPPORT	OUND WATER	AMPLES & ELD TESTS	LEVATION (RL) DEPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	Sc	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	DNSISTENCY RELATIVE DENSITY	DCP		STRUCTURE & Other Observations
, ≚	ŤŤŤ	: 0,	5	s≣	- 0.0	1	5	Silty SAN	ID: dense, fine to medium grained, light brown to brown, slight		ö	5 101	520	ALLUVIUM
					301.0		SM	grey brow	vn, varying clay fines content, grass and rootlets to 0.15m	۵	D			-
				0.30m B			СІ	0.30m Sandy CL brown to sand	LAY: very stiff to hard, medium plasticity, brown, some light orangey yellow brown, fine rootlets, fine to medium grained					-
				B	- 0.5			becoming increase i	hard, low to medium plasticity, light brown/yellow brown, in fine to medium grained sand					-
				0.80m			CL-CI							-
					1.0									-
					300.0			1.20m becoming brown, ind	g light brown, mottled pale grey, light grey brown and yellow crease in silt content.					-
					1.5 —		CL-CI			to M	t to H			_
					- 299.5					Ω	N			-
					2.0-			2.00m shading li	ight yellow bown and light grey brown/pale grey, mottled					-
					- 539.0		CL-CI	2.30m						
1 8.2.900					-		SM	Increase I	in sand content					
3/2014 14:2				B	2.5			Clayey S/ brown, va	AND: dense to very dense, fine to medium grained, light yellow arying clay content, grading silty sand.	v				-
/ingFile>> 01/0				2.70m	- 298.5		SC	2.90m						
E.GPJ < <drav< td=""><td></td><td></td><td></td><td></td><td>3.0-</td><td></td><td></td><td>EXCAVA No ground Test pit ba</td><td>TION TP29 TERMINATED AT 2.90 m id water encountered. ackfilled on completion.</td><td></td><td></td><td></td><td></td><td>-</td></drav<>					3.0-			EXCAVA No ground Test pit ba	TION TP29 TERMINATED AT 2.90 m id water encountered. ackfilled on completion.					-
A FLOOU LEVE					- 298.0									· · · ·
	 F N	PHOTO	 Graph:	s	⊥ _   YES	ļ	<u> </u> [	NO NO						
BANE LIBRARY - COLOUR.GLB LOG 1ESI	PHOTOGRAPHS NOTES       YES       NO         METHOD       N       Natural Exposure       SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS & SOLD DESCRIPTION         N       Natural Exposure       Support       Support       U50 - Undisturbed Sample 50 mm diameter       U50 - Undisturbed Sample B       Buildozer Blade       Buildozer Blade       MOISTURE       Buildozer Blade       No Cest shown water inflow       U50 - Van Shear, P-Peak, R-Remouded (uncorrected kPa)       MOISTURE       D - Dry M - Moist         VB - Disturbed Sample       VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa)       VS - Wet       Wet									S &	CO REI VS S F St VSt H VL L MD D VD	NSISTENCY/ _ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense		
SWEC BKIS deta & b	e Expla ails of asis o	anator abbre f desc	y Note viatior ription	es for ns is.					SMEC AUSTRALIA					

PRO		CT : ON :	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCA	VATION - G	EOLOGICA : Ostwald Bros F	<b>LLOG</b> Pty Ltd			<b>PIT</b> File She	<b>F NC</b> E / JO E T :	D : <b>TP30</b> B NO : 30031182 1 OF 1
PO	SITIC	: NC	E: 67	7660.29	0, N: 70	63278.	000 (	55 MGA94)		SURFACE ELEV	ATION : 303.0	02 (A	AHD)			
EQI	UIPN TE E			: KUB	OTA Ex	cavator	4T			METHOD : Exca	avator				СН	
EXC		ATION		NSION:	s : 0.5	0 m WI	DE								011	
		D	RILLIN	IG			-			MATE	ERIAL	1				
Ψ.,	PENETRATION	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	S	MATERIAL D oil Type, Colour, Plasticit Secondary and N	ESCRIPTION ty or Particle Character linor Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 101	520	STRUCTURE & Other Observations
			-		- 0.0 <u>-</u> 303 -		сі	Sandy C sand, gra	LAY: hard, medium plasti ass and rootlets to 0.1m	city, brown, fine to med	lium grained		-			ALLUVIUM
				0.50m B	- - - 305 - - - - - - - - - - - - - - - - - - -		сі	<u>0.20m</u> some wh	ite powdery calcite nodul	es less that 10mm diam		-				-
				0.80m B			 CI	0.80m shading l crystallin	ight brown, less fragmen e nodules less than 5mm	ed white nodules, ocas: diameter	 sional glass like	-	т			
				1.00m	0, 1.0		 CI	1.00m without n	odules			W	· -			-
					s:1.5 - 1.5 - 2.0 - 0:0.0 - - - - - - - - - - - - - - - - - -		CI	1.70m increase 1.90m Silty SAN varying c	in fine to medium grained ID: dense to very dense, lay fines content	d sand content						-
UrawingFile>> 01/05/2014 14:21 8.2.900				2.70m B 2.90m	ین 2.5 000		SM SM	<u>2.70m</u> shading I	ighter, less fines content			-	D to VD			
00D LEVEE.GPJ <<					0. <del>3.0</del> 00 00 -	 - -		3.00m EXCAVA No groun Test pit b	TION TP30 TERMINATE d water encountered. ackfilled on completion.	D AT 3.00 m						
PIT ROMA FLL		PHOTON	GRAPHS		 		[	NO NO								
ANE LIBRARY - COLOUR GLB LOG 1ESII	ETHC Na ED Bu Bu Ri JPPO Ti	DD atural E xisting E ackhoe ulldozer ipper DRT mbering	SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS & SOIL DESCRIPTION       CONSISTENCY/ RELATIVE DENSITY         g Excavation g Excavation e Bucket er Blade       No Resistance       U50 - Undisturbed Sample 50 mm diameter       U50 - Undisturbed Sample 50 mm diameter       Soil DESCRIPTION Based on Unified Classification System       VS       - Very Soft S         WATER ng       10 Oct., 73 Water Level on Date shown water inflow       10 Oct., 73 Water Now water outflow       No Resistance       VS       - Very Soft S       VS       - Very Soft S       S         PBT       Plate Bearing Test       PBT       Plate Bearing Test       PBT       VS       - Very Soft Classification System       VS       - Very Soft S       - Very Soft S       - Soft F       - Firm S								NSISTENCY/ _ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense					
See deta & ba	e Exp ails o asis o	lanator f abbre of desc	ry Note viatior ription	es for is s.					SMEC AL	ISTRALIA						

PRO.	JECT	Г:F N:F	Roma Roma	Flood I	Vitigatio	n		EXCA	٩V		I - GEC	OLOGICA Ostwald Bros I	AL LOG Pty Ltd			<b>PI</b> FIL SH	I <b>T NC</b> LE / JC	<b>D</b> : <b>TP31</b> DB NO : 30031182 : 1 OF 1
POS	ITION	N : E	E: 677	666.82	0, N: 70	63239.	000 (	55 MGA94)			SI	JRFACE ELEV	ATION : 303	.530 (/	AHD)			
EQU		ENT T	YPE	: KUB	OTA Ex	cavator	4T				M	ETHOD : Exc	avator				0	
DAT	Ε ΕΧ( Δ\/ΔΤ			: 2/5/1 NSION	4 S · 0 5	0 m WI	DE				LC	DGGED BY : (	CM				CF	IECKED BY :
E/(0)	(0)(1	DF		IG	0.0.0							MAT	ERIAL					
NETPATION		IPPORT	JND WATER EVELS	MPLES & D TESTS	EVATION (RL) PTH (m)	RAPHIC LOG	SIFICATION	\$	Soil 1	MAT Type, Colour Second	ERIAL DESC , Plasticity or	CRIPTION Particle Characte	eristic	DISTURE	SISTENCY ELATIVE ENSITY	acu	5	STRUCTURE & Other Observations
ш <		รา	GRO	SAI		Ū	CLAS							¥ 0	CON CON	5 10	1520	
					303.5		СІ	Sandy ( fine to n	CLAY mediu	Y: very stiff to um grained s	hard, mediu and, grass ar	m plasticity, brown nd rootlets to 0.1m	to dark brown,					ALLUVIUM .
					-		СІ	some w	white	powdery calo	cite nodules le	ess than 10mm dia	meter					
					- 0.5 — 0.5 0.5 - - -		— — Сі	shading 10mm c	g light diame	t brown to br eter	own, frequen	t white powdery no	odules less that	_				-
					- - 0.1 - 305 - 305 -			0.80m shading content	g light t grad	t brown, sligt ling silty clay	tly yellow brc with sand in	wn, without nodule parts	es, varying sand	_				
					- 1.5 — 0.00 0.00 - -		CI							D to M	VSt to H			
				2.00m B 2.30m	- 2.0 2019 - 301 - 301 		 CI	2.00m shading	g light	t brown with	dark grey mo	ttling and staining	along fissures	_				
					2.5 0.100 0.000		CI	shading parts	g light	t brown, incr	easing in san	d content, grading	clayey sand in					-
5					-		SM	2.70m Silty SA varying 2.90m decreas	AND: g sand	dense to ver d content, gra	y dense, fine ading clayey s	to medium grained sand in parts	d, light brown,	_	D to VD			
					<u>3.0</u> <u>3.0</u> <u>3.0</u> <u>3.0</u> <u>-</u>	<b>k</b> . <u>1</u> <b>k</b>	C.W	3.00m EXCAV No grou Test pit	VATIC ound w it back	DN TP31 TEF vater encoun kfilled on con	RMINATED A tered. npletion.	T 3.00 m						
					-													
	PH NC	HUTOG DTES	RAPHS		] YES		[	NO NO										
ME <sup>®</sup> N BH B R <b>SUF</b>	ETHOD     PENETRATION     SAMPLES & FIELD TESTS     CLASSII       Natural Exposure     > u u x →     No Resistance     U50 - Undisturbed Sample     SC       Backhoe Bucket     Buildozer Blade     WATER     D - Disturbed Sample     MOIST       VPPORT     10 Oct., 73 Water     VS - Vane Shear; P-Peak, water inflow     VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)     W - Moist						CLASSIFICA SOIL DI Basec Classific MOISTURE D - Dry M - Moist W - Wet	TION SY ESCRIP d on Unit cation Sy	(MBOLS TION fied /stem	5&	CORE VS S F St S H VL L D D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Dense - Very Dense						
See I detail & bas	Explaits of a sis of	natory abbrev descr	v Note viation	s for s s.	1 1					SME	CAUS	TRALIA	1					SMEC

								EXCA	VATION - GEOLOGICAL L	LOG				NO : TP32	
PROJ LOCA		· :   N :	Roma Roma	Flood N	/litigatio	n			CLIENT : Ostwald Bros Pty Lto FEATURE :	d			FILE / . SHEET	JOB NO : 30031182 T : 1 OF 1	
POSI		N : Г	E: 67 YPF	7695.82 • KUB(	0, N: 70	63207. cavator	000 (	55 MGA94)	SURFACE ELEVATIO	N : 303.49	0 (A	HD)			
DATE	EEX	CAVA	ATED	: 2/5/14	4	ou rator			LOGGED BY : CM				C	CHECKED BY :	
EXCA	AVA1	TION DF	DIME	INSION:	S : 0.50	0 m WI	DE		MATERIAL	L					
NETRATION		JPPORT	UND WATER LEVELS	MPLES & LD TESTS	EVATION (RL) EPTH (m)	RAPHIC LOG	SSIFICATION SYMBOL	So	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components			VSISTENCY ELATIVE DENSITY	DCP	STRUCTURE & Other Observation	s
Ч щ ц	<u>. т</u>	S	GRO	SA	법 법 - 0.0	υ	CLAS	Clavey S	AND: very dense, brown, fine to medium grained sand	t high to	≊ö	0°°°	5 10152		
					-		sc sc	0.10m medium p shading c	plasticity clay fines, grass and rootlets to 0.1m dark grey, brown		Ω				
				0.30m			sc	some whi	ite powdery calcite nodules to 10mm diameter						
				В	-			shading s	slightly lighter						
				0.50m	e 0.5		sc								-
					- 30										
				0.70m B				0.70m shading l	ight brown/ grey, frequent white powdery calcite nodule	les, slight					
					-			40016436							
				1.00m	_بي 1.0										_
					- 302										
					-		sc								
					-										
					-						M	9			
					302.0						D				
					-			1.70mshading l							
				1.80m B			sc	ondoning							
					-			2.00m							
				2.10m	9: 2.0			slight dec grey brow	crese in clay fines, increase in moisture content, shadin wn, pale grey and yellow brown, mottled.	ng light					_
					-										
8.2.90					-		SC								
4 14:21					0. 2.5										-
1/06/201					-										
le>> 0															-
DrawingF						17.J.	1	2.90m							
					s: 3.0			No groun Test pit b	ackfilled on completion.						-
EVEE.6					-										-
LOODL															
	PH NC	HOTOG	RAPH		] YES		[	NO NO	· · · ·						
MET	HOD			F	PENETRA ∛⊔⊥≖	TION ≟ ≸			SAMPLES & FIELD TESTS	ASSIFICATIO	N SY	MBOLS	3& C R	CONSISTENCY/ RELATIVE DENSITY	
N E L	Natu Exis	ting Ex	posure cavat	e ion		N	lo Res	stance	50 mm diameter	Classificatio	onif on Sy	stem	S F		
B B R	Bullo	dozer   ber	Blade		VATER				B - Bulk Disturbed Sample M MC - Moisture Content	IOISTURE				/St - Stiff /St - Very Stiff H - Hard	
SUP	POR	г			<sup>1</sup>	0 Oct., 7 evel on l	3 Wat Date s	er nown	HP - Hand Penetrometer (UCS kPa) M VS - Vane Shear; P-Peak, W	1 - Moist V - Wet				/L - Very Loose - Loose MD - Medium Der	ise
ANE LIB	Timb	pering				ater inflo ater out	ow low		R-Remouded (uncorrected kPa) PBT - Plate Bearing Test					D - Dense /D - Very Dense	
See E details & basi	xpla s of a is of	nator ibbre desci	y Note viatior ription	es for is s.					SMEC AUSTRALIA				I	SME	C

PR LO		T : DN :	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCA	VATION - G	EOLOGICA : Ostwald Bros F	ALLOG Pty Ltd			<b>P</b> FI SI	IT N LE / J HEET	O: TPB01 OB NO : 30031182 : 1 OF 1
PO	SITIO	N :	E: 679	9427.81	5, N: 70	62423.4	427 (	55 MGA94)		SURFACE ELEV	ATION : 300.2	227 (/	AHD)			
EQ	UIPM TE F>	ENT KCAV		: CAT	⊨xcava 13	tor 23T				LOGGED BY · C	avator CM				CI	HECKED BY : JSM
EX	CAVA		DIME	INSIONS	- S : 0.6	0 m Wl	DE									
		DI	RILLIN	IG			7			MATE	ERIAL					
Æ	E PENETRATION	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	S	MATERIAL I bil Type, Colour, Plastic Secondary and	DESCRIPTION city or Particle Character Minor Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	요 ) 1520	STRUCTURE & Other Observations
Ť	ĪĪĪ				- 0.0		СІ	Sandy Cl	AY: very stiff, hard, me	dium plasticity, brown, sl	light grey brown,		ст. В.т.			ALLUVIUM
					300.0		SM	0.25m Silty SAN	ID: dense, fine to mediu	im grained, brown		-				
				0.60m	0.5 -			0.50m								
				В		 		grains, va	arying silt content	alam plastory,						
				0.90m			SM									
					1.0-											
				1.30m B	299.0			1.30m SAND: de	ense, fine to medium an	ained, light brown,		-				
				1.60m	1.5 -			slight ligh	t grey brown, trace of si	ilt						
					20-											
					2.0		SP									
					298.0											
					2.5 -							Σ				
					-							Ġ				
					3.0			3.00m				-				
					-			shading I	ight brown, less grey br	own						
					297.0											
					3.5 -		SP									
					-											
					4.0			4.00m				-				
					-			slightly hi	gner moisture content							
					296.		SP									
					4.5 -											
$\parallel$		_	<u> </u>				•	4.80m		TED &T 4 80 m				+	++	
					5.0			No groun Test pit b	d water encountered. ackfilled on completion.	1 EU AT 4.00 M						
					o	1										
					295.	1										
					5.5 -	1										
					-	1										
					6.0 -	1										
					0.4											
					594											
					0.5 -											
					-											
⊢	L L L F	 РНОТОС	 GRAPHS	L 3	⊥ <sub>7.0</sub>	I	<u> </u>					1				
L	Ň	NOTES			j YES		ļ	NO NO					MPC			
M	ETHO	D		F	PENETRA	TION			SAMPLES & FIELI	DTESTS	CLASSIFICAT SOIL DE	SCRIP	MBOLS	5 &		ELATIVE DENSITY
N E	Na Exi	tural Ex isting E	kposure xcavati	e ion		_́ N	o Res	istance	U50 - Undisturb 50 mm di	oed Sample ameter	Based Classifica	on Unif Ition Sy	ied stem		S F	- very Soπ - Soft - Firm
B	H Bao Bul	ckhoe I Ildozer	Bucket Blade						D - Disturbed B - Bulk Distu	l Sample urbed Sample	MOISTURE				St VS	- Stiff St - Very Stiff
R	Rip	per		v	VATER	0 Oct 7	3 \N/~+	er	MC - Moisture HP - Hand Per	Content netrometer (UCS kPa)	D - Dry M - Moist				H VL	- Hard - Very Loose
s		RT				evel on [	Date s	hown	VS - Vane She R-Remo	ear; P-Peak, uded (uncorrected kPa)	W - Wet					- Loose D - Medium Dense
T	Tim	nbering				vater inflo vater outf	w low		PBT - Plate Bea	aring Test						- Dense D - Very Dense
See det & b	e Expla ails of asis of	anator abbre f desc	y Note viatior ription	es for is s.					SMEC AU	JSTRALIA						SMEC

PROJ LOCA		- : I N : I	Roma Roma	I Flood M	Vitigatio	n		EX	(CA)	VA	ATION CLIE FEA	- GE NT FURE	EOLO : Ostwa	GICA	AL LOO Pty Ltd	3			F F S	PIT N FILE / J	<b>IO</b> : JOB NO	<b>TPE</b> D : 30031 DF 1	<b>182</b>	
POSI		א: ידא:	E: 67	9897.10 · CAT	3, N: 70	62391.	824 (	55 MGA	494)				SURFAC		ATION: 3	313.198	B (A	AHD)						
DATE	EEX	CAV	ATED	: 26/9/	13	101 231							LOGGE	D BY : (	CM					C	HEC	KED BY	: JSM	
EXCA	VAT				S : 0.6	0 m WI	DE							MAT										
S			Ê	30 21 20	Z C	0	NOI							IVIA II			шZ	≻						
VE E PENETRATI	LΙ	SUPPOR	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIC BLEVATIC RL) DEPTH (n	GRAPHIC LOG	CLASSIFICAT SYMBOL		So	Soil Ty	MATE ype, Colour, Secondar	RIAL DE Plasticity y and Mi	SCRIPTIC or Particle nor Compo	N Characte onents	ristic	I I LOI V	CONDITION	CONSISTEN RELATIVE DENSITY	5 10	요 0152	D	STRL & Other C	JCTURE Observation	IS
					313.0		CI	0.40m	Gravelly C o coarse, ine to coa oots to 20 	cLA e, rou oarse 20mm	Y: very stiff to unded to suba grained san m thick to 0.5	o hard, m angular g d, grass a m. 	edium plas ravels and and rootlets	ticity, dark cobbles to s to 0.4m, o	grey brown, fi 100mm, some occasional tree	ine e e	۵				ALI	LUVIUM		
				0.70m B	0.5		СІ	0.60m C S Ie C	silty CLA cobbles to	AY: han trac	ard, medium es of fine to communication of fine to communication of fine to communication of fine to communication of the first of the	to high p oarse, ro ssuring, b	lasticity, ora ounded to s blocky in pa	ange brown ubrounded rts	n, d gravels and									-
				1.00m 1.30m	- 1.0 - 		сі-сн	1.30m									M - 0	т						_
				B 1.60m 1.80m			сі-сн	y s v ir	ellow/ora and or gr ery low s nclusions	range grave stren ns, so	e brown and l el content, so ngth, extreme ome rock strue	ight grey/ me fine to ly to high cture in p	/brown, hig o course gr lly weather arts,	her silt con avel sized ed, siltston	ntent in parts, r extremely low ie/mudstone	no r to					RE	SIDUAL S	OIL	-
				B 2.10m	2.0		> > >	2.10m X e 2.40m V	W SILTS extremely arying ba	TSTO ly to h bands	DNE/MUDST( highly weathers, laminations	DNE: ext ered, yello , friable,	remely low ow/orange remoulds t	to very low brown and o gravelly s	v strength, light grey brov silty clay	wn,					WE	ATHERED	ROCK	
					2.5		~ ~ ~ ~ ~ ~ ~	vb	rery low s prown, red	v stren redbro	ngth, highly w own and light	reathered grey brow	l in parts, si wn	hading yell	low/orange		- M							-
					3.5 — - - 4.0 — - - - - - - - - - - - - - - - - - - -			<u>3.70m</u> b fr	 becoming riable, slo	ng ver slow d	ry low to low s digging	strength,	highly to m	oderately v		 35								-
							>	4.50m E N T C	EXCAVAT lo ground est pit ba DCP 101	ATION nd wa backf 1 carr	N TPB02 TEF ater encounte filled on comp ried out adjac	RMINATE ered. oletion. ent test p	D AT 4.50 bit.	m										
					0, - 80, - 5.5 — -	-																		-
					- 0.0 	-																		-
					6.5																			
	PH NC		RAPH	s	] YES		[	N N	0	_			_											
MET N BH B R SUP	HOD Natu Exis Bacl Bullo Ripp POR	ural Ex ting E khoe E dozer l ber <b>F</b> pering	posure kcavat Bucket Blade	e ion		NTION = → N 0 Oct., 7 evel on I vater inflc vater outf	lo Res 3 Wat Date s ow flow	istance er nown			SAMPLES 8           U50         -         Un           50         -         Dis           B         -         Bui           MC         -         Mo           HP         -         Ha           VS         -         Va           R-F         PBT         -	FIELD 1 disturbed mm diam turbed S lk Disturb isture Cc nd Penel ne Shear Remoude te Bearir	ESTS d Sample neter ample bed Sample bed Sample bontent trometer (L r; P-Peak, ed (uncorre ng Test	e JCS kPa) ected kPa)	CLASSIFI SOII Ba Clas MOISTU D - D M - M W - W	ICATIO L DESC ased on ssificatio IRE Inry Ioist Vet	N SY RIP Unit	YMBOL: TION fied ystem	S &	CR VSFSVHVLZDV	ONSIS ELATI S t St L D	TENCY/ VE DENSI - V - S - F - S - V - H - V - L - M - D - V	TY 'ery Soft oft irm tiff lard ery Loose oose ledium Der lense 'ery Dense	nse
See E details & bas	xpla s of a is of	nator abbre desci	y Note viatior	es for ns is.							SMEC	; AU	STRA	ALIA								Œ	SM	EC

EXCAVATION - GEOLOGICAL LOG           PROJECT : Roma Flood Mitigation         CLIENT : Ostwald Bros Pty Ltd           LOCATION : Roma         FEATURE :	PIT NO : TPB03 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 680286.923, N: 7062331.850 (55 MGA94) SURFACE ELEVATION : 318.613	3 (AHD)
EQUIPMENT TYPE : CAT Excavator 23T METHOD : Excavator	
EXCAVATED : 20/9/13 LOGGED BY : CM	CHECKED BY : JSM
DRILLING MATERIAL	
MATERIAL DESCRIPTION South Description MATERIAL DESCRIPTION Secondary and Minor Components Material Description Secondary and Minor Components Material Description Secondary and Minor Components	NULLING A STRUCTURE & Other Observations
0.0 Silty CLAY: very stiff to hard, medium to high plasticity, dark grey brown, trace of fine to medium grained sand and fine to coarse rounded to 0.30m subrounded gravels, grass and rootlets to 0.3m	
Bbecoming hard	Σ I I I I I I I I I I I I I I I I I I I
Cl 0.00m becoming brown, slightly orange brown, trace of fine to coarse grained	
B 1.00m 1.0 CI-CH 1.40m 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	-
1.5 - trace to no sands, gravels and white nodules	», т
B 2.10m 2.10m 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0	RESIDUAL SOIL
2.50m 2.70m 2.70m 2.70m	-
B       5         3.00m       3.0    W SILTSTONE/MUDSTONE: extremely low strong, extreme to highly weathered yellowlight brown, orange brown and grey brown, blocky, friable, interbedded bands/laminations, remoulds to gravelly silty clay	WEATHERED ROCK
3.5 3.50m 2 3.6 3.70m 2 3.70m 2 3.70	×
4.0	-
4.5- 9 6.0- 0 0 0 0 0 0 0 0 0 0 0 0 0	
PHOTOGRAPHS YES NO	
METHOD       PENETRATION       SAMPLES & FIELD TESTS       CLASSIFICATION         N       Natural Exposure       Soluburg       Soluburg       Soluburg       Based on U         E       Existing Excavation       Soluburg       Soluburg       Soluburg       Soluburg       Based on U         B       Buildozer Blade       WATER       D       Disturbed Sample       MOISTURE       D       Classification         Support       T       Timbering       10 Oct., 73 Water       Level on Date shown water inflow       VS       Vane Shear, P-Peak, R-Remouded (uncorrected kPa)       W       W       W et	N SYMBOLS & CONSISTENCY/ RIPTION RELATIVE DENSITY Unified S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions. SMEC AUSTRALIA	

PF	roj DCA	ECT TIO	- <u>:</u>   N :	Roma Roma	I Flood N	litigatio	n		EXCA	CLIENT : Ostwald Bros Pty Ltd FEATURE :			F	PIT   FILE / SHEE	NC JO	D : <b>TPB04</b> B NO : 30031182 1 OF 1
P( E( D/ E)	DSI QUI ATE KCA	TION PME E EX	N : NT 1 CAV/	E: 679 TYPE ATED DIME	9659.91 : CAT : 26/9/ NSION	5, N: 70 Excavat 13 S · 0.6	62294. tor 23T	933 (	55 MGA94)	SURFACE ELEVATION : 299.4 METHOD : Excavator LOGGED BY : CM	44 (A	AHD)			СН	ECKED BY : JSM
	-		DF	RILLIN	١G					MATERIAL						
Æ	E PENETRATION	. T	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL		MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	5 1	0152	20	STRUCTURE & Other Observations
					0.30m B 0.60m	0.0			0.20m Silty CL grained thick to becomi to medi 0.60m bigher	AY: very stiff, hard, high plasticity, dark grey brown, trace of fine Isand, grass and rootlets to 0.2, occasional tree roots to 20mm 0.3m / ng hard, high plasticity, dark grey to black, less than trace of fine ium grained sand and subrounded to angular gravels	Q W - Q	VSt - H	-		>	ALLUVIUM 0.30: HP In-situ >500 kPa
					0.90m			CI-CH	1.00m	ng moist, trace to no sand and gravels	Σ	T			>	
					1.90m B	0. 1.5 0. 1.5 0		СН	1.90m Clayey brown,	GRAVEL: very dense, high plasticity clay fines, light brown to slight grey brown, no sand content	×		-	×	< label{eq:states}	2.00: HP In-situ =350 kPa
					2.20m 2.70m B			СН	2.70m Silty SA	AND: dense, fine grained, light brown, slight yellow brown, high silt		VSt	-			
					3.00m	- 3.0 -    		               	content	, grading sandy silt in parts, trace to some clay fines, moist						
								  -  -  -  -  -	4.50m		×	۵				
12:57 8.2.900						5.0		SM	5.00m becomi 5.20m	ng moist to wet	M - W	-				
ngFile>> 12/03/2014						- 			EXCAV No grou Test pit DCP 10	ATION 17B04 TERMINATED AT 5.20 m und water encountered. backfilled on completion. 11 carried out adjacent test pit.						
LEVEE.GPJ < <drawi< td=""><td></td><td></td><td></td><td></td><td></td><td>6.0</td><td>- - - -</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></drawi<>						6.0	- - - -									
PIT ROMA FLOOD		PI	HOTOC		6											
ANE LIBRARY - COLOUR GLB Log TESTI	MET N E BH B R SUP T	HOD Natu Exis Bac Bulle Ripp POR	ural Ex ting E khoe E dozer ber <b>F</b> bering	posure xcavat Bucket Blade	e ion V		TION <sup>-</sup> → 0 Oct., 7 evel on vater inflo vater out	lo Res 73 Wat Date s ow flow	istance er hown	SAMPLES & FIELD TESTS       CLASSIFICATION         U50       -       Undisturbed Sample       Soil DES         D       -       Disturbed Sample       Based of Classificat         MC       -       Moisturbed Sample       MOISTURE         MC       -       Moisturbed Sample       D       -         MP       -       Hand Penetrometer (UCS kPa)       VS       -       D       -       Dry         VS       -       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W       -       Wet         PBT       -       Plate Bearing Test       -       -       -       -	ON SY SCRIPT on Unif tion Sy	TMBOL ΓΙΟΝ ied stem	S &		COI REL VS S F St VSt H VL L MD D VD	NSISTENCY/ _ATIVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
SMEC BRISE	ee E tails bas	xpla s of a is of	nator abbre desc	y Note viatior ription	es for ns s.					SMEC AUSTRALIA					_	

	IECT	r . I	Domo	Flood	Vitigat	ion		EXCA	VATION -		AL LOG			<b>PIT</b>		<b>TPB05</b>	
LOC	ATIO	N :	Roma		villigat	ION			FEATU	JRE :				SHE	ET :	1 OF 1	
POS		N :	E: 680	0056.37	5, N: 7	'062233	.911 ( -	55 MGA94)		SURFACE ELE	VATION : 313.	861 (	AHD)				
				: 26/9/	Excav	ator 231				LOGGED BY :	CM				СН	ECKED BY : JSM	
EXC	AVAT	TION	DIME	NSION	S : 0.	60 m W	IDE								0		
		DF	RILLIN	IG						MAT	FERIAL						
		RT	ATER	STS STS	NO	Ê Ω	L		MATED			E NO	Z S S S S S S S S S S S S S S S S S S S				
VOLU	Į.	РРО		D TE	(RL)	APH LOG	SIFIC/	S	oil Type, Colour, Pl	asticity or Particle Charact	eristic	NDITION	SISTE	DCP		STRUCTURE & Other Observations	
	į u I	SU	GROL	SAN	Ш	Ξ <sup>5</sup>	CLAS		Secondary	and Minor Components		N N	CON	5 101	520		
ΠĪ	ŤŤ				0.0		CLCH	Silty CLA	Y: very stiff to hard	, medium to high plasticity,	dark grey brown,		Ξ			ALLUVIUM	_
								0.30m gravels a	ind cobbles to 50mr	m, grass, rootlets and tree r	oots to 15mm	/	VSt				-
					0.5		CI	0.50m becoming	g hard, medium plas	sticity, brown to dark brown	/ 					<	
				0.60m B	-		t	medium t fewer gra	to high plasticity, bro avels and cobbles, s	own slight light brown, less some white, powdery, calcite	sand content and e nodules with					'0.50: HP In-situ >500 kPa	-
				0.90m			CI-CH	coarse gi	rained sand to fine	gravel sized shiny crystals		Σ			×	, `0.80: HP In-situ >500 kPa	-
				1.00m B	1.0 E			1.00m									
								and oran	ge brown, fine to m	edium gravel sized, extrem	ely low to very low				*	1 20: HD In eitu >500 kDe	-
				1.30m	-		CI-CH	strengtri		Some fock s	diucture in parts					1.20: HP In-situ >500 kPa	-
					1.5		<u>4</u>	1.50m XW SILT	STONE/MUDSTON	IE: extremely low to very low	w strength,					WEATHERED ROCK	
				1.70m B	-		X	extremely	y to highly weathere ded bands/laminatio	ed, yellow/orange brown an ns, laminated to very thinly	d light grey brown bedded, friable,						-
				2.00m	12.0	1 I I	X	remoulds	s to gravity silty clay								-
				2.0011	- స్ 2.0	1	8										_
							8										-
					25		X										_
					2.0		X										-
							₹	2.80m				_					-
					311.0	1	8	siltstone	beds, predominantly	y light grey brown mudston	e beds in parts,	Σ					_
							8	mable									_
							X										-
					3.5	-==>	X										_
							8										
					0		Å										-
					မ် ရှိန် 4.0		≹	4.00m very low	to low strength in pa	arts, slower digging		-					-
							X		0 1								-
							8	4.50m									1
		_			-4.5		1	EXCAVA	TION TPB05 TERM	IINATED AT 4.50 m							_
						1		No groun Test pit b	nd water encountere backfilled on comple	ed. etion.							-
					0.90	-											-
					rð 5.0	-											-
						1											_
					5.5	_											_
					0.0	-											-
						-											-
					0.806.0	-											_
						1											_
						1											-
					6.5	-											_
						-											-
					0.7	1											-
	PH	нотос	RAPHS	·	ஜ 7.0	_	,										
L	NC	OTES			] YES	,		NO NO									
ME	THOD				PENETI	RATION			SAMPLES & F	IELD TESTS	CLASSIFICAT SOIL DF	FION S	YMBOLS	5&	CON	ISISTENCY/ ATIVE DENSITY	
Ν	Natu	ural Ex	posure		ц П	. <u>.</u>	No Res	istance	U50 - Undis	sturbed Sample	Based	on Un	ified		VS S	- Very Soft - Soft	
E BH	Exis Bacl	ting E khoe F	kcavati Bucket	on					50 m D - Distu	m diameter rbed Sample	Classific	auori S	ysiem		F St	- Firm - Stiff	
В	Bullo	dozer	Blade						B - Bulk MC - Moist	Disturbed Sample	MOISTURE				VSt H	- Very Stiff - Hard	
к	кірр	Jer			WATER	10 Oct.,	73 Wat	er	HP - Hand	I Penetrometer (UCS kPa)	D - Dry M - Moist				VL I	- Very Loose	
SU	PPOR	T				Level on	Date s	hown	VS - Vane R-Re	Shear; P-Peak, mouded (uncorrected kPa	W - Wet				MD	- Medium Dens	e
	rimt	vering				water ou	tflow		PBT - Plate	Bearing Test					VD	- Very Dense	
See l detai	Explai Is of a	nator	y Note	s for s	1				SMEC	AUSTRALIA	1						c
G Dd		2000	-pu01	<b>.</b> .													

PROJECT : Roma Flood Mitigation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	PIT NO : TPB06 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679739.057, N: 70 EQUIPMENT TYPE : CAT Excavat DATE EXCAVATED : 25/9/13	S2195.238 (55 MGA94)         SURFACE ELEVATION : 302.40           or 23T         METHOD : Excavator           LOGGED BY : CM	01 (AHD) CHECKED BY : JSM
EXCAVATION DIMENSIONS : 1.20 DRILLING	m WIDE MATERIAL	
VE F PENETRATION H SUPPORT GROUND WATER GROUND WATER LEVELS FIELD TESTS FIELD TESTS FIELD TESTS DEPTH (m)	OF DE COLOR Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	NOLLIGNOOD STRUCTURE & Other Observations 5 101520
0.40m B B S R 0.5-	CL-CI       Silty CLAY: stiff, low to medium plasticity, brown, some fine to medium grained sand, trace of fine to medium gravels, grass and rootlets to 0.2m         CI       0.40m         sand, some fine to coarse, rounded to subangular gravels and cobbles to 50mm, some white calcite nodules.         CI-CH       becoming medium to high plasticity, light brown, orange and red brown, grey brown	□ □ □ □ □ □ □ □ □ □
0.80m 0.90m B 1.0-	XW MUDSTONE: extremely low strength, orange and red brown, pale grey and grey brown, weathered to silty clay, high plasticity	
S 1.5	1.30m         XW SANDSTONE: extremely low strength, extremely weathered, medium to coarse grained grained, light brown, slight yellow/orange brown, very low strength in parts, remoulds with trace of silt content         2.00m	Ψ -
ରୁ 2.5	<ul> <li>becoming very low to low strength, fine to medium grained, pale grey and grey, high silt content, remoulds in parts to fine to medium grained silty sand</li> <li>2.40m</li> <li>HW MUDSTONE/SANDSTONE: very low to low strength, highly to moderately weathered, interbedded bands of grey to dark grey mudstone and fine to coarse grained, orange brown and light brown sandstone, 2.80m</li> </ul>	Σ
3.0- - - § 3.5-	EXCAVATION TPB06 TERMINATED AT 2.80 m No ground water encountered. Test pit backfilled on completion.	
4.0-		
98 4.5		
0 5 5 5 		
, 6.0		
PHOTOGRAPHS NOTES     YES       METHOD     PENETRA Support       N     Natural Exposure E       E     Existing Excavation BH       B     Bulldozer Blade R       R     Ripper       SUPPORT T     Timbering	NO       SAMPLES & FIELD TESTS       CLASSIFICATION         Solid DES       U50 - Undisturbed Sample       Based of Classificat         D       - Disturbed Sample       Based of Classificat         D       - Disturbed Sample       Based of Classificat         D       - Bulk Disturbed Sample       MOISTURE         D       - Bulk Disturbed Sample       MOISTURE         D       - Woisture Content       HP         HP       - Hand Penetrometer (UCS kPa)       M         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W         PBT       Plate Bearing Test       W	ON SYMBOLS & SCRIPTION     CONSISTENCY/ RELATIVE DENSITY       In Unified tion System     VS     - Very Soft S       F     - Firm St     - Stiff VSt       VU     - Very Stiff H     - Hard VL       VL     - Very Loose L     - Loose MD       MD     - Medium Dense D     - Dense VD
See Explanatory Notes for details of abbreviations & basis of descriptions.	SMEC AUSTRALIA	SMEC.

PRC LOC	)JECT	Г : I N : I	Roma Roma	I Flood N	litigatio	n	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			FILE / SHEET	<b>NO</b> : <b>TPB07</b> JOB NO : 30031182 T : 1 OF 1
POS		N :	E: 68	0240.82	9, N: 70 Execute	62094.724	(55 MGA94) SURFACE ELEVATION : 312.4	138 (/	AHD)		
DAT	TE EX	CAVA		: 25/9/1	Excavat 13	01 23 1	LOGGED BY : CM			C	CHECKED BY : JSM
EXC	AVA			INSION	5 : 1.20	0 m WIDE					
	Z		кіссіг Ш	wΩ	Z C	C Z	MATERIAL		≿		
VE E	F PENETRATI	SUPPOR	GROUND WAT LEVELS	SAMPLES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG CLASSIFICATI	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	වි 5 10152	STRUCTURE & Other Observations
				0.30m B 0.60m 0.80m		CI-C	Sitty CLAY: very stiff to hard, medium to high plasticity, dark grey brown, trace of fine to coarse grained sand and fine subangular gravels and 0.30m cobbles to 80mm, grass and rootlets organic matter and tree roots to 10mm thick to 0.3m becoming hard 0.60m becoming orange brown and brown, trace of fine to coarse grained sand, 0.80m some white, powdery, calcite nodules and white, coarse grained sand	- 0	H VSt - H		
				в <u>1.10m</u> 1.80m			<pre>station cystats: / shading orange brown, some light brown mottling, without sand content or orange nodules 1.80m</pre>	Ψ	т		× 1.00: HP In-situ >500 kPa
				2.10m 2.50m B	2.0	95 95 95 95 95 95 95 95 95 95 95 95 95 9	Clayey GRAVEL: very dense, well-graded, orange brown some light brown mottling  2.50m with extremely weathered siltstone/mudstone in parts. extremely low	- D	۵		×2.00: HP In-situ >500 kPa
				3.50m	<ul> <li> <sup>∞</sup> <sup>−</sup> <sup>−</sup></li></ul>		3.00m     XW SILTSTONE/MUDSTONE: extremely low strength, extremely low     weathered, yellow/light brown and grey brown, blocky, friable, interbedded     bands/laminations, remoulds to gravelly sitly clay     3.50m     becoming low strength, highly weathered	M-D	т		WEATHERED ROCK
IOMA FLOOD LEVEE GPJ < <drawingfile>&gt; 12/03/2014 12:57 8 2.900</drawingfile>					0: 4.5 0: 4.5 0: 5.0 		4.50m EXCAVATION TPB07 TERMINATED AT 4.50 m No ground water encountered. Test pit backfilled on completion.				
BBANE LIBRARY - COLOUR GLB LOG TEST PIT F	PIN Nati Exist Ball Ripp IPPOR Tim	ural Ex sting Ex khoe E dozer I ber T	posure ccavat Bucket Blade	S P e ion V		No Re No Re 0 Oct., 73 Wa evel on Date vater inflow vater outflow	NO       SAMPLES & FIELD TESTS       CLASSIFICAT         sistance       U50 - Undisturbed Sample 50 mm diameter       Based Classifica         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       MOISTURE D - Dry M - Moist W - Wet         PBT - Plate Bearing Test       Part	ION SY SCRIP on Uni ation Sy	YMBOLS TION fied ystem	S& C F V S S V V F V L L V V V V V V V V	CONSISTENCY/ RELATIVE DENSITY /S - Very Soft - Firm St - Stiff /St - Very Stiff - Hard /L - Very Loose - Loose /D - Medium Dense /D - Dense /D - Very Dense
See deta & ba	Expla ils of a asis of	nator abbrev desci	viatior viatior	es for is s.			SMEC AUSTRALIA				

PROJE( LOCATI	CT ION	: Roi : Roi	na Floo na	od Mi	itigatio	n		E	XCA	V		<b>DN -</b> CLIENT FEATUI	GEC	Ostwald E	ICA Bros F	The second secon	•			F	PIT N FILE /	NO JOE T :	: <b>TPB08</b> 3 NO : 30031182 1 OF 1	
POSITI	ON	: E:	679823	.467	, N: 70	62073.	694 (	55 MC	GA94)				SL	JRFACE E	ELEV	ATION : 3	05.710	(AH	D)					
EQUIP	MEN		E : C		ixcavat	or 23T							ME	ETHOD :	Exca	avator						~ · · ·		20.4
DATE E	=XCA идтіс	ייח אי	-D: 25	)/9/10 7NI9	3 · 1 ?/	) m \//I	DF						LC	IGGED B	r:C	JVI					(	JHE	ECKED BY : JS	SIVI
		DRIL	LING	0.10											MATE	ERIAL								
NOI			<u>م</u>	TS	N (u	0	NOI										ш	Z √	шs					
/E E PENETRATI	_	SUPPUR GROUND WA	SAMPLES	FIELD TES	ELEVATIO (RL) DEPTH (r	GRAPHIC	CLASSIFICAT SYMBOL		S	Soil T	Type, Co Seco	MATERIA blour, Plas ondary ar	AL DESC sticity or nd Minor	RIPTION Particle Cha Componen	aracter its	ristic	MOISTUR		DENSITY	5 1	요 0152	20	STRUCTU & Other Obser	RE vations
	Ī				- 0.0	////	GW	0.20m	Clayey G	GRA	VEL: ver	ry dense,	brown, f	ine to coars	e, roun	ided to							ALLUVIUM	
			0.50m	n	-	19 19	GW	0.20m	subangu friable, m some wh	nediu nite, j	jum plast powdery	ticity clay f	fines, tra odules	ice of fine g	rained	sand.	_/		н -				, `0.30: HP In-situ >50	00 kPa
			B 0.80m	1	05.0		GW	0.80m	becoming white cal	ig ligi Icite	hter bro nodules	wn, some	fine to c	coarse grain	ed san	d, occasional								
			B 1.10m	1	- ۳ 1.0 –		сі	1.10m	Sandy Cl grey, son	me c	<pre>/: shadin calcite no</pre>	ig light bro odules, lov	own, han w to mec	d, orange br liun plasticit	rown ar y 	nd dark brown		:	I			×	, `1.00: HP In-situ >5(	00 kPa
			1.40m B	<u>1</u>	- - 15		sc		medium	grair	ined san	d, high sil	t content	t, grading si	andy si	ilt in parts.			9					
			1.70m	<u>1</u>				1.80m																
			2,20m	,	بی 2.0 –		CL-CI	2.20m	Silty CLA grained s	AY: h sand	hard, low d, light bi	v to mediu rown grey	um plasti to white	city, yellow t in parts, hig	brown, gh silt c	trace of fine content			т				, 2.00: HP In-situ >50	00 kPa
			B 2.50m	1	- - 25		> > >	2.2011	XW SILT extremely friable, w	TSTC ly to veath	ONE/MU highly w hered to	JDSTONE veathered gravel.	: extrem , yellow l	ely low to ve brown and p	ery low bale gre	strength, ey, laminated,	Σ						WEATHERED ROO	CK .
					1 103.0		CL-CI											1						
			3.00m B	<u>1</u>	3.0			3.00m	XW MUD weathere	DST( ed, li	ONE: ex	tremely lc wn grey, c	ow to ver brange/re	y low streng	ith, exti d grey	reme to highly brown, friable								
			3.50m	1	- - 3.5 —		-																	
					302.0																			
					4.0			4.00m	becoming	ig ve	ery low st	trength, hi	ighly wea	athered, les	s friable									
					- 4.5 —			4.50m	becoming	 ig ve	ery low to	o low strer	ngth, hig	hly to mode	 rately v	weathered in	_							
					301.0			4.80m	EXCAVA	ATIO nd w	ON TPB0 vater end	)8 TERMII	NATED A	AT 4.80 m					_					
					5.0				Test pit b	back	kfilled on	. completio	on.											
					- 5.5 — -	-																		
					0.00 6.0																			
					-																			
					6.5																			
					- 500.																			
	PHO NOTE	IOGRA ES	PHS [		YES		[	$\bowtie$	NO	_							CATION	SYME	301 9	3.8		2014	ISISTENCY	
METHO N N E E BH B B B R R SUPPO T T	DD latura Existing Backho Bulldoz Ripper DRT Timber	Expos g Exca be Buck er Blac	ure vation ket de	PE W/		TION 	o Res 3 Wat Date s w low	istance er hown	2		SAMPL           U50         -           D         -           B         -           MC         -           HP         -           VS         -           PBT         -	LES & FIE Undistu 50 mm Disturb Bulk Di Moistur Hand F Vane S R-Rem Plate E	ELD TES urbed Sa diamete bed Sam isturbed re Conte Penetron Shear; P- nouded ( Bearing T	ample er ple Sample ent neter (UCS -Peak, uncorrected Fest	kPa) d kPa)	CLASSIFIC SOIL Ba Class MOISTUF D - Dr M - Mc W - W	CATION : DESCRI sed on U sification RE y bist et	SYME IPTIO nified Syste	BOLS DN I em	5&		CON REL /S - St /St - /D /D	ISISTENCY/ ATIVE DENSITY - Very S - Soft - Firm - Stiff - Very S - Hard - Very L - Loose - Mediur - Dense - Very D	oft tiff oose n Dense ense
See Exp details o & basis	olana of abb of de	tory N previat script	otes for ions ons.								SM	EC A	AUS'	TRAL	IA						•			5MEC

PROJECT : Roma Flood LOCATION : Roma	Mitigation	EXCAVATION - GEO CLIENT : C FEATURE :	LOGICAL LOG	PI FIL S⊦	IT NO : TPB09 LE / JOB NO : 30031182 HEET : 1 OF 1
POSITION : E: 679840.54	10, N: 7061655.350 (5	5 MGA94) SUI	RFACE ELEVATION : 301.807	(AHD)	
EQUIPMENT TYPE : CAT	Excavator 23T	ME	THOD: Excavator		
EXCAVATION DIMENSION	IS : 1.20 m WIDE	LOG			
DRILLING			MATERIAL		
K E PENETRATION H SUPPORT GROUND WATER LEVELS SAMPLES & FIELD TESTS	ELEVATION DEPTH (m) GRAPHIC LOG CLASSIFICATION SYMBOL	MATERIAL DESC Soil Type, Colour, Plasticity or F Secondary and Minor (	RIPTION article Characteristic Somponents	CONDITION CONSISTENCY RELATIVE DENSITY 01 2 000	STRUCTURE & Other Observations
		Silty CLAY: very stiff to hard, medium pl fine grained sand, grass and rootlets to	asticity, dark brown grey, trace of 0.5m	VSt - H	ALLUVIUM + + 0.30: HP In-situ >500 kPa
0.70m B 1.00m 1.20m	о с-с-сн с-с-сн с-с-сн	becoming hard, light brown to brown, tra sand, less than trace of fine gravel	light brown, slight orange brown,		×0.80: HP In-situ >500 kPa
1.50m		.00mshading vellow brown on sand content		T	
2.50m B	2.5	.50m Clayey GRAVEL: very dense, yellow bro some fine grained sand.	wn and light brown grey, mottled,	<u>а</u>	
3.00m	3.0 - 20	.00m			-
B 3.20m	SM	50m	asticity, yellow brow grey	I	
4.30m	0 86 4.0 - - - - - - - - - - - - - - - - - - -	brown, trace of clay content in pockets,	race of fine to coarse gravels		
B 4.60m 5.00m	4.5 - 1	shading yellow brown, moist		۵	
5.50m	- 5.5 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	becoming fine to coarse grained, some subangular gravels and cobbles to 120r pockets, higher moisture content .80m	ine to coarse, rounded to m, trace of clay content in ≥		
		occasional yellow/orange brown laminal very low strength siltstone inclusions, te pockets EXCAVATION TPB09 TERMINATED A No ground water encountered. Test pit backfilled on completion.	ed fine to coarse gravel sized, nding weathered siltstone in		
PHOTOGRAPHS		2 NO			
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	YES PENETRATION <sup>y</sup> <u>w</u> <u>w</u> <u>x</u> <u>x</u> <u>x</u> No Resis water WATER 10 Oct., 73 Wate Level on Date sh water inflow water outflow	SAMPLES & FIELD TEST U50 - Undisturbed Sat 50 mm diameter D - Disturbed Samp B - Bulk Disturbed S MC - Moisture Conter HP - Hand Penetrom VS - Vane Shear; P-I R-Remouded (u PBT - Plate Bearing Te	rs CLASSIFICATION SOIL DESCR Based on U Classification ter (UCS kPa) Peak, ncorrected kPa) est	SYMBOLS & IPTION nified System	CONSISTENCY/ RELATIVE DENSITY         VS       - Very Soft         S       - Soft         F       - Firm         St       - Stiff         H       - Hard         VL       - Very Loose         L       - Loose         MD       - Medium Dense         D       - Dense         VD       - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUST	RALIA		

PF	ROJI DCA	ECT TIOI	 N :	Roma Roma	I Flood N	<i>l</i> itigatio	n		EXCA	VA		- GEC	OLOGIC Ostwald Bro	CAL	LOG			F	<b>PIT I</b> FILE / SHEE	NO Joe T :	: <b>TPB10</b> 3 NO : 30031182 1 OF 1
P	DSIT	ION	1 :	E: 67	8031.11	6, N: 70	62797.	333 (	55 MGA94)			SI		LEVATIC	ON : 300.5	586 (4	AHD)				
E(	JUIE A⊥E	ME' אי			: CAT	Excavat 13	tor 3T					M	ETHOD : E	Excavato	r				(	СНі	
E	KCA			DIME	ENSIONS	S : 0.6	0 m WI	DE						. 0101							
			D	RILLIN	١G	1							М	<b>IATERIA</b>	L		1	1			
μ Π	E PENETRATION	т	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	oil Ty	MATE pe, Colour, Secondar	ERIAL DESC Plasticity or ry and Mino	CRIPTION r Particle Chara r Components	racteristic S		MOISTURE	CONSISTENCY RELATIVE DENSITY	5 1	요 0152	20	STRUCTURE & Other Observations
Í						- 0.0		СН	Silty CLA grained s	Y: ve sand,	ery stiff to ha grass and i	ard, high plas rootlets to 0.	sticity, dark gre .2m	ey brown, tr	race of fine		T t				ALLUVIUM
					0.30m B	- 300		сн	0.20m	light g	grey brown,	 occasional v	white calcite no	 odules to 10	0mm		N	-			
					0.60m	0.5		сн	0.70m	trace	e of fine grai	ined sand				M-D				×	∕`0.50: HP In-situ >500 kPa
									no sand c	 conte	nt, without v		nodules							   *	<ul> <li>√1 00: HP In-situ &gt;500 kPa</li> </ul>
						- 299.5		СН	4.50m								т				
					4.05	1.5			0.1.50m occasiona fissures	 al fiss	suring, occa	 sional dark (	grey to black ve		ing along	Σ					
					1.90m B 2.20m	2.0 — 9.867		СН													
					2.60m	2.5-			2.50m Sandy CL	LAY:	hard, low to	medium pla	asticity, light bro	own/yellow	r brown,						
2:57 8.2.900					B 2.90m	- 298.0		CL-CI		euluff	n yraineu sa	mu, myn silt	Contell			Σ	т				
						- <del>0.6</del> <del>0.6</del> 		2	EXCAVA No ground Test pit ba	TION nd wat backfil	I TPB10 TEI ter encounte lled on comp	RMINATED ered. pletion.	AT 3.00 m								
PJ < <drawingru< td=""><td></td><td></td><td></td><td></td><td></td><td>3.5 -</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></drawingru<>						3.5 -															
						297.0															
						4.0-															
NOTES     YES     NO       METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Buldozer Blade       R     Ripper         VES     NO         SAMPLES & FIELD TESTS       U50     - Undisturbed Sample       50 mm diameter       D     - Disturbed Sample       B     - Buldozer Blade         WATER         SAMPLES & FIELD TESTS         CLASSIFICATION SYMBOL         Soil DESCRIPTION         Based on Unified       Classification System         MOISTURE         D - Dry										S &	<b>0</b> <b>F</b> 9 9 1	CON REL /S 3 St /St - /St	ISISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard								
BANE LIBKARY -	SUPF	PORT	r bering				0 Oct., 7 evel on l vater inflo vater out	3 Wat Date s ow low	er hown	F F	HP - Ha /S - Va R-I PBT - Pla	and Penetro ine Shear; F Remouded ate Bearing	meter (UCS kF P-Peak, (uncorrected k Test	Pa) N kPa) V	A - Moist V - Wet				\ L N C	/L MD D /D	- Very Loose - Loose - Medium Dense - Dense - Very Dense
SMEC BRIS	ee Ex tails basis	kpla of a s of	nator Ibbre desc	y Note viatior ription	es for ns s.					ę	SMEC	AUS	TRALI	A							SMEC

EXCAVATION - GEOLOGICAL LOG           PROJECT : Roma Flood Mitigation         CLIENT : Ostwald Bros Pty Ltd           LOCATION : Roma         FEATURE :	PIT NO : TPB11 FILE / JOB NO : 30031182 SHEET : 1 OF 1									
POSITION         E: 678060.447, N: 7062993.219 (55 MGA94)         SURFACE ELEVATION         301.193 (AHD)           FOURDMENT TYPE         AT F. W. M. OT         METHOD         Surface ELEVATION         301.193 (AHD)										
EQUIPMENT TYPE : CAT Excavator 31     METHOD : Excavator       DATE EXCAVATED : 2/10/13     LOGGED BY : CM	CHECKED BY : JSM									
EXCAVATION DIMENSIONS : 0.60 m WIDE										
$\begin{array}{c c} & & & \\ \hline \\ & & & \\ \hline \\ \hline$										
MATERIAL DESCRIPTION MATERIAL DESCRIPTION Solid Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components Secondary and Minor Components	STRUCTURE & Other Observations 5 10 1520									
CH Silty CLAY: very stiff to hard, high plasticity, dark brown grey, trace of fine grained sand, grass and rootlets to 0.15m	ALLUVIUM -									
CI-CH nodules to 20mm 0.5										
1.00m       1.00m         1.0       Sandy CLAY: hard, light to medium plasticity, yellow brown, fine grained sand, some silt content, occasional white powdery calcite nodules to 10mm, varying sand content tending towards sandy clay/clayey sand in parts, higher moistue content         1.30m       1.30m         B       increasing silt content, shading yellow brown, slight light grey brown, less silt content										
2.20m becoming medium plasticity, shading toward light grey brown, less sand CL-Cl content 2.5 CL-Cl content Less sand content 2.60m										
grading silty clay, some fine grained sand in parts 2.80m Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow brown, some fine grained sand 3.0 - CI-CH CI-CH CI-CH CI-CH Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow cI-CH Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow cI-CH Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CLAY: hard, medium to high plasticity, light grey brown, slight yellow Silty CL										
3.20m     3.20m       EXCAVATION TPB11 TERMINATED AT 3.20 m       No ground water encountered.       Test pit backfilled on completion.       3.5										
PHOTOGRAPHS 4.0 4.0 YES NO										
PHOTOGRAPHS NOTES       YES       NO         METHOD       N       Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper       PENETRATION Sum IIIIS IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII										
See Explanatory Notes for details of abbreviations & basis of descriptions. SMEC AUSTRALIA	SMEC									

PR LO	OJE CATI	CT ION	: F : F	Roma Roma	Flood N	litigatio	n		EXCAVATION - GEOLOGICAL L CLIENT : Ostwald Bros Pty Ltd FEATURE :	JOG			<b>P</b> FI	ITN LE/、 HEET	IO: JOB NC	<b>TPB12</b> D : 30031182 DF 1
PC	SITI	ON	: E	: 677	950.58	3, N: 70	62930.	873 (	55 MGA94) SURFACE ELEVATION	N : 301.3	31 (/	AHD)				
EC		MEN			: CAT I	Excavat I3	or 3T			•				<u>ر</u>		
EX	CAV				NSIONS	S : 0.60	0 m WI	DE						C		
			DR	ILLIN	IG				MATERIAL							
Æ	PENETRATION	- I	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components		MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	5 10	2 2 152	0	STRUCTURE & Other Observations
Í	Ī	Ī				- 0.0		сн	Silty CLAY: very stiff to hard, high plasticity, brown grey, trace of grained sand, grass and rootlets to 0.15m	f fine	۵	St - H				
					0.20m B			— — СН	0.20m			>				
					<b>0.40m</b> B	0.5 -			0.40m becoming hard, high plasticity, brown trace of fine grained sand, white powdery calcite nodules to 20mm diameter	 , some						-
					0.70m			сн								
						1.0 — -			1.00m shading light brown, slight yellow brown, higher moisture content frequent smaller white nodules to 10mm to 1.5m							-
						- 0. 000 - 1.5		сн			M - M	т				-
						- - - - - - - - - - - - - - - - - - -										-
						- 0. 667 - 2.5 -		— — СІ-СН	2.20m	 ge brown, fine						
					2.70m			сі-сн	2.70m							
2.30 0.2.2					B 00m	298.5		СІ	some pale grey, dark grey brown, light grey brown mottling, fine sand, some medium grains	grained	D - M	т				
									EXCAVATION TPB12 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.							
						- 0. 867 - 3.5 —										
	ЛЕТНО	PHO NOT	TOGI ES	RAPHS	P	YES		[	NO SAMPLES & FIELD TESTS CLA				S &	C	ONSIS	TENCY/ VE DENSITY
N       Natural Exposure       Solid DESCRIPTION         N       Natural Exposure       Usu ±±       Usu ±±         N       Natural Exposure       Usu ±±       Usu ±±         N       Natural Exposure       Usu ±±       Usu ±±       Solid DESCRIPTION         B       Based on Unified       Solid DESCRIPTION       Based on Unified         Classification System       No Resistance       D       Disturbed Sample         B       Bulklozer Blade       WATER       Moisture Content       MOISTURE         SUPPORT       10 Oct., 73 Water       Vare Share Peak,       No Resistance       No Solid DESCRIPTION         T       Timbering       10 Oct., 73 Water       Vare Share Peak,       No Noist       Noist         PBT       Plate Bearing Test       Noist       Noist       Noist										×SFS∨H∨L≥DV	t S S S S S S L ID D	<ul> <li>Very Soft</li> <li>Soft</li> <li>Soft</li> <li>Firm</li> <li>Stiff</li> <li>Very Stiff</li> <li>Hard</li> <li>Very Loose</li> <li>Loose</li> <li>Medium Dense</li> <li>Dense</li> <li>Very Dense</li> </ul>				
Se dei dei & t	e Exp tails o basis	olana of abl of de	tory prev scri	Note iation ptions	s for s s.				SMEC AUSTRALIA							SMEC

PROJEC	CT : Roma ON : Roma	a Flood M	litigatio	n	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			FILE / SHEET	<b>NO</b> : <b>TPB13</b> JOB NO: 30031182 T: 1 OF 1			
POSITIC	ON : E: 67	9251.71	1, N: 70	60870.038 (	55 MGA94) SURFACE ELEVATION : 297.	778 (A	AHD)					
DATE EX	XCAVATED	: CAT I ): 2/10/1	=xcavai 3	or 31	LOGGED BY : CM			C	CHECKED BY : JSM			
EXCAVA		INSIONS	6 : 0.6	0 m WIDE								
z		NG ∞Ω	z ~	Z	MATERIAL		×					
- VE - E - F - F - F - F - F - F - F - F - F - F	SUPPORT GROUND WATI	SAMPLES	+ ELEVATIO	GRAPHIC LOG CLASSIFICATIV SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	5 10152	STRUCTURE & Other Observations			
			297.5	СН	Silty CLAY: very stiff to hard, high plasticity, dark grey brown, grass and rootlets to 0.15m	٥	VSt - H		ALLUVIUM			
		0.60m B	0.5		0.60m	_						
		1.00m	- 582 - 782 - 1.0 -	СН	1.20m				-			
		B		СН	grey brown, slight light brown	- W- D	т		-			
			- 96 7 2.0 —	сн	1.80m         slightly higher silt content         2.10m         trace of fine grained sand, shading lighter brown, slight yellow brown	_			-			
		2.40m B 2.60m		CH	2.40m     Sandy CLAY: hard, low to medium plasticity, light brown and yellow     brown, fine grained sand, some silt content				-			
014 12:58 8.2.900		3.00m B	- - 0.920 - 3.0 -	CL-Cl	3.00m Silty SAND: dense to very dense, fine grained, yellow brown, some clay	- D	т		-			
awingFile>> 12/0.3/2		3.30m	- 294.5	SM	3.30m EXCAVATION TPB13 TERMINATED AT 3.30 m No ground water encountered.	M - D	D- U					
NFLOOD LEVEE.GPJ < <u< td=""><td></td><td></td><td>3.5 — - 0.460 -</td><td></td><td></td><td></td><td></td><td></td><td>-</td></u<>			3.5 — - 0.460 -						-			
HI KOW	PHOTOGRAPH NOTES	s	⊥ <sub>4.0</sub> — YES									
METHOI N Na BH Ba BH Ba BU SUPPOF T Tin	NOTES       YES       NO         METHOD       PENETRATION       SAMPLES & FIELD TESTS       SOIL DESCRIPTION       CONSISTENCY/ RELATIVE DENSITY         N       Natural Exposure       Support       Samples       Soil Description       Based on Unified       VS       Very Soft         B       Buildozer Blade       No Cet., 73 Water       Soil Description       Based on Unified       St       St       St         SUPPORT       T       Timbering       10 Oct., 73 Water       Mater outflow       VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       D       Dry       VL       Very Soft S       VL         PBT<											
See Expl details of & basis o	lanatory Note f abbreviation of description	es for ns is.			SMEC AUSTRALIA							

PROJ LOCA	JECT ATIO	· : F N : F	Roma Roma	Flood N	litigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT</b>   FILE / SHEE	<b>NO</b> : JOB N T:1	<b>TPB14</b> O : 30031182 OF 1
POSI		N : I	E: 679	9259.91	5, N: 70 Execute	61190.	953 (	55 MGA94) SURFACE ELEVATION : 297	.918 (	AHD)			
DATE	EEX	CAVA	TED	: 2/10/1	Excaval 13			LOGGED BY : CM			(	CHEC	KED BY : JSM
EXCA	AVAT			INSIONS	5 : 0.60	0 m WI 	DE	MATERIAI					
ENETRATION		SUPPORT	OUND WATER LEVELS	AMPLES &	LEVATION (RL) EPTH (m)	3RAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	NSISTENCY RELATIVE DENSITY	DCP		STRUCTURE & Other Observations
<u>۳</u>	<u> </u>		5	ωE	— 0.0 —		G	Sandy SILT: very stiff to hard, low to medium plasticity, dark grey brown,		ö	5 10152	20 AL	LUVIUM
				0.30m B			ML	fine grained sand, some varying clay fines content	٥	VSt - H			
					5.0.5		ML	lower sand content, higher clay fines content	M - 0	т			-
				0.60m B				0.60m Silty CLAY: hard, medium plasticity, dark grey brown, some fine sand content, high silt content					-
				0.90m	- - - 54.0 - 1.0 -		СІ		M - D	т			- - -
				1.20m B 1.50m	- - - - - - - - - - - - - - - - - - -			1.20m Sandy CLAY: hard, medium plasticity, light brown fine grained sand					- - -
				2.00m B	- - - - - - - - - - - - - - - - - - -		CI	2.00m Sandy SILT: hard, low plasticity, yellow brown, fine grained sand, some medium grains	- Q	т			- - - - -
				2.30m	- - - - - - - - - - - - - - - - - - -		ML	2.50m Sith: SAND: doppo to your doppo, fine grain and, modium grains, yollow,	M - D	т			- - -
12:58 8.2.900				2.80m B 3.00m			SM	Silty SAND: dense to very dense, fine grain sand, medium grains, yellow brown	M - D	D - VD			-
JFile>> 12/03/2014					-		SM	3.30m					-
LOOD LEVEE.GPJ < <drawin< td=""><td></td><td></td><td></td><td></td><td>ی - 23 3.5 - -</td><td>-</td><td></td><td>No ground water encountered. Test pit backfilled on completion.</td><td></td><td></td><td></td><td></td><td>- - - -</td></drawin<>					ی - 23 3.5 - -	-		No ground water encountered. Test pit backfilled on completion.					- - - -
ROMAF	pi		RAPH										-
ANNELIBRARY - COLOUR.GLB LOG TEST PIT	PH NO Natu Exis Back Bullo Ripp PPOR Timb	iral Ex ting Ex ting Ex toper f ber	posure ccavati sucket 3lade	e on v	YES	0 Oct., 7 evel on l vater inflo	[ No Res 73 Wate Date st ow flow	SAMPLES & FIELD TESTS       CLASSIFICA SOIL D         stance       U50 - Undisturbed Sample 50 mm diameter       Baser Classific         D - Disturbed Sample       B         B - Bulk Disturbed Sample       MOISTURE         MC - Moisture Content       D - Dry         HP - Hand Penetrometer (UCS kPa)       VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT - Plate Bearing Test       W - Wet	TION S ESCRIP I on Uni ation S	YMBOLS TION fied ystem	S& (	CONSI: RELAT /S 3 3 5 4 /St 4 /St 4 /L 0 /D	STENCY/ IVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See E details & bas	Expla is of a sis of	natory Ibbrev descr	/ Note /iation iption:	es for Is s.				SMEC AUSTRALIA					

PPO		г.)	Domo	Elood N	litication	2		EXCAVATION - GEOLOGICAL LOG			PIT N	IO : TPB15
LOC		<u>N:</u>	Roma			62079 (	016 (	FEATURE :	0.609 (		SHEET	: 1 OF 1
EQU		N : ENT 1 CAV		: CAT	5, N: 70 Excavat	62078.0 tor 3T	) 010	MGA94) SURFACE ELEVATION : 29: METHOD : Excavator	9.008 (	AHD)	C	
EXC		TION	DIME	NSION	S : 0.60	) m WI	DE				0	
	_	DF		IG (0	1		z	MATERIAL				
L E	F PENETRATION H	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	වි 5 101520	STRUCTURE & Other Observations
					- 539.5		СІ-СН	Silty CLAY: very stiff to hard, medium plasticity, brown, grass and rootlets to 0.4m, trace of fine grained sand	0	VSt - H		ALLUVIUM .
				0.40m B	0.5 -			0.70m	M - D	Ţ		-
				В	- 1.0	1 1 1 1 1 1 1 1 1	SC	Silty SAND: dense to very dense, fine grained sand, brown, varying clay fines content	۵	т		
				1.10m 1.30m B	- 298.5			1.10m Sandy CLAY: Hard, low to medium plasticity, brown to dark brown, fine grained sand				
				1.70m	- 1.5 0.865 - -		CI-CH					
				<u>2.00m</u> B	- 2.0		сі-сн	2.00m light brown to brown	 	т		-
				2.30m 2.50m B			— — сі-сн — —	2.30m				
				2.70m	297.0		сі-сн	2.70m Sithy SAND: very dense, fine to medium orained, vellow/orange brown				
:58 8.2.900				2.80m B			SM	some clay fines, varying clay content grading toward clayey sand in parts	M - D	ą		
wingFile>> 12/03/2014 12				3.00m	<del>- 3.0 -</del> - 2369 - 73 - 73 - 73			3.00m EXCAVATION TPB15 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				
0D LEVEE.GPJ < <ur></ur>					- 3.5  - 500 							
	P	НОТОС	RAPHS		4.0-			7				
Difference       PHOTOGRAPHS NOTES       YES       NO         METHOD       YES       NO         METHOD       PENETRATION       YES       NO         N       Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper       PENETRATION       SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS & SOIL DESCRIPTION Based on Unified       CONSISTE RELATIVE         WATER       WATER       WATER       US0 - Undisturbed Sample B - Bulk Disturbed Sample B - Bulk Disturbed Sample NC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       MOISTURE D - Dry M - Moist W - Wet       VS H D												ONSISTENCY/ ELATIVE DENSITY S - Very Soft - Soft t - Stiff St - Very Stiff - Hard L - Very Loose - Loose D - Medium Dense - Dense D - Very Dense
MEC BRISBAN detai & ba	Expla ils of a isis of	nator abbre desc	y Note viatior ription	es for is s.	<u> </u> " "			SMEC AUSTRALIA				SMEC.

PRC LOC		- :   N :	Roma Roma	Flood N	Mitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / JO SHEET	<b>D</b> : <b>TPB16</b> DB NO : 30031182 : 1 OF 1
POS		N :	E: 678	3560.80	0, N: 70	63094. cavator	100 ( 4т	55 MGA94) SURFACE ELEVATION : 301.7	700 (A	AHD)		
DAT		CAV	ATED	: 30/10	)/13	Javalui		LOGGED BY : CM			Cł	HECKED BY : JSM
EXC	CAVAT	TION DF	DIME RILL IN	NSION:	S : 0.5	) m WI	DE	MATERIAI				
	PENETRATION	SUPPORT	OUND WATER LEVELS	AMPLES &	LEVATION (RL) DEPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	INSISTENCY RELATIVE DENSITY	DCP	STRUCTURE & Other Observations
۳.	<u> </u>	0,	49	Ω≣	- 0.0 -		5	Sandy CLAY: very stiff to hard, low to medium plasticity, dark grey, fine	- 0	SC CC	5 101520	
				0.30m	301.5		сі	0.30m		VSt to		
				0.60m B	0.5		сі — —	0.60m	_			-
				1.00m	0 0 0 0		СІ	dark grey brown signify less sin content, nine grained sand content.				_
				1.20m B	300.5			1.20m becoming light brown, slight light grey brown, fine grained sand	-	т		
				1.50m	- 1.5		СІ		to M			-
				2.00m B	2.0 -		 SМ	1.80m         increasing fine grained sand content         2.00m         Silty SAND: dense, fine to medium grained, light brown/yellow brown,	Di			-
				2.30m	- 500.2		SM	trace of fine grained, dry to moist				
8.2.900				2.70m B	2.5 — - - - - - - - - - - - - - - - - - - -			2.60m grading finer grains, less medium grains, increasing in clay fines content, grading toward clayey sand in pockets, shading light brown, less yellow brown, higher moisture content	_	٥		-
00.21 4103/20121				3.00m	- 3.0 		SM	3.20m				-
					867 - - 3.5			No ground water encountered. Test pit backfilled on completion.				-
					- 598.0							
				<u> </u>	4.0-							
	PHOTOGRAPHS NOTES       YES       NO         WETHOD       YES       NO         N Natural Exposure       SAMPLES & FIELD TESTS       Solid DESCRIF         Bild Schoe Bucket       Sample       50 mm diameter       Disturbed Sample         Bild Schoe Bucket       WATER       No Resistance       U50 - Undisturbed Sample       CLASSIFICATION S SOL DESCRIF         Buildozer Blade       No Resistance       0 mm diameter       D - Disturbed Sample       B - Bulk Disturbed Sample         SUPPORT       10 Oct., 73 Water Level on Date shown water inflow       Notes Share, P-Peak, R-Remouded (uncorrected kPa)       Moist         PBT - Plate Bearing Test       W - Wet											DNSISTENCY/ ELATIVE DENSITY - Very Soft - Stiff - Stiff - Stiff - Very Stiff - Hard - Very Loose - Loose D - Medium Dense - Dense - Very Dense
See deta & ba	Expla iils of a asis of	nator abbre desc	y Note viatior ription	es for Is s.				SMEC AUSTRALIA				SMEC

PROJ	ECT TION	: Ro : Ro	oma	Flood N	vlitigatio	n		EXCA	VATION - C CLIENT FEATUR	E Costwald Bros F	<b>AL LOG</b> Pty Ltd			<b>PIT</b> FILE SHEI	<b>NO</b> / JOB   ET : 1	: <b>TPB17</b> NO : 30031182 OF 1
POSIT EQUIF DATE		: E: IT TY AVAT	679 PE ED	700.60 : KUB : 9/11/	0, N: 70 OTA Ex 13	62784. cavatoi	.200 ( r 4T	55 MGA94)		SURFACE ELEV METHOD : Exca LOGGED BY : C	ATION : 302.8 avator CM	500 (/	AHD)		CHE	CKED BY : JSM
EXCA	VATIO	ON D DRII		NSION G	S : 0.4	5 m WI 	IDE			MATE	ERIAL					
NETRATION		JPPORT	LEVELS	MPLES &	EVATION (RL) (PTH (m)	RAPHIC LOG	SSIFICATION SYMBOL	So	MATERIAL bil Type, Colour, Plast Secondary and	DESCRIPTION icity or Particle Character Minor Components	ristic	DISTURE	ISISTENCY ELATIVE ENSITY	DCP		STRUCTURE & Other Observations
	ιI	ຮ	6K0	SAI	ط - 0.0 ول	0	CLAS					ĭ ₹ 0	S S R D	5 1015	520	
					- 3021		СІ	Sandy CL fine grain grained c 0.30m	AY: very stiff, medium ed sand, grass and roo obbles to 50mm	) plasticity, dark brown an otlets to 0.15m, traces of	d grey, traces of fine to medium	۵	VSt			
			-	0.40m B	 0.0.0 805.0 		СІ	Silty CLA grey brow	Y: hard, medium to hiç ın trace of fine grainec	h plasticity, light brown to sand	) brown, slight					-
			-	<u>0.80m</u> B 1.10m				0.80m XW Muds yellow bro powdery to clayey	stone: extremely low st own, some fine grained calcide nodules >20mi sand	rength, pale grey light bro I sand content in parts wi n diameter, easily friable,	own and orange/ th some white remoulds easily				v	VEATHERED ROCK - - - -
			-	<u>1.30m</u> B <u>1.60m</u>	0 1.5 - 0 1.5 - 100 -	• • • • • • • •		1.30m HW Sand grey, lighi breaks do low streng	istone: very low streng t brown and yellow/ or won to sandy gravel, w gth to very low strengt	th, fine to medium graine ange brown with high fine tith some fines content. S n siltstone and mudstone	d sand, pale is content. Easily ome extremely bands/ pockets	D to M	т			- - - -
			-	2.10m B 2.30m	۔ بر 2.0 – 000 – –			1.80m varying e: sandston slightly cc	tremely low strength, e bends, generally inc barse, breaking down t	very low strength siltston reasing in sand content in o clayey sand/ silty sands	e/ mudstone and parts grading in parts				1	- 80: Slow digging 
0					0, 2.5			2.50m less frequ 2.80m		breaking down to clayey		_				-
					- - 0.8 2 - 5 - - -			EXCAVA No groun Test pit b	TION TPB17 TERMIN. d water encountered. ackfilled on completion	ATED AT 2.80 m						-
					- 3.5 0.66 N -	-										
	PHO	TOGR	APHS		4.0	-	   									-
PHOTOGRAPHS NOTES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper T Timbering METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper T Timbering MOEXTURE T Timbering MOEXTURE NO SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B Bulk Disturbed Sample MOISTURE D - Dry M - Moist W - Wet MOISTURE D - Dry M - Moist W - Wet MOISTURE D - Dry M - Moist W - Wet MOISTURE D - Dry M - Moist W - Wet MOEXTURE D - Dry M - Moist MOEXTURE D - Dry M - Moist MOEXTURE D - Dry M - Moist M - M - Moist M - Mois											/MBOLS TION fied ystem	÷ &	CONS RELA VS S F St VSt H VL L MD D VD	ISTENCY/ TIVE DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Dense - Very Dense		
See E details & basi	xplana of abl s of de	atory I brevia escrip	Note: ation: tions	s for s s.					SMEC A	USTRALIA						

PF	ROJEC	CT : ON :	Roma Roma	I Flood N	litigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	PIT NO : TPB18 FILE / JOB NO : 30031182 SHEET : 1 OF 1
P( E( D/	DSITIC QUIPN ATE E	DN : IENT XCAV	E: 67 TYPE ATED	9698.30 : KUB( : 9/11/	0, N: 70 DTA Exc 13	62762. cavator	700 (! 4T	5 MGA94) SURFACE ELEVATION : 302.100 (AHD) METHOD : Excavator LOGGED BY : CM	CHECKED BY : JSM
E	(CAVA	ATION D		NSION	5 : 0.4	5 m WI	DE	MATERIAL	
VE	E PENETRATION	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	5 IO 1520
					0 -		CI	Sandy CLAY: very stiff, medium plasticity, dark brown and grey, fine grained sand in parts and grass and rootlets to 0.2m	ALLUVIUM
				0.30m B 0.70m	- 305        		SM	.20m     Image: Constraint of the second secon	
				0.90m B 1.10m			сн	.90m Silty CLAY: hard, high plasticity, dark brown and grey to black, with a trace of fine grained sand	
					1.5		сн	50m	
2:58 8.2.900				2.30m B 2.60m	- 3000 - 3000       		сн	.30m Sandy CLAY: hard, high plasticity, dark brown grey to black, with fine to medium grain sand. Varying sand content grading, silty clay some sand content in parts	
A FLOOD LEVEE.GPJ < <drawingfile>&gt; 12/03/2014 11</drawingfile>								EXCAVATION TPB18 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.	
SBANE LIBRARY - COLOUR.GLB Log TEST PI1 KUMA	METHO N Na E Ex BH Ba B Bu R Rij SUPPOI T Tir	PHOTOO NOTES D atural E: ackhoe l aldozer pper RT mbering	xposure xposure fxcavat Bucket Blade	S F	4.0	TION 5 0 Oct., 7 evel on l ater inflo ater outf	o Resi 3 Wate Date slow low	NO         SAMPLES & FIELD TESTS         U50 - Undisturbed Sample         50 mm diameter         D - Disturbed Sample         B - Bulk Disturbed Sample         MC - Moisture Content         HP - Hand Penetrometer (UCS kPa)         VS - Vane Shear; P-Peak,         R-Remouded (uncorrected kPa)         PBT - Plate Bearing Test	LS & CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
ap %	tails of basis c	f abbre	viation	s ioi is s.				SMEC AUSTRALIA	SMEC
PROJECT : Roma LOCATION : Roma	<b>PIT NO</b> FILE / JO SHEET	D: <b>TPB19</b> DB NO : 30031182 : 1 OF 1							
-----------------------------------------------------------------------------------------------	------------------------------------------------------------------------	-------------------------------------------------	---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	----------------------	-----------------------------------	-------------------------------------------	--------------------------------------------------------------------------------------------------------------------------		
POSITION : E: 67	9696.800, N: 70	62735.500 (	55 MGA94) SURFACE ELEVATION : 301	.900 (	AHD)				
DATE EXCAVATED	): 9/11/13		LOGGED BY : CM			CF	IECKED BY : JSM		
EXCAVATION DIME	ENSIONS : 0.50	0 m WIDE							
DRILLII	NG	z	MATERIAL		2				
VE E PENETRATIO H SUPPORT GROUND WATE LEVELS	SAMPLES & SAMPLES & FIELD TEST ELEVATION ELEVATION (RL)	GRAPHIC LOG CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	වි 5 101520	STRUCTURE & Other Observations		
	-	sc	Clayey SAND: dense, high content clay fines, dark grey brown, fine to medium grained sand in parts, trace of fine to coarse angular to subrounded gravels, grass and rootlets to 0.15m, medium plasticity clay fines. 0.25m				ALLUVIUM		
	B		Sandy CLAY: hard, medium plasticity, light brown/ yellow brown, some fine to medium grained sand in parts				-		
	0.60m	sc	0.00-		D to N		-		
	B 0, 0, 0, 0, 0, 0, 0, 0,	sc	Clayey SAND: dense to very dense, fine to coarse grained sand, light brown/ yellow and brown some fine to coarse gravels and cobbels to 50mm				-		
	1.10m 1.20m B		1.10m XW Mudstone/ Siltstone: extremely to very low strength, pale grey and light grey brown and yellow brown, varying bands pockets and sandstone bands, breaking down to clayey sand, some fine to medium gravels				WEATHERED ROCK		
	1.50m						-		
	-			D to M			-		
	2.00m <sup>8</sup> 2.0 –		2.00m grading sandy with less frequent mudstone bands, breaking down to clayey sand, less weathered, higher gravel content				-		
	<u>2.30m</u> بم		2.50m				-		
			grading higher clay content, increase in grey and pale grey mudstone bands, higher moisture content	Σ			-		
	0 66 		3.00m EXCAVATION TPB19 TERMINATED AT 3.00 m				-		
	-		No ground water encountered. Test pit backfilled on completion.						
	بن - 867 ع.5 – -						-		
							-		
PHOTOGRAPH	s <sup>88</sup> 4.0								
METHOD		. <b>TION</b> - ₹	SAMPLES & FIELD TESTS CLASSIFICA SOIL D	TION S	YMBOLS	& CO RE	NSISTENCY/ LATIVE DENSITY		
N Natural Exposur E Existing Excaval BH Backhoe Bucket B Bulldozer Blade R Ripper		0 Oct., 73 Watevel on Dates	istance U50 - Undisturbed Sample Base 50 mm diameter Classifi D - Disturbed Sample <b>MOISTURE</b> B - Bulk Disturbed Sample <b>MOISTURE</b> MC - Moisture Content D - Dry HP - Hand Penetrometer (UCS kPa) hown VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)	d on Uni cation S	fied ystem	VS F St VS H VL L MD	- Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose Decision		
I limbering		ater outflow	PBT - Plate Bearing Test			VD	- Very Dense		
See Explanatory Not details of abbreviation & basis of description	es for ns is.		SMEC AUSTRALIA						

PROJECT : Roma Flood Mitigation CLIENT : Ostwald Bros Pty Ltd											<b>P</b> FI	IT N	IO : IOB NO :	<b>TPB20</b> 30031182				
POS		<u>DN :</u>	Roma E: 679	9734.90	0, N: 70	62785.	500 (	55 MGA94)	Ft	<u>EATURE</u> : SI	JRFACE ELEV	/ATION : 302.	600 (/	AHD)	SI	HEEI	: 1 OF	1
EQU				: KUB	OTA Ex	cavator	4T			M	ETHOD : Exc	avator						
EXC				. 9/11/	S : 0.4	5 m WI	DE				JOGED BT . V					U		
		D	RILLIN	IG	1		7				MAT	ERIAL						
ve e	F PENEIKALION H	SUPPORT	GROUND WATEF LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	M/ bil Type, Colo Secon	ATERIAL DEStour, Plasticity of adary and Mino	CRIPTION r Particle Characte r Components	eristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	2 ) 1520	& C	STRUCTURE Other Observations
					- 0.0 -		sc	Clayey S/ fines, dar	AND: dense, i k grey brown,	fine to medium , grass and roo	grained, medium p tlets to 0.2m	plasticity clay	٥				ALLUV	IUM -
				0.30m B	- ~ -		sc sc	0.20m shading li 0.30m increasing shading d	ight brown, in g clayey sand dark grey brow	crease in fine to and sandy cla wn, decreasing	o medium grained y in parts in fine to medium s	sand content,	-	Q				-
				0.50m	0.5-		sc	0.50mslightly lig	 jther colour w	ith some white	powdery calcide no	odules	-	D to M				-
					- 302		sc	0.70m	n with increa	sing moisture c	 ontent		_					-
					- 1.0 - 1.0 - 2.1 - 2.0			XW Muds pale grey down to lo and silty o	stone/ Siltston / yellow brown ow to medium clay.	ne: extremely lo n, some sandst n to plasticity, fi	w strength, light gr one bands, easily ne to medium grair	ey brown and friable, breaking ned sandy clay					WEATH	HERED ROCK
					-								D to M					-
	1.5																	-
					2.0-	-												-
					300.5													-
					2.5	-		2.60m										-
					300.0	-		EXCAVA No ground Test pit ba	TION TPB20 d water encou ackfilled on c	TERMINATED untered. ompletion.	AT 2.60 m							
					3.0 —	-												-
					299.5	-												-
5					3.5 —	-												-
					299.0													-
					4.0-													_
		PHOTO NOTES	GRAPHS		] YES		[	NO NO										
ME N E BH B R SU	METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering PENETRATION Support T Timbering						lo Res 3 Wat Date s ow flow	istance er nown	SAMPLE           U50         -           D         -           B         -           MC         -           HP         -           VS         -           PBT         -	S & FIELD TE Undisturbed S 50 mm diamel Disturbed Sam Bulk Disturbed Moisture Cont Hand Penetro Vane Shear, F R-Remouded Plate Bearing	STS iample ter nple d Sample ent meter (UCS kPa) -Peak, (uncorrected kPa) Test	CLASSIFICAT SOIL DE Based Classific MOISTURE D - Dry M - Moist W - Wet	FION S SCRIP on Uni ation Sy	(MBOLS TION fied /stem	S &	CIR VS F S V H V L M D V	ONSISTEN ELATIVE I S t St L D	VCY/ DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Dense - Very Dense
See detai & ba	Expl ils of sis c	lanator abbre of desc	y Note viation	es for is s.					SME	EC AUS	TRALIA							

PRO		: R · R	oma	Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG			<b>PIT N</b> FILE / JO SHEET	O: TPB21 DB NO: 30031182
POS EQU DATI	ITION IPMEN E EXC	: E IT T` AVA	: 679 YPE TED	9735.40 : KUB : 9/11/	0, N: 70 DTA Ex 13	62760. cavator	800 (\$ 4T	55 MGA94) SURFACE ELEVATION : 302. METHOD : Excavator LOGGED BY : CM	600 (/	AHD)	Cł	HECKED BY : JSM
EXC	AVATI	DN E DR		NSION G	S : 0.4	5 m WI	DE	MATERIAL				
	T	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 101520	STRUCTURE & Other Observations
					- 0.0 - - 305:2 - 305:2	1       	SM	Silty SAND: loose to medium dense, brown, fine to coarse grained sand, trace of fine to medium rounded to sub-angular gravels. Grass and rootlets to 0.15m	٥	L to MD		ALLUVIUM -
					0.5	( 	SM	varying interbedded bands/ lenses of brown sitly sand and vert stiff dark grey brown, medium plasticity, sandy clay, varying 10mm - 180mm thick generally 50-50		D		-
				0.90m	305.0		SM	0.90m		MD to		-
				B 1.30m	- 1.0 - 23 - 301 - 301		sc	Clayey SAND: dense, medium plasticity, dark brown and grey to black, fine to coarse grained sand				-
				В			СІ	Sandy CLAY: hard, medium to high plasticity, dark brown grey with fine to medium grained sand	to M			-
				<u>1.70m</u>	2.0-		СІ	shading slightly to dark grey brown, increasing sand content and higher moisture content		VSt to H		-
				2.20m B	- 300.5 -			2.20m	-			-
					- 2.5 0.0 0.00 - - -		СІ	3.00m				
					- <u>3.0</u> 			EXCAVATION TPB21 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				-
					3.5							
	PHC	TOGE	RAPHS					NO NO				-
, ME BH B R SUF T	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     In Oct., 73 W       T     Timbering					TION 0 Oct., 7 evel on vater inflor vater out	lo Resi 3 Wate Date show 10w	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	SCRIP on Uni ation S	YMBOLS TION fied ystem	S& CC RE VS S F St VS H VL L ME D VD	DNSISTENCY/ ELATIVE DENSITY - Soft - Firm - Stiff - Hard - Very Losse - Losse D - Medium Dense - Dense D - Very Dense
See I detail & bas	Explana s of ab sis of de	atory brev escri	Note iation ptions	s for s s.				SMEC AUSTRALIA				

PROJECT : Roma Flood Mitigation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	PIT NO : TPB21 FILE / JOB NO : 30031182 SHEET : 2 OF 2
POSITION : E: 679735.400, N: 7062760.800 (55	5 MGA94) SURFACE ELEVATION : 302.600 (AHD)	
EQUIPMENT TYPE : KUBOTA Excavator 4T DATE EXCAVATED : 9/11/13	LOGGED BY : CM	CHECKED BY : JSM
EXCAVATION DIMENSIONS : 0.45 m WIDE		
	MATERIAL	
VE E PENETRATIO A SUPPORT SUPPORT CRONDOWATE CRONDOWATE ELEVATION ELEVATION ELEVATION CRAPHIC LEVATION CLOG CLASSIFICATIC SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	5 101520
A.0       A.0         4.0       A.0         4.0       A.0         98       A.5         0       A.5	NO          SAMPLES & FIELD TESTS         US0         US0	&         CONSISTENCY/ RELATIVE DENSITY S         - Very Soft - Soft 
R     Ripper     WATER       SUPPORT     10 Oct., 73 Water       T     Timbering       See Explanatory Notes for	MC - Moisture Content HP - Hand Penetrometer (UCS kPa) WN VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
details of abbreviations & basis of descriptions.	SMEC AUSTRALIA	SMEC

PROLOC	DJEC <sup>.</sup> CATIC	T : N :	Roma Roma	a Flood N	<i>l</i> itigatio	n		EXCA	VATION - GI	EOLOGICA : Ostwald Bros F	AL LOG Pty Ltd			<b>PI</b> FILE SHE	<b>f nc</b> = / jo ==t :	D: <b>TPB22</b> B NO : 30031182 1 OF 1
PO	SITIO	N :	E: 67	9728.80	0, N: 70	62726.	000 (	55 MGA94)		SURFACE ELEV	ATION : 302.6	600 (A	AHD)			
EQ		ENT		: KUB	DTA Ex	cavator	4T			METHOD : Exca	avator				СН	
EX		TION			S : 0.4	5 m WI	DE			LOGGED BT . C					СП	ECKED BT . JSW
		DI	RILLIN	NG						MATE	ERIAL					
Ψ.	PENETRATION	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	So	MATERIAL DI oil Type, Colour, Plasticity Secondary and M	ESCRIPTION y or Particle Character inor Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	00 5 101	520	STRUCTURE & Other Observations
					- 0.0		SM	Silty SAN medium g	ID: low to medium density grained sand, grass and re	, medium plasticity, bro potlets to 0.25m	own, fine to	۵	L to MD			ALLUVIUM
				0.40m			sc	Clayey Sa medium g	AND: medium density, bro grained sand, 	own to slightly grey bro	own, fine to		MD			-
				0.70	0.5 -		sc	clay in pa	g dense, increasing clay c arts	ontent grading Clayey	sano/ Sanoy		۵			-
				B	_ <sup>®</sup> .			Sandy Cl medium g	LAY: hard, medium plastic grained sand, varying san	ity, brown to grey brow d content, traces of silf	vn fine to ty clay in parts					
				1.00m	– 1.0 – بې کې		сн									-
					۳ .							D to M				-
	1.5						1.60m XW Sand	dstone/ Siltstone: Extreme	ly low strength, pale gr	rey and yellow/					WEATHERED ROCK	
					۳.	•••		light brow to low to with some	vn and light grey brown, so medium plasticity sandy c e fine grained gravels	ome mudstone bands, lay with fine to mediun	Dreaking down n grained sand					-
					2.0 – 19.000	•••		2.20m								-
								becoming content w	g very stiff, highly wheathe vhen broken down	ered in parts, increasing	g in gravel					-
006					- <del>2.5</del> 0.00 0.00 0.00 0.00 0.00			2.50m EXCAVA No groun Test pit b	TION TPB22 TERMINATE to water encountered. backfilled on completion.	ED AT 2.50 m						
2014 12:58 8.2					3.0	-										-
12/03					299.5	-										
.GPJ < <drawir< td=""><td></td><td></td><td></td><td></td><td>3.5 -</td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td></drawir<>					3.5 -	-										-
LOOD LEVEE					. 299	-										-
	P	нотос		s	4.0-		<u> </u>									
LIBRARY-COLOUR.GLB Log TEST PIT 그 1월 권 명 편 전 M	PHOTOGRAPHS NOTES VES VETHOD N Natural Exposure Existing Excavation 3H Backhoe Bucket 3 Bulldozer Blade R Ripper SUPPORT Γ Timbering					NTION 0 Oct., 7 evel on l vater inflo	[ lo Res 3 Wat Date s	NO stance er nown	SAMPLES & FIELD U50 - Undisturber 50 mm diar D - Disturbed S B - Bulk Disturt MC - Moisture Cr HP - Hand Pene VS - Vane Shea R-Remoud	TESTS d Sample neter sample bed Sample ontent trometer (UCS kPa) r; P-Peak, ed (uncorrected kPa)	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	CION SY SCRIP on Unif ation Sy	<b>MBOLS</b> TION fied /stem	5&	COP REL VS S F St VSt H VL L MD D	NSISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense
SMEC BRISBANE deta & bi	e Expla ails of a asis of	anator abbre f desc	y Note viatior ription	es for ns is.	<u> </u>  v	vater out	low		SMEC AU	STRALIA					VD	- Very Dense

PROJECT : Roma Flood Mitigation LOCATION : Roma										VATION - CLIENT	GEOLOGICA : Ostwald Bros F RE :	AL LOG Pty Ltd			<b>PIT</b> FILE SHE	<b>NO</b> / JOI	D : <b>TPB23</b> B NO : 30031182 1 OF 1
F	POS	ITIC	N :	E: 679	767.70	0, N: 70	62789.	900 (	55 MGA94)		SURFACE ELEV	ATION : 303.	300 (A	AHD)			
	)AT	E E)	ENT (CAV	ATED	: 9/11/	13	cavator	41			LOGGED BY : 0	CM				СН	ECKED BY : JSM
E	XC	AVA	TION	I DIME	NSION	S : 0.4	5 m Wl	DE									
	-	7		RILLIN	G			z			MATI	ERIAL		≻			
!	VE E demetadation	F FENEIRALIO	SUPPORT	GROUND WATE LEVELS	SAMPLES & FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC SYMBOL	So	MATERIA bil Type, Colour, Plas Secondary an	L DESCRIPTION ticity or Particle Characte d Minor Components	ristic	MOISTURE	CONSISTENC RELATIVE DENSITY	වි 5 101:	520	STRUCTURE & Other Observations
						-		СІ	Sandy CL medium g	AY: very stiff, mediun grained sand, grass a	n plasticity, light brown to nd rootlets to 0.2m	brown, fine to	۵	VSt			ALLUVIUM - -
						0.5		сі — —	0.50m	medium to high plas	ticity, with dark brown grey	y brown					
						-			increase i	in moisture content							-
						1.0 –		СІ						т			- - -
			0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2					сı	1.20m becoming yellow/ or 1.50m	g hard, medium plasti ange brown, residual	city, grey brown to light gre soil	ey brown and	D to M				
						5 -			XW Siltst yellow/ or easily fria grained s	one/ Mudstone: extre ange brown with som ble, breaking down to and, sandy clay and	mely low strength, light gre e grey brown, some sands low to medium plasticity, silty clay in parts	ey brown and stone bands, fine to medium					WEATHERED ROCK - -
						2.0-											
			_			0.00			2.40m EXCAVA No groun	TION TPB23 TERMIN d water encountered.	IATED AT 2.40 m						-
						- ju	-		Test pit b	ackfilled on completic	n.						-
						3.0	-										-
						300.0	-										-
						3.5	-										-
						- 586.5											-
		F	PHOTO	GRAPHS		] YES		[	NO NO								
	METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering Con Evaluation Materia						0 Oct., 7 evel on vater inflo	lo Resi 3 Wate Date sl ow flow	istance er nown	SAMPLES & FIE U50 - Undist. 50 mm D - Disturb B - Bulk Di MC - Moistur HP - Hand F VS - Vane S R-Rem PBT - Plate B	LD TESTS Irbed Sample diameter ed Sample sturbed Sample e Content Penetrometer (UCS kPa) hear; P-Peak, ouded (uncorrected kPa) earing Test	CLASSIFICAT SOIL DE Based Classific MOISTURE D - Dry M - Moist W - Wet	FION SY ESCRIP on Unit ation Sy	/MBOLS TION fied /stem	\$&	CON REL VS S F St VSt H VL L MD D VD	ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
	etai ba	Expl Is of sis o	anato abbre f dese	ry Note viation criptions	s for s s.					SMEC A	USTRALIA						

PR LO	OJEC CATIC	T : N :	Roma Roma	I Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / J SHEET	O: TPB24 OB NO : 30031182 : 1 OF 1
PC EG DA	SITIO	N : ENT (CAV	E: 67 TYPE ATED	9768.10 : KUB( : 10/11	0, N: 70 DTA Ex /13	62751. cavator	400 ( 4T	55 MGA94) SURFACE ELEVATION : 303 METHOD : Excavator LOGGED BY : CM	.200 (	AHD)	С	HECKED BY : JSM
EX	CAVA	TION DI	DIME RILLIN	NSIONS NG	S : 0.4	5 m WI	DE	MATERIAL				
VE	E F H	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	වි 5 101520	STRUCTURE & Other Observations
					- 0.0		СІ	Sandy CLAY: very stiff, medium plasticity, dark grey brown, fine to medium grained sand, grass and rootlets to 0.25m 0.25m Silty SAND: medium dense, brown/ dark grey brown, fine to coarse	٥	VSt		ALLUVIUM
	0. 970 1.1						SM	grained sand, varying interbedded bands' lenses of silly sand and very stiff, medium plasticity sandy clay, varying 10-120mm thick	D to M	ΦM		
					- 0, - - - 1.5		. SM	1.10m Sand: medium dense to dense, light brown to brown, trace of silt content, trace of fine to medium grained sand		to D		
				1.70m B 2.00m	- <u>19</u>		SC	Clayey SAND: dense, fine to medium grained, medium plasticity fines, yellow brown, sand, grading clayey sand in parts		đ		- - -
				2.20m B	301.0		sc	becoming yellow brown, fine to coarse grained sand, some fine to coarse rounded to sub-angular gravels and cobbels to 150mm	Z			
12:58 8.2.900				2.50m	- 2.5 - - - - - - - - - - - - - - - - - -		sc	grading coarser sands, increase in gravel content, decreasing in clay content, grading gravelly sand in parts				
vingFile>> 12/03/2014					- 0.00 0.00 -			EXCAVATION TPB24 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				
DOD LEVEE.GPJ < <drav.< td=""><td></td><td></td><td></td><td></td><td>3.5 — - - - - - - - - - - -</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></drav.<>					3.5 — - - - - - - - - - - -							
		PHOTO	GRAPHS	S			 [	NO NO				-
SANE LIBRARY - COLOUR.GLB Log TEST → SA B B B A Z	METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering T Timbering					TION 	o Res 3 Wat Date s w 1ow	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	t	YMBOLS TION fied ystem	S& C(RR) RR SS SS H VS H UL M D VI	DNSISTENCY/ ELATIVE DENSITY S - Very Soft - Soft - Firm - Stiff St - Very Stiff - Hard - Very Loose - Loose D - Medium Dense - Dense D - Very Dense
MEC BRISE det & b	e Expla ails of asis of	anator abbre f desc	y Note viatior ription	es for ns s.				SMEC AUSTRALIA				

PROJ LOCA	ECT	· :   N :	Roma Roma	Flood N	<i>l</i> itigation	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			PIT N FILE / SHEET	NO: TPB25 JOB NO: 30031182 T: 1 OF 1
POSI		ו : י דא	E: 679	9771.70	0, N: 70	62714.	600 ( 4т	55 MGA94) SURFACE ELEVATION : 30: METHOD : Excepted	3.600 (	AHD)		
DATE	EX	CAVA	ATED	: 10/11	/13	Javalu		LOGGED BY : CM			C	CHECKED BY : JSM
EXCA	VAT			INSION	S : 0.45	5 m WI	DE	MATERIAI				
PENETRATION		SUPPORT	OUND WATER	AMPLES &	LEVATION (RL) EPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	INSISTENCY RELATIVE DENSITY	DCP	STRUCTURE & Other Observations
<u>, ж</u> ш,	Ť		6R GR	ωE	— 0.0 —		5	Silty SAND: lose, brown, fine to coarse grained sand, grass and rootlets		ö	5 10152	0 ALLUVIUM
					303.5	1     	SM	to 0.2m 0.40m Sandy CLAY: very stiff, low to medium plasticity, dark brown grey to black				
				0.60m B	0.5		C∟	fine to medium grained sand 0.60m becoming medium plasticity, dark grey brown and grey brown, decreasing in sand content				-
				1.00m	- - 1.0 - 30 - 30 -		CI		D to M	VSt		-
	1.90m B							1.90m Sandy CLAY: very stiff to hard, medium to high plasticity, light brown/				
				<b>2.20m</b> B	- 2.0 - 301 - 302 		СІ — — СІ	yellow brown and slight light grey brown, fine to medium grained sand, grading sandy clay in parts 2.10m becoming hard, low plasticity, light yellow brown, fine to coarse grained sand, some rounded to subangular fine to coarse gravels and cobbles to 80mm, grading clayey sand in parts		т		
		-		2.50m	- 2.5 0.10 	/////     	SM	2.40m     Silty SAND: dense, light yellow brown, fine to coarse grained sand, some rounded to sub angular fine to coarse gravels and boulders to 250mm	2	۵		-
ile>> 12/03/2014 12:58 8.2.900								EXCAVATION TPB25 TERMINATED AT 2.70 m No ground water encountered. Test pit backfilled on completion.				
DD LEVEE.GPJ < <drawingf< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td></drawingf<>												-
MA FLOC					-							
	PH NC		RAPHS	ـــــــــــــــــــــــــــــــــــــ	⊥ <sub>4.0</sub> ] YES	ļ	<u>ו</u> ן	NO NO		1		
ANE LIBRARY - COLOUR GLB LOG 1ES IF	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering						lo Res 3 Wat Date s ow flow	stance SAMPLES & FIELD TESTS SOLL U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	ATION S DESCRIP d on Unication S	YMBOLS TION fied ystem	S& C R V S F S V H V L M D V	CONSISTENCY/         RELATIVE DENSITY         VS       - Very Soft         St       - Firm         St       - Stiff         St       - Very Stiff         I       - Hard         IL       - Very Loose         ID       - Medium Dense         ID       - Dense         ID       - Very Dense
See E details & basi	xplai s of a is of	hator bbre desci	y Note viatior	es for Is s.				SMEC AUSTRALIA			I	

PR LO		:T : DN :	Roma Roma	<b>PIT</b> FILE SHE	<b>f no</b> E / Joe Eet :	: <b>TPB26</b> 3 NO : 30031182 1 OF 1					
PC EC DA	SITIC UIPM TE EX	N : ENT KCAV	E: 679 FYPE ATED	9798.200 : KUBC : 10/11	), N: 70 DTA Exe /13	62789.9 cavator	900 (! 4T	5 MGA94)         SURFACE ELEVATION : 304.100 (AHD)           METHOD : Excavator         LOGGED BY : CM		CHE	ECKED BY : JSM
ΕX	CAVA	LION DI	DIME	NSIONS NG	5 : 0.50	) m WIL	DE	MATERIAL			
VE	e penetration F H	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	00 5 101	520	STRUCTURE & Other Observations
				0.30m B	- 0.0 -		CI	Sandy CLAY: very stiff, medium plasticity, dark grey brown, fine to medium grained sand, grass and rootlets to 0.3m  .30m Silty CLAY: hard, medium to high plasticity, dark grey brown, traces of fine			ALLUVIUM - -
				0.60m	- - 0.5 - - - - - - - - -		CI	to medium grained sand			- - -
				<u>0.90m</u> B	1.0 —			.90m XW Mudstone/ Siltstone: extremely low strength, pale grey and yellow brown, laminated, friable, break down to medium plasticity silty clay		-	WEATHERED ROCK
				1.20m	- <sup>100</sup>			.50m			-
					302.5			HW Siltstone/ Sandstone: extremely to very low strength, pale grey and yellow brown, breakes down to gravelly fine to medium grained sand and some fines			-
					- 2.0 — 0. 0 — 0. 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0 0.0 0 0.0 0 0.0 0 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			.30m			- - -
00					- 2.5 — 100 —			increase in fine to coarse grained sand, sandstone beds, decreasing fines content			- - -
2014 12:58 8:2.91					3.0-			.90m EXCAVATION TPB26 TERMINATED AT 2.90 m No ground water encountered. Test pit backfilled on completion.			-
awingFile>> 12/03					301.0						-
LEVEE.GPJ < <u< td=""><td></td><td></td><td></td><td></td><td>3.5 - -  - - -</td><td></td><td></td><td></td><td></td><td></td><td>-</td></u<>					3.5 - -  - - -						-
	F	PHOTO	GRAPHS	s	4.0		 ۱	Z NO			-
BANE LIBRARY - COLOUR.GLB LOg 1E317.	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering						D Resi	tance SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	&	CON REL VS S F St VSt H VL L MD D VD	ISISTENCY/ ATIVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Very Stiff - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
SMEC BRIS det & b	e Explais of asis o	anator abbre f desc	y Note viatior ription	es for ns s.				SMEC AUSTRALIA			

PROJECT : Roma Flood Mitigatio	n CLIENT : Ostwald Bros Pty Ltd FEATURE :	PIT NO : TPB27 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679811.800, N: 70 EQUIPMENT TYPE : KUBOTA Ex DATE EXCAVATED : 10/11/13	62756.500 (55 MGA94)         SURFACE ELEVATION : 303.600 (AHD)           cavator 4T         METHOD : Excavator           LOGGED BY : CM         CM	CHECKED BY : JSM
EXCAVATION DIMENSIONS : 0.5 DRILLING	0 m WIDE MATERIAL	
E PENETRATION SUPPORT SROUND WATER LEVELS SAMPLES & FIELD TESTS ELEVATION CFPT (L)	ND         MATERIAL DESCRIPTION         NO           NO         MATERIAL DESCRIPTION         NO         NO           Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components         Soil Type, Colour, Plasticity or Particle Characteristic         Soil Type, Colour, Plasticity or Particle Characteristic	5 101520
0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.	CI       Sandy CLAY: very stiff, medium plasticity, dark brown grey, fine to medium grained sand, grass and rootlets to 0.25m       ©       §         0.25m       Sitty SAND: medium density, dark grey brown, fine to coarse grained sand, interbedded bands/ layers of silty sand and very stiff, medium plasticity with dark grey brown sandy clay, 10-50mm thick       ©       §         0.50m       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       §         CI       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       ©         CI       CI       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       ©         CI       CI       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       ©         CI       CI       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       ©         Six       CI       Sandy CLAY: hard, medium plasticity, dark brown grey to black, fine to medium grained sand       ©       ©         Six       Six       Six       Six       Six       ©       ©       ©       ©       ©       ©         Six       Six       Six       Six       Six       Six       Six       ©       ©       ©	ALLUVIUM
2.40m B 2.5- 2.70m	SC 2.40m XW Siltstone/ Mudstone: extremely low strength, light brown/ yellow brown, pale grey and white, interbedded, some fine to medium grain sandstone bands. very low strength/ highly weathered in parts, breaks down to clayey sand, some fine to medium gravels. 2.70m HW Sandstone: extremely to very low strength, fine to coarse grained sand, light brown and yellow/ orange brown and grey brown, breaks down to gravely sand, some fines, some sittone bands.	WEATHERED ROCK
	2.90m     EXCAVATION TPB27 TERMINATED AT 2.90 m     No ground water encountered.     Test pit backfilled on completion.	
PHOTOGRAPHS NOTES WETHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering See Explanatory Notes for details of abbreviations	NO       SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS         5       U50 - Undisturbed Sample       SOIL DESCRIPTION         0       0.0 ct., 73 Water       D - Disturbed Sample       Based on Unified         WH       - Bulk Disturbed Sample       MOISTURE         0       - Moisture Content       D - Dry         HP       - Hand Penetrometer (UCS kPa)       D - Dry         VS       - Vane Shear, P-Peak,       R-Remouded (uncorrected kPa)         PBT       Plate Bearing Test       Wet         SMEC AUSTRALIA       SMEC AUSTRALIA	S& CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense

PROLOC		T: N:	Roma Roma	Flood N	litigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / SHEE	NO: TPB28 JOB NO: 30031182 T: 1 OF 1
PO			E: 679	)819.100	0, N: 70	62730.	700 (	55 MGA94) SURFACE ELEVATION : 30: METHOD : Exceptote	8.700 (/	AHD)		
DA	TE EX	(CAV	ATED	: 10/11	/13	avalui	-	LOGGED BY : CM			(	CHECKED BY : JSM
EX	CAVA	TION	DIME RILL IN	NSIONS IG	5 : 0.50	) m Wll	DE	MATERIAI				
	ENETRATION	UPPORT	DUND WATER	AMPLES &	EVATION (RL) EPTH (m)	SRAPHIC LOG	SSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	OISTURE ONDITION	NSISTENCY RELATIVE DENSITY	DCP	STRUCTURE & Other Observations
٣.	<u> </u>	s s	GRO	S/ FIE	□ ⊡ - 0.0 -		CLA	Sandy CLAY: very stiff, medium plasticity, dark brown and grey, fine to	20	8	5 10152	0 ALLUVIUM
					303.5		СІ	medium grained sand, grass and rootlets to 0.2m 0.30m Silty SAND: medium density, brown, fine to coarse grained sand, varying	0	VSt		
				0.60m	0.5		SM	interbedded bands/ layers of silty sand and very stiff, medium plasticity with dark grey brown sandy clay, 10/80mm thick		QW		-
				B	303.0		СІ	Sandy CLAY: high plasticity, dark brown grey to black, fine grained sand silty clay in parts				
				1.00m	- 1.0 -  305:2 		 CI	1.00m				
				1.70m B				1.60m Silty CLAY: hard, high plasticity, dark grey brown, trace of fine grained sand	D to M	т		
				2.00m	- 2.0  912  - -		сн					-
006.2.0					2.5			2.60m EXCAVATION TPB28 TERMINATED AT 2.60 m No ground water encountered. Test pit backfilled on completion.				
200/21 4102/00/21					3.0							-
< DIawingrie >>					35							
. FLOUD LEVEE. GF					- 300							
	P	нотос	 BRAPHS	;			 	✓ N0				
	NOTES     YES       METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     In Oct., 73 W       T     Timbering						lo Res 3 Wate Date slow low	stance SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test CLASSIFIC SOL I Base Classif MOISTURI D - Dry M - Moi W - We	ATION S DESCRIP Id on Uni ication S	(MBOLS TION fied /stem	5 & C F S S S S S S S S S S S S S S S S S S	CONSISTENCY/ RELATIVE DENSITY /S - Very Soft - Soft - Firm - Firm - Hard /St - Very Stiff - Hard /L - Very Loose - Loose MD - Medium Dense /D - Dense /D - Very Dense
Mec BKISE deta & ba	e Expla ails of asis of	anator abbre f desc	y Note viation	es for IS S.				SMEC AUSTRALIA				

PRO		. : F	Roma	Flood I	<i>l</i> itigatio	n		EXCA	VATION - GEOLOG CLIENT : Ostwald	Bros Pty Ltd	ì		PIT FILE SHE	<b>NO</b> / Joe	5 : <b>TPB29</b> 3 NO : 30031182
POS		1 : 1	E: 67	7940.06	7, N: 70	62760.	726 (	55 MGA94)	SURFACE	ELEVATION : 30	0.880 (	AHD)	UNE		
EQU DAT	IPME E EX(	NT T CAVA	YPE ATED	: SUM : 11/11	/13	Excava	itor 2	UI	METHOD LOGGED I	: Excavator BY : CM				CHI	ECKED BY : JSM
EXC	AVAT			NSION	S : 1.0	0 m WI	DE								
Z	5			sΩ ∞₽	Z a		NOI			MATERIAL	ωz	, Z			
VE E denetrati	ц. Н.	SUPPOR	GROUND WA	SAMPLES FIELD TES	ELEVATIO (RL) DEPTH (r	GRAPHIC	CLASSIFICAT SYMBOL	5	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle C Secondary and Minor Compone	characteristic ents	MOISTUR	CONSISTEN RELATIVE DENSITY	요 5 101	520	STRUCTURE & Other Observations
				0.30m B 0.60m 0.70m B 1.00m 1.60m B 1.80m	0.5		сі сн сн сн	medium 0.30m becomin 0.60m shading 1.00m less free 1.50m shading nodules	n grained śand, grass and rootlets to 0.15 ng hard g light brown, some white powdery calcite r quent white nodules g dark brown grey/ grey brown, some whit < 5mm diameter, traces of fine grained s	m		VSt			- - - - - - - - - - - - - - - - - - -
				2.40m B 2.60m	2.5		сн	2.40m Sandy ( brown, 1 3.40m	CLAY: hard, medium plasticity, light grey l fine grained sand	brown with slight yellow	DtoM	т			2.40: Slow digging.
				3.30m B 3.90m 4.30m B 4.60m	- 3.5		CL	4.10m Clayey brown n parts	And the plasticity, light grey brown, pare gr high fine to coarse grained sand content SAND: very dense, light grey brown, pale nottled, fine to coarse grained sand, varia	e grey and yellow/ orang able grading sandy clay	n je in	VD			
2		-			0 96 5.0 - -		SM	5.10m Silty SA 5.30m EXCAV	ND: dense, light grey brown to white wea	akly cemented in parts		D			
					5.5			No grou Test pit	ind water encountered. backfilled on completion.						
	PH NC	IUTOG DTES	RAPHS		] YES		[	NO NO	1						
ME" N BH R SUF T	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering   WATER Used to Date water outflow Water outflow					0 Oct., 7 evel on I vater out	o Res 3 Wat Date s w low	istance er hown	SAMPLES & FIELD TESTS         U50       - Undisturbed Sample         50 mm diameter         D       - Disturbed Sample         B       - Bulk Disturbed Sample         MC       - Moisture Content         HP       - Hand Penetrometer (UC         VS       - Vane Shear, P-Peak, R-Remouded (uncorrect)         PBT       - Plate Bearing Test	CLASSIFIC SOIL Bat Class MOISTUF D - Dr M - Mc W - We ed kPa)	CATION S DESCRIP sed on Un sification S RE y pist et	YMBOLS TION fied ystem	S &	CON REL VS S F St VSt H VL L MD D VD	ISISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See I detail & bas	Explais s of a sis of	nator ibbrev desci	/ Note /iatior iption	es for is s.					SMEC AUSTRAI	_IA					SMEC

PRO.		- : I N · I	Roma	Flood I	Vitigatio	n		EXCA	VATION - GEOLOGIC CLIENT : Ostwald Bros	S Pty Ltd			PIT N FILE / J	IO: <b>TPB30</b> IOB NO: 30031182
POSI	TION	N :	E: 67	7934.06	1, N: 70	62789.3	307 (	55 MGA94)	SURFACE ELE	EVATION : 300	.965 (	AHD)	JI IEE I	
EQU			YPE	: SUM		Excava	tor 20	)T	METHOD : Ex	cavator				
EXCA				NSION	S : 1.00	0 m WI	DE		LUGGED BY :	UNI			С	HECKED BY : JSM
		DF	RILLIN	IG					MA	TERIAL				
/E E PENETRATION	. T	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	S	MATERIAL DESCRIPTION coil Type, Colour, Plasticity or Particle Charac Secondary and Minor Components	cteristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	පි 5 101520	STRUCTURE & Other Observations
				0.30m B 0.60m 0.70m B 1.00m 1.50m B 2.30m B 2.50m 3.10m B			сі сі сі сі сі	2.40m Silty CLA grained s grained s becoming bec	YY: very stiff, medium plasticity, dark brown gr         g hard         g hard         light brown, some white powdery calcite nodu         r         uent white nodules         dark brown grey/ grey brown, some white pow         dark brown grey/ grey brown, some white pow         c5mm diameter, traces of fine grained sand         iLAY: hard, medium plasticity, light grey, brown ine grained sand	rey, fine to medium	D to M	L N		ALLUVIUM
	3.10m B 3.40m 3.40m 3.5 - 4.20m B 4.50m 4.5 - 4.60m B 4.5 - 4.5 -					CL SC	3.40m mottles, 1 parts. 4.60m Clayey S brown mc parts	g low plasticity, light grey brown, pale grey an high fine to coarse grained sand content, grad SAND: very dense, light grey brown, pale grey tottled, fine to coarse grained sand, variable g	d grey brown ding clayey sand in and yellow/ orange rading sandy clay in		ц		0.6m wide bucket and single tooth ripper.	
0					- 5.0		1	5.10m Silty SAN	ND: dense, light grey brown to white weakly co	emented in parts	_			-
							SM	5.40m EXCAVA No groun Test pit b	ATION TPB30 TERMINATED AT 5.40 m nd water encountered. backfilled on completion.					
					- 6.0									
	PH NC	HOTOG DTES	RAPHS		] <sup>Ř</sup> YES		[	NO NO						
, MET N BH R SUP T	Natu Exis Bacl Bullo Ripp PORT	ural Ex ting E: khoe E dozer I ber <b>F</b> pering	posure ccavati Bucket Blade	e on		TION <sup>±</sup> → N 0 Oct., 7 evel on I vater inflo vater outf	o Resi 3 Wate Date sl w low	stance er nown	SAMPLES & FIELD TESTS         U50       -         U50       -         U50       -         D       -         Disturbed Sample         B       -         B       -         Bulk Disturbed Sample         MC       -         Moisture Content         HP       -         Hand Penetrometer (UCS kPa         VS       -         Vane Shear; P-Peak,         R-Remouded (uncorrected kP         PBT       -         Plate Bearing Test	A) CLASSIFICA SOIL D Base Classific MOISTURE D - Dry M - Mois W - Wet	tion s ESCRIP d on Uni cation S	YMBOLS TION fied ystem	& CI RI V: S F SI V: H VI L M D VI	ONSISTENCY/ ELATIVE DENSITY S - Very Soft - Soft t - Stiff St - Very Stiff - Hard L - Very Lose - Loose D - Medium Dense - Dense D - Very Dense
See E detail & bas	Explais s of a sis of	nator abbre desci	/ Note /iatior iption	es for Is s.					SMEC AUSTRALIA	\				

PROJE LOCAT	CT : ION :	Roma Roma	Flood N	<i>l</i> itigation	ı		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / J SHEET	O : TPB31 OB NO : 30031182 : 1 OF 1
POSIT	ION : MENT	E: 67 TYPF	7967.614 : SUMI	4, N: 70	62790. Excava	223 ( ator 2	55 MGA94) SURFACE ELEVATION : 299	9.536 (	AHD)		
DATE	EXCAV	ATED	: 12/11	/13			LOGGED BY : CM			С	HECKED BY : JSM
EXCA	ATION/ ח		INSIONS	S : 1.00	) m WI	DE	ΜΔΤΕΡΙΔΙ				
PENETRATION	SUPPORT	ROUND WATER	AMPLES &	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	ONSISTENCY RELATIVE DENSITY	L 00 00	STRUCTURE & Other Observations
<u> </u>	<u>т</u>	ō	아프	- 0.0	1	ซี SC	Silty SAND: loose, brown, fine to medium grained		ō _	5 10 1520	ALLUVIUM
			0.70m B 1.00m 1.70m B 2.00m 3.00m B 3.30m			сı-сн сı-сн сı-сн сı-сн	Silty CLAY: stiff to very stiff, medium to high plasticity, dark grey brown, trace of fine grained sand     Silty CLAY: stiff to very stiff, yellow brown, trace to no fine grained sand in parts     shading grey brown, stiff, yellow brown, trace to no fine grained sand in parts     shading light brown/ yellow, slight decrease in moisture content     Sandy CLAY: very stiff, medium to high plasticity, yellow brown and pale grey mottled, fine to medium grained sand, some silt fines	Δ	VSt Sto VSt L		
						sc	4.00m         Clayey SAND: dense, pale grey and light yellow/ orange, fine to coarse grained sand, varying clay fines content, decreasing with depth         4.60m         Silty SAND: dense, pale grey to light grey brown and yellow brown, fine to coarse grained sand         5.00m         EXCAVATION TPB31 TERMINATED AT 5.00 m No ground water encountered. Test pit backfilled on completion.	Dto M	0		
	PHOTO NOTES OD Natural E Existing E Backhoe Bulldozer Ripper ORT Fimbering	xposure xxcovati Bucket Blade	S P on V	5.5 6.0 7 YES PENETRA ≫ш ш I 11 VATER 11 W W	TION 5 0 Oct., 7 evel on I ater inflc ater outfl	lo Res	SAMPLES & FIELD TESTS       CLASSIFIC. SOIL I         stance       U50 - Undisturbed Sample 50 mm diameter       Base Classif         D - Disturbed Sample       Classif         B - Bulk Disturbed Sample       MOISTURE         MC - Moisture Content       D - Dry         HP - Hand Penetrometer (UCS kPa)       M - Mois         VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W - Wei         PBT - Plate Bearing Test       W - Wei	ATION S DESCRIP id on Unication S	YMBOLS TION ified ystem	5 & CC Rt VS S S S H VL M D D VI	DNSISTENCY/ ELATIVE DENSITY S - Very Soft - Firm - Stiff St - Very Sliff - Hard - Very Loose - Loose D - Medium Dense D - Dense D - Very Dense
details of & basis	planator of abbre of desc	y Note viatior ription	es for IS S.				SMEC AUSTRALIA				SMEC

PROJE( LOCATI	CT : ION :	Roma Roma	a Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT NO</b> FILE / JO SHEET	D: <b>TPB33</b> DB NO : 30031182 : 1 OF 1
POSITIO	ON :	E: 67	7895.51	0, N: 70	62940.	236 (	5 MGA94) SURFACE ELEVATION : 30	.895 (	AHD)		
DATE E	EXCAV	ATED	): 12/11	/13	LAGAVO		LOGGED BY : CM			CH	ECKED BY : JSM
EXCAV				S : 1.00	) m WI	DE	ΜΑΤΕΡΙΔΙ				
JETRATION	PPORT	IND WATER	APLES & D TESTS	NATION (RL) PTH (m)	APHIC LOG	SIFICATION	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	015TURE NDITION	SISTENCY LATIVE ENSITY	DCP	STRUCTURE & Other Observations
а В Ш н :	- S	GROL	SAN	Ë E	Б	CLAS	Secondary and minor components	Q Q	CONS	5 101520	
			0.30m			сі	Sandy CLAY: very stiff to hard, medium plasticity, dark brown, fine to medium grained sand, grass and rootlets to 0.2m		VSt to H		ALLUVIUM
			0.60m 1.00m B 1.30m	0.5		сі 	0.80m shading light brown/ yellow brown, increasing fine to medium grain sand content		т		-
				1.5 — - - - - - 00 2.0 — - - - - -		CI SC	1.50m increasing sand content with depth, grading clayey sand in parts 1.80m Silty SAND: dense, light brown yellow brown, some clay fines in parts, fin to medium grain sand	W q C			-
				2.5		sc	2.50m decreasing clay fines content		۵		
							4.30m EXCAVATION TPB33 TERMINATED AT 4.30 m No ground water encountered. Test pit backfilled on completion.				
				- - 5.5 — - - - - - - - - - - - - - - - - - - -							
METHC N N. E E: BH Bi R R SUPPO T Ti	PHOTO NOTES DD latural E existing E eackhoe fulldozer tipper DRT imbering	gRAPH xposur xcavat Bucket Blade	e ion	→ 1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (	TION 	lo Res 3 Wat Date s w low	NO         stance       SAMPLES & FIELD TESTS         U50 - Undisturbed Sample 50 mm diameter       Base Classif         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content       MOISTURE D - Dry MC - Moisture Content         MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       Moisture W - Wei         PBT - Plate Bearing Test       W - Wei	tion s ESCRIP d on Uni cation S	YMBOLS TION fied ystem	S& CO RE VS S F VS F U U U VD VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Sliff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See Exp details o & basis o	of abbre of desc	y Note viation	es tor 1S IS.				SMEC AUSTRALIA				

PROJE LOCAT	CT	: Ro : Ro	oma F oma	lood N	litigatior	ı		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			FILE / SHEET	NO: TPB34 JOB NO: 30031182 F: 1 OF 1
POSITI		: E:	6780	52.110	), N: 700	63041.	470 (	5 MGA94) SURFACE ELEVATION : 30	1.160 (	AHD)		
	IVIEN EXCA	i I'Y VAT	PE : ED :	SUMI 13/11/	10MO   /13	Excava	ator 20	ME I HOD : Excavator           LOGGED BY : CM			C	CHECKED BY : JSM
EXCAV	/ATIC	DN D	IMEN	SIONS	6 : 1.00	) m Wl	DE					
z			LLING	∞Ω Ω	z c		S	MATERIAL		5		
-VE E PENETRATIO	I	INOPPOR	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIO	GRAPHIC LOG	CLASSIFICATI	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	5 10152	STRUCTURE & Other Observations
			0. B 0.	40m 60m	301.0		Сі-Сн	Silty CLAY: very stiff, medium to high plasticity, dark grey brown trace of fine grained sand		VSt		ALLUVIUM
			0.	90m	- - 1.0 - - 0:000 - -		сі	1.30m				-
			2. B	<u>00m</u>	1.5		СІ	o own stanning	to M	т		-
	2.40m 2.5 3.00m 3.0 8 3.40m 3.5 4.0						CI	shading yellow brown, light off grey brown, without white nodules, slight increase in moisture content	Δ			
					4.0	1 1 1	SM	4.20m Silty SAND: dense to very dense, light to yellow brown, fine to medium grained sand 4.60m SAND: dense, light yellow brown, fine to medium grained sand, trace of fines 5.00m		D to VD		
								EXCAVATION TPB34 TERMINATED AT 5.00 m No ground water encountered. Test pit backfilled on completion.				
METH N N E E BH E B E R F SUPPO	PHO NOTE OD Natura Existing Backho Balldoz Ripper ORT Timber	I Expo g Exca pe Bud cer Bla	APHS osure avation cket ade	P V	6.0 → YES ENETRA Suu⊥ VATER	TION ; ₹ 0 Oct., 7 evel on I ater infic ater out	lo Resi 3 Wate Date sl ow flow	NO       SAMPLES & FIELD TESTS       CLASSIFIC. SOIL 1         stance       U50 - Undisturbed Sample 50 mm diameter       Base Classif         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content       MOISTURI D - Dry MC - Moisture Content         r own       VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W - We         PBT - Plate Bearing Test       W	ATION S DESCRIF ed on Un ication S	YMBOLS TION ified ystem	5& C R V S F S V V V U V V V V V V V	CONSISTENCY/ RELATIVE DENSITY 'S - Very Soft - Firm - Firm - Stiff 'St - Very Stiff - Hard L - Very Losse - Losse MD - Medium Dense - Dense D - Very Dense
See Exp details o & basis	plana of abb of de	tory I previa scrip	Notes ations tions.	for				SMEC AUSTRALIA				

PRO.	JEC' ATIC	T : N :	Roma Roma	I Flood N	vlitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			FILE / SHEET	<b>IO</b> : JOB NO: : :: 1 OF 1	<b>TPB36</b> 30031182
POS		N :	E: 67	8070.19	4, N: 70	63016.	144 (	55 MGA94) SURFACE ELEVATION : 301.22	20 (A	AHD)			
DAT	E EX	CAV		: 12/11	/13	Excava		LOGGED BY : CM			C	HECKED	BY : JSM
EXC	AVA			NSION	S : 1.0	0 m Wl	DE						
20	5		KILLIN H	wΩ ∞Ω	Z C		NO	MATERIAL		<u>ک</u>			
VE E DENETBATI	L H	SUPPOR	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	වි 5 10152	80	STRUCTURE ther Observations
				0.20m B 0.40m B 0.60m	0.5 - 0.5 -		СІ-СН — — СІ-СН СІ	Sitty CLAY: very stiff, medium to high plasticity, dark grey brown, traces of 0.20m fine grained sand, grass and rootlets to 0.15m becoming very stiff to hard 0.40m Sandy CLAY: hard, medium plasticity, light brown to brown, fine to medium grained sand, occasional white calcite nodules <20mm diameter, light brown/ yellow brown shading		VSt to H VSt		ALLUVI	
				1.00m B 1.30m	- 1.0 - 		CI	0.90m shading light brown/ yellow brown					- - - - - - - - - - - - - - - - - - -
				2.20m B 2.40m	2.0		сі	2.10m	D to M	т			
	3.60m B 3.90m 4.0 4.5 4.5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5						CL-CI SM	3.60m         becoming low to medium plasticity, light yellow brown/ light grey brown, fine grained sand content, higher silt content         4.10m         Silty SAND: dense to very dense, yellow brown, fine to medium grained         4.60m         SAND: dense, light yellow brown, fine to medium grained, trace of fines         4.80m		D D to VD			
					5.0			EXCAVATION TPB36 TERMINATED AT 4.80 m No ground water encountered. Test pit backfilled on completion.					
, ME BH B R SUF T	P Nati Exix Bac Bul Rip PPOR Tim	HOTOG OTES ural Ex sting E skhoe I Idozer per CT Sbering	posure ccavat Bucket Blade	S For	YES           >>=           >>=           >>=           >>=           >>=           >>=           >>=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >=           >= <tr< td=""><td>ATION TO Oct., 7 Level on vater inflo vater out</td><td>lo Res 3 Wat Date s ow flow</td><td>NO       SAMPLES &amp; FIELD TESTS       CLASSIFICATION         iistance       U50 - Undisturbed Sample       Based or         50 mm diameter       D       Disturbed Sample         B       Bulk Disturbed Sample       MOISTURE         MC       Moisture Content       D         HP       Hand Penetrometer (UCS kPa)       N         VS       Vane Shear, P-Peak, R-Remouded (uncorrected kPa)       W - Wet         PBT       Plate Bearing Test       Vet</td><td>ON SY CRIPI n Unif on Sy</td><td>TION ied stem</td><td>S&amp; C F V S S V V L L U V V</td><td>ONSISTEN ELATIVE D S t St L D</td><td>CY/ Very Soft - Soft - Firm - Stiff - Very Stiff - Very Loose - Loose - Medium Dense - Dense - Very Dense</td></tr<>	ATION TO Oct., 7 Level on vater inflo vater out	lo Res 3 Wat Date s ow flow	NO       SAMPLES & FIELD TESTS       CLASSIFICATION         iistance       U50 - Undisturbed Sample       Based or         50 mm diameter       D       Disturbed Sample         B       Bulk Disturbed Sample       MOISTURE         MC       Moisture Content       D         HP       Hand Penetrometer (UCS kPa)       N         VS       Vane Shear, P-Peak, R-Remouded (uncorrected kPa)       W - Wet         PBT       Plate Bearing Test       Vet	ON SY CRIPI n Unif on Sy	TION ied stem	S& C F V S S V V L L U V V	ONSISTEN ELATIVE D S t St L D	CY/ Very Soft - Soft - Firm - Stiff - Very Stiff - Very Loose - Loose - Medium Dense - Dense - Very Dense
detail & bas	s of sis of	abbre desc	viation	s ioi s.				SMEC AUSTRALIA					SMEC

PROJECT : Roma	a Flood Mitigatio	n	EXCAVATION - GEOLOGICAL LC	DG		PIT NC	<b>D</b> : <b>TPB37</b> B NO : 30031182
POSITION : E: 67	8049.585, N: 70	62997.314 (5	5 MGA94) SURFACE ELEVATION	: 301.164 (	AHD)	SHEET .	
EQUIPMENT TYPE DATE EXCAVATED	: SUMITOMO : 13/11/13	Excavator 20	T METHOD : Excavator LOGGED BY : CM			СН	IECKED BY : JSM
	ENSIONS : 1.0	0 m WIDE	MATEDIAI				
	NG NG NG	NO NO	WATERIAL	шZ	≿		
VE E PENETRATIG H SUPPOR GROUND WAT GROUND WAT	SAMPLES SAMPLES FIELD TES ELEVATIO ELEVATIO	GRAPHIC LOG CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	요 5 101520	STRUCTURE & Other Observations
	0.30m         0.0           0.50m         0.5           0.60m         0.5           0.60m         0.5           0.80m         0.5           0.80m         0.5           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           1.0         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00         0.00           0.00		Sitty CLAY: very stiff, medium to high plasticity, dark grey brown, tra         becoming very stiff to hard         550n         Sandy CLAY: hard, medium plasticity, light brown to brown, fine to medium grained sand, some while calcite nodules <15mm diameter grey/sh brown	aces of r, light r, light i with rained d, slight n, fine	DtoVD D H VSttoH VSt		ALLUVIUM
METHOD N Natural Exposur E Existing Excavat BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	e ion WATER	0 Oct., 73 Wate evel on Date shi vater inflow vater outflow	stance SAMPLES & FIELD TESTS CLASS 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	SIFICATION S SOIL DESCRIP Based on Uni Classification S STURE • Dry • Moist • Wet	(MBOLS TION fied /stem	& COI REL VS S F St VSt H VL L D D VD	NSISTENCY/ - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
See Explanatory Note details of abbreviation & basis of description	es for ns is.		SMEC AUSTRALIA				SMEC

PROJE LOCAT		Roma Roma	a Flood I	Vitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / J SHEET	IO: TPB38 JOB NO: 30031182
POSIT	ION	E: 67	8911.98	8, N: 70	63046.	991 (	55 MGA94) SURFACE ELEVATION : 301.	333 (	AHD)		
DATE	EXCA	VATED	): 27/11	1/13	Excava	ator 2	LOGGED BY : CM			С	HECKED BY : JSM
EXCA\	/ATIO		ENSION	S : 0.60	) m WI	DE					
z			NG ∞≌	Z G		NO	MATERIAL		5		
VE E PENETRATIO	H H	GROUND WAT	SAMPLES FIELD TES	ELEVATIO RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	원 5 101520	STRUCTURE & Other Observations
						SM	Silty SAND: loose, medium to high plasticity, brown to dark brown, fine to medium grained sand, varying clay fines, grass and rootlets to 0.25mm thick to 0.7m		L		ALLUVIUM -
			1.30m			   SM	0.70m becoming medium dense, increasing in clay fines content, variable	D to M	DW		-
			B 1.50m			CI	Sandy CLAY: very stiff, medium plasticity, grey brown and some pale grey to grey mottling in parts, varying fine to medium grained sand content 1.70m becoming brown to dark brown, slight greyish brown, fine to medium grained sand		_		-
			2.00m B 2.30m	2.0 - - - - - - - - - - - - - - - - - - -					vSt		
				3.0-		СІ		×			
	4.00m 4.0						3.80m Silty SAND: dense, grey brown, fine to medium grained sand, high silt content				
			<u>4.30m</u>	- - - - - - - - - - - - - - - - - - -		I I I I I I					
							5.00m becoming light brown and light grey brown, fine to medium grained sand, slight light grey brown, grading less fines with depth	D to M			
					1	.  . 	5.80m EXCAVATION TPB38 TERMINATED AT 5.80 m No ground water encountered.				
				6.0			Test pit backfilled on completion.				-
METH N I BH E B F R F SUPP T -	PHOT NOTE Natural Existing Backho Bulldoze Ripper ORT	DGRAPH Exposur Excava Bucke Bucke Blade	e tion	YES PENETRA ≫ ш ш ш WATER ↓ ↓ ↓	TION = → 0 Oct., 7 evel on 1 vater inflo vater out	lo Res 3 Wat Date s bw flow	NO         stance       SAMPLES & FIELD TESTS         U50       Undisturbed Sample         50 mm diameter       Based         D       Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       Plate Bearing Test	FION S SCRIP on Uni ation S	YMBOL: TION fied ystem	S& C R VS F S V V H V L M D V	ONSISTENCY/ ELATIVE DENSITY S - Very Soft - Soft Firm t - Stiff St - Very Stiff L - Very Loose - Loose D - Medium Dense D - Very Dense
See Ex details & basis	planat of abb of des	ory Not eviatio criptior	es for ns ns.				SMEC AUSTRALIA				

PRO.		. : I	Roma	Flood I	Vitigatio	n		EXCA	VATION - GEOLOGICA CLIENT : Ostwald Bros	AL LOG Pty Ltd			PIT N FILE / SHEET	<b>IO</b> : <b>1</b> IOB NO: 31	<b>PB39</b> 0031182
POSI		1 : 1	E: 678	3897.16	9, N: 70	63121.	187 (	55 MGA94)	SURFACE ELE	/ATION : 301.	489 (	AHD)	JULLI	01 1	
EQU	IPME	NT T	YPE	: SUM	ITOMO	Excava	ator 24	4T	METHOD : Exc	avator					
DATE	= ΕΧ( 4νατ			: 27/1' NSION	1/13 S:06	0 m WI	DF		LOGGED BY :	CM			С	HECKED	вл : JSW
		DF		IG					MAT	ERIAL					
EPENETRATION		SUPPORT	ROUND WATER LEVELS	SAMPLES & IELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	So	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characte Secondary and Minor Components	eristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 101520	S & Oth	TRUCTURE her Observations
<u>&gt; ш</u>			0		- 0.0		SM	Silty SAN clay fines	ND: brown to dark brown, fine to medium graine s, grass and rootlets to 0.3m	d sand, varying	Σ			ALLUVIU	M -
				1.00m B 1.50m 2.20m B 2.50m			SM CI CI SM SM	0.30m	g medium dense	ccasional pale nely mottled, fine ed sand, some sand, some clay yellow brown	DtoM	D VSt MD			
								No groun Test pit b	nd water encountered. backfilled on completion.						-
	PH	I IOTOG	RAPHS	L 3	⊥ <sub>6.0</sub>	L									
	NC	DTES			] YES		Į	X NO	1						
MET N BH B R SUP T	<b>THOD</b> Natu Exis Bacl Bullo Ripp <b>PORT</b> Timb	iral Ex ting E: thoe E dozer I ber f pering	posure ccavati Bucket Blade	e on		0 Oct., 7 evel on l vater inflo	lo Res 3 Wat Date s bw flow	istance er hown	SAMPLES & FIELD TESTS         U50       Undisturbed Sample         50 mm diameter         D       Disturbed Sample         B       Bulk Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa         PBT       Plate Bearing Test	CLASSIFICAT SOIL DE Based Classific MOISTURE D - Dry M - Moist W - Wet	CION S SCRIP on Uni ation S	YMBOLS TION fied ystem	S& C R V S F S V H V L M D V	ONSISTENC ELATIVE DE S t St L D D	Y/ NSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
See E detail & bas	Explais s of a sis of	nator ibbre desci	/ Note /iation iption:	es for Is s.	_	_	_		SMEC AUSTRALIA		_	_	_	1	SMEC

PR		T :	Roma	Flood N	litigation	ı		EXCAVATION - GEOLOGICAL LOG			FILE / JO	O: TPB40 OB NO : 30031182
PO	SITIO	''N : N :	-coma E: 678	3902.12	1, N: 70	63096.	366 (	55 MGA94) SURFACE ELEVATION : 301	.499 (.	AHD)	SHEET	: 1 OF 1
EQ				: SUM	TOMO	Excava	itor 24	T METHOD : Excavator			0	
EX				NSION	5 : 0.60	) <u>m </u> WI	DE					
		DF	RILLIN	IG			7	MATERIAL	1			
a '	e penetration H	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	පි 5 101520	STRUCTURE & Other Observations
					- 0.0		SM	Silty SAND: loose, brown to dark brown, fine to medium grained sand, varying clay fines, grass and rootlets to 0.3m	D to M			ALLUVIUM
					- 301.0 - 301.0 		SM	becoming medium dense	۵	DM		-
				1.30m			SM	increasing clay fines in parts 1.20m Clayey SAND: very dense, grey brown and pale grey to grey mottled in clayer SAND: very dense, grey brown and pale grey to grey mottled in				-
				B 1.60m	- 0:1.5 - 0:00 - - - - - - - - - - - - - - - -		СІ	parts, riigh day content, medium plasticity,				-
				2.00m B	- 2.0			grey brown mottled in parts, some clayey sand lenses		VSt		-
				2.50m			СІ					-
					3.0		SM	3.00m     Silty SAND: dense, light brown slight yellow brown, fine to medium     grained sand, some clay fines in parts	D to M			-
					- 738 - 738 - 4.0 		SM	grading less fines content		0		-
10.0/2014 12:00 0:2:0								4.70m				-
<							SM	some pale grey and yellow orange brown, sandy silt and clayey silt bands, gradual increase in fines content 5.30m				-
OOD LEVEE.GL					- 5.5 - 5.6 - 5 - 5 - 5 - 5 - 5.6 - 5			EXCAVATION TPB40 TERMINATED AT 5.30 m No ground water encountered. Test pit backfilled on completion.				-
	P	HOTOC	BRAPHS	\$ 	6.0 -		[	NO				
	ETHOD Nat Exis H Bac Bull Rip JPPOR Tim	ural Ex sting E khoe E dozer per T bering	cposure xcavati Bucket Blade	e on V		TION	lo Resi 3 Wate Date sl w low	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) vown VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	TION S ESCRIP I on Uni cation S	YMBOLS TION fied ystem	&     CC       RE     VS       S     F       St     VS       H     VL       MM     D       VE     VE	DNSISTENCY/ ELATIVE DENSITY 6 - Very Soft - Firm - Stiff St - Very Stiff - Hard - Very Loose - Loose D - Medium Dense - Dense 0 - Very Dense
See deta & ba	e Expla ails of a asis of	inator abbre desc	y Note viation	es for Is s.				SMEC AUSTRALIA				

PROJ LOCA	ect Tion	: R : R	oma oma	Flood N	<i>l</i> itigatio	n		EXCA	AVATION - GEOLOGICA CLIENT : Ostwald Bros FEATURE :	AL LOG Pty Ltd			<b>PIT N</b> FILE / J SHEET	IO: TPB41 JOB NO: 30031182 T: 1 OF 1
POSI	ΓION	: E	: 678	906.98	6, N: 70	63072.	247 (	55 MGA94)	SURFACE ELEV	/ATION : 301.3	397 (	AHD)		
EQUIF DATE				: SUM : 27/11	ITOMO /13	Excava	ator 24	4T	METHOD : Exc LOGGED BY : (	avator CM			С	CHECKED BY : JSM
EACA	VATIC			G	3.0.00		DE		MAT	ERIAL				
/E PENETRATION		SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL		MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characte Secondary and Minor Components	eristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	පි 5 101520	STRUCTURE & Other Observations
			_		0.0		SM SM	Silty S/ varying 0.50m becomi	AND: loose, brown to dark brown, fine to medium g clay fines, grass and rootlets to 0.2m , ing medium dense	grained sand,	D to M	MD		ALLUVIUM
			-	1.30m B 1.50m	     		SM	1.20m Clayey varying 1.80m	/ SAND: dense, light brown grey, fine to medium g g clay content, grading sandy clay in parts	grained sand,	0	۵		
			- - -	2.00m B 2.30m	2.0		СІ	Sandy grey br	CLAY: very stiff, medium plasticity, light brown to rown, fine to medium grained sand, some clayey s	brown slightly sand lenses		VSt		
					0		SM SM	3.30m Silty S/ 3.60m increas 4.20m shading	AND: dense, yellow brown, fine to medium graine	d sand	D to M			
					0, 4.5		SM SM	5.00m	n parts, increase in moisture content	ht grey brown	_			
	РНО		Арне					EXCAV No groi Test pit	VATION TPB41 TERMINATED AT 5.50 m und water encountered. it backfilled on completion.					-
, METI N E BH R R Supp T	NOTI HOD Natura Existing Backho Bulldoz Ripper PORT Timber	I Exp g Exc pe Bu zer Bl	osure avatic icket ade	on N		0 Oct., 7 evel on l vater inflo	lo Res 3 Wat Date s ow flow	NO istance er hown	SAMPLES & FIELD TESTS         U50       -         D       -         D       -         D       -         D       -         D       -         D       -         D       -         D       -         D       -         D       -         D       -         U50       -         Noisture Content         HP       -         Hand Penetrometer (UCS kPa)         VS       -         Vane Shear; P-Peak,         R-Remouded (uncorrected kPa)         PBT       -         Plate Bearing Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION S SCRIP on Uni ation S	YMBOLS TION fied ystem	5 & C R V S S F F V V U L L M D D V V	CONSISTENCY/ LELATIVE DENSITY S - Very Soft - Firm t - Stiff St - Very Stiff - Hard L - Very Loose - Loose ID - Medium Dense - Dense D - Very Dense
& basi	s of de	scrip	otions	5 5.					SMEC AUSTRALIA					SMEC

PRO	DJEC	T : I DN : I	Roma	Flood	<i>l</i> itigatio	n		EXCA	ATION - G	EOLOGICA : Ostwald Bros F	LLOG Pty Ltd			<b>PIT</b> FILE SHEE	<b>NO</b> / Job =T : 1	: <b>TPB42</b> NO : 30031182
PO		N :	E: 678	8092.40	4, N: 70	62356.	942 (	55 MGA94)		SURFACE ELEV	ATION : 303.0	) 080	AHD)	UNE		
DAT	JIPM TE EX	ENT I KCAV	YPE ATED	: KUB : 18/12	2/13	cavator	41			LOGGED BY : C	avator CM				CHE	CKED BY : JSM
EXC	CAVA			NSION	S : 0.4	5 m WI	DE			MATE	FRIAI					
	NOI-	5.	TER	STS STS	NO Ê	υ	NOI					щZ	≻ Zuzc			
J I	F PENETRA	SUPPOI	GROUND W	SAMPLE FIELD TE	ELEVATI (RL)	GRAPH LOG	CLASSIFICA SYMBO	Sc	MATERIAL D vil Type, Colour, Plastici Secondary and N	ISCRIPTION y or Particle Character linor Components	ristic	MOISTUR	CONSISTE RELATIV DENSIT	년 5 1015	20	STRUCTURE & Other Observations
					0.0-		SM	Silty SAN and rootle	D: loose to medium dens ets to 0.15m depth	se, fine to medium grair	ned, brown, grass	٥	to MD		ŀ	ALLUVIUM .
					- 3			0.20m trace of cl								-
					-		SM						0			
					0.5 —			0.50mshading li		 easing in clav fines con		-	MD to			
				0.70m	302.5	l L	CI	0.70m								
				В				Sandy CL and some	AY: very stiff to hard, me yellow brown, fine to me	edium plasticity, grey, s edium grained sand	light brown grey	1				-
					-		CI					W				
				1.00m	- 1.0 -			1.00m shading y content	ellow brown, brown, son	ne brown grey/grey, inci	rease in moisture	ā				-
				1.20m	302.1			content					tto H			
				в	-		CI						_ S>			
				1.50m	-											
					1.5											
				4.00	- 30			1.70m Silty SAN	D: dense, fine to mediun	n grained yellow brown,	, varying in fines					
				B			SC	content, g	rading silty sand in lense	es some sandy clay len	ses/bands.					
					2.0 —											-
				2.10m	301.0			2.10m Sandy CL	AY: very stiff to hard, m	edium to high plasticity,	ligth yellow	-				-
							SC	brown, fin	e to medium grained sai	nd						
					-							Σ				
				2.50m B	2.5			2.50m high silt c	ontent, grading silty clay	/ clayey silt in parts		-	tt to H			-
					300.5								SV			
				2.80m			SC									
					-			3.00m								
		1			- <del>3.0 -</del>			EXCAVA No ground	FION TPB42 TERMINAT d water encountered.	ED AT 3.00 m						
					300	-		Test pit ba	ackfilled on completion.							-
0					-											
					3.5											-
					99.5	-										
						-										
	F	рнотос	RAPHS	<u>ا</u>			<u> </u>									
<b> </b>	1	NOTES						NU NU			CLASSIFICAT	ION S	MBOI	s&	CON	SISTENCY/
M	ETHOI	D tural Fv	DOSU		PENETRA >⊔uu⊐	TION ≞∃ ■^		istance	SAMPLES & FIELD	TESTS ed Sample	SOIL DE Based	SCRIP on Uni	TION fied		RELA VS	TIVE DENSITY - Very Soft
E BH	Exi I Ba	isting E ckhoe E	kcavati Bucket	ion					50 mm dia D - Disturbed	meter Sample	Classifica	ation S	ystem		S F St	- Soπ - Firm - Stiff
B R	Bu Rip	lldozer   per	Blade	\ \	VATER				B - Bulk Distu MC - Moisture C	bed Sample Content	D - Dry				VSt H VI	- Very Stiff - Hard - Very Loose
SL	IPPOF	RT				0 Oct., 7 evel on	3 Wat Date s	er hown	HP - Hand Pen VS - Vane Shea	etrometer (UCS kPa) ar; P-Peak, ded (uncorrected kPa)	M - Moist W - Wet				L MD	- Loose - Loose - Medium Dense
Т	Tin	nbering				/ater inflo /ater out	ow low		PBT - Plate Bear	ing Test					D VD	- Dense - Very Dense
See deta & ba	Explais of asis o	anator abbre f desci	/ Note /iatior iption	es for ns s.					SMEC AL	ISTRALIA						

PRO		CT : ON :	Roma	a Flood I	Vitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT NC</b> FILE / JC SHEET	<b>D</b> : <b>TPB43</b> DB NO : 30031182 : 1 OF 1
PO: EQ DA	SITIC UIPN TE E	ON : //ENT XCA\	E: 67 TYPE ATED	8131.57 : KUB ): 18/12	'3, N: 70 OTA Exe 2/13	62359. cavator	779 ( 4T	55 MGA94) SURFACE ELEVATION : 302.9 METHOD : Excavator LOGGED BY : CM	99 (/	AHD)	CH	IECKED BY : JSM
EX	CAVA				S : 0.50	0 m WI I	DE	MATERIAI				
	ETRATION	PPORT	ND WATER	IPLES &	VATION (RL) PTH (m)	APHIC -0G	SIFICATION MBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic	ISTURE	SISTENCY LATIVE ENSITY	DCP	STRUCTURE & Other Observations
۳.	и Ш ц ц ц ц ц	Sn 1	GROU	SAN	E E	В _	CLASS	Secondary and Minor Components	Q D	CONS	5 101520	
					- 0.0		SM	Silty SAND: loose to medium dense, fine to medium grained brown, grass and rootlets to 0.15m	۵	L to MD		ALLUVIUM -
					- 5.0 2	l .	SM	0.60m		D to D		
				0.70m		1	SM	increasing clay fines content 0.70m		Σ		-
				B 1.00m	- - 0.1.0 - 305 - -		СІ	Sandy CLAY: very stiff to hard, medium plasticity, ligth yellow brown and pale grey, fine to medium grained sand	D to M			-
				1.50m B 1.80m	- 1.5 - 1.5 - 100 - - - - - - - - - - - - - - - - - - -		сі-сн	Silty CLAY: very stiff to hard, medium to high plasticity, grey slight brown grey, some light brown yellow brown, trace to some fine grained sand content		VSt to H		-
	2.50m B 2.80m						сі-сн	2.20m shading less ligth brown/yellow brown 3.00m	- <u>&gt;</u>			
					το 5.3.5 - - - - - - - - - - - - - -			EXCAVATION TPB43 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				
: <b> </b>			GRAPH	s _			<u>،</u> ا					1
MI E B B B R SL	ETHO Na E> H Ba Bu Ri JPPO Ti	atural E kisting I ackhoe ulldozel pper <b>RT</b>	xposur Excavat Blade	e ion		0 Oct., 7 evel on l vater inflo	lo Res 3 Wat Date s w low	istance SAMPLES & FIELD TESTS CLASSIFICATI 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	ON S SCRIP on Uni tion S	/MBOLS TION fied /stem	& CO RE VS S F St VS H VL L D VD VD	NSISTENCY/ LATIVE DENSITY - Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Dense - Dense - Very Dense
See deta & ba	e Exp ails of asis o	lanato f abbre of dese	ry Note viation ription	es for ns ns.				SMEC AUSTRALIA				

PROJECT : Roma Flood Mi LOCATION : Roma	tigation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :		PIT NO :         TPB44           FILE / JOB NO :         30031182           SHEET :         1 OF 1
POSITION : E: 678170.320	, N: 7062362.000 (5	5 MGA94) SURFACE ELEVATION : 302.8	24 (AHD)	
EQUIPMENT TYPE : KUBO	TA Excavator 4T			
EXCAVATION DIMENSIONS	: 0.50 m WIDE			CHECKED DT . JOW
DRILLING		MATERIAL		
E PENETRATION SUPPORT SHOUND WATER LEVELS & SAMPLES &	ELEVATION (RL) DEPTH (m) GRAPHIC LOG SLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
	- 0.0 - SM	Clayey SAND: loose to medium dense, fine to medium grained, brown, ligth brown and ligth grey brown, some clayey sand and sandy clay lenses pockets, grass and rootlets to 0.15m	L to MD	ALLUVIUM
0.30m B 0.50m	2500 0.5	becoming ligth grey brown and yellow brown, fine to medium grained sand.		
1.20m B 1.50m	1.5 - CL-Cl 0 2.0	1.20m         Sandy CLAY: very stiff to hard, medium plasticity, yellow brown, varying fine grained sand content         2.00m         shading less ligth grey brown, more yellow brown	D to M VStto H	
2.40m B 2.70m	5000 2.5 0000 	2.40m Silty CLAY: very stiff to hard, medium plasticity, yellow brown, varying fine grained sand content.	W	
	289.0 	No ground water encountered. Test pit backfilled on completion.		
PHOTOGRAPHS NOTES	N YES NETRATION → u L T → NO Reside ATER 10 Oct., 73 Wate Level on Date sh water inflow water outflow	NO       SAMPLES & FIELD TESTS       CLASSIFICATI SOIL DES 50 mm diameter         0       - Undisturbed Sample 50 mm diameter       Based Classifica         0       - Disturbed Sample B       Bulk Disturbed Sample         MOSTURE       MOISTURE         MO       - Moisture Content         HP       - Hand Penetrometer (UCS kPa) VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       - Plate Bearing Test	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		

PROJECT : Roma Flood Mitig LOCATION : Roma	EXCA	VATION - GEOLOGICAL L CLIENT : Ostwald Bros Pty Ltc FEATURE :	LOG	PIT NO : TPB45 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679283.399, N	7062806.446 (55 MGA94)		N : 299.838 (AHD)	
DATE EXCAVATED : 13/1/14	Excavator 41	LOGGED BY : CM	r	CHECKED BY : CM
EXCAVATION DIMENSIONS :	0.50 m WIDE			
		MATERIAL		
VE F F B BUPPORT GROUND WATE LEVELS FIELD TES1 ELEVATION	DEPTH (m.c.) GRAPHIC LOG CLASSIFICATIC SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENC RELATIVE DENSITY	5 101520
y w w x x     0     y w w x     0     y w w x     w w x       0.50m     0.50m     0     0     0       1.00m     0     0     0     0       1.00m     0     0	Sandy i varying Somm of CI-CH .5 .5 .5 .5 .5 .5 .5 .5 .5 .5 .5 .5 .5	CLAY: stiff to very stiff, medium to high plasticity, dark br fine grained sand content, grass, rootlets and content in pa lepth, grading silty clay with some fine sand content in pa ng firm to stiff, moist (base of dam) ng medium plasticity, light brown to brown, some dark gre ,	rown grey, ontent to parts	5 101520 ALLUVIUM
3.20m	CI 3.60m Sitty SA Sitty	IND: medium dense to dense, fine to medium grained, lig yellow brown 	ght brown,	
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper WAT	In Oct., 73 Water Level on Date shown water inflow water outflow	SAMPLES & FIELD TESTS       CL         U50       Undisturbed Sample         50 mm diameter       D         D       Disturbed Sample         B       Bulk Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak,         R-Remouded (uncorrected kPa)       W         PBT       Plate Bearing Test	ASSIFICATION SYMBOLS SOIL DESCRIPTION Based on Unified Classification System IOISTURE - Dry - Dry - Moist / - Wet	Image: Second State Sta
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		SMEC

PI L(	ROJE	CT : ION :	Roma Roma	a Flood N a	litigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / JO SHEET	<b>D</b> : <b>TPB46</b> DB NO : 30031182 : 1 OF 1
P			E: 67	9196.89	9, N: 70	62801.	784 ( • 4т	55 MGA94) SURFACE ELEVATION : 299.9	971 (/	AHD)		
D	ATE	EXCA	ATEC	): 13/1/	14	Javatol		LOGGED BY : CM			Cł	HECKED BY : CM
E	XCAV	ATIOI T	n dime Drili II	ENSION: NG	S : 0.50	) m WI	DE	MATERIAI				
	PENETRATION	SUPPORT	OUND WATER	AMPLES & ELD TESTS	LEVATION (RL) DEPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	DNSISTENCY RELATIVE DENSITY	DCP	STRUCTURE & Other Observations
4		<u> </u>	9	ΩĒ	— 0.0 —		5	Sandy CLAY: very stiff to hard, medium to high plasticity, dark grey,	- 0	ö	5 101520	ALLUVIUM
				0.30m B 0.70m 0.80m B 1.20m 1.40m B 1.80m			сі-сн	2.70m       fine to medium grained sand, shading grey brown, without rootlets       1.30m     Clayey SAND: fine to medium grained, light brown, slight yellow brown, medium plasticity clay fines	DtoM	VStorH		
File>> 12/03/2014 12:59 8.2.900							SM	2.50m Silty SAND: dense, fine to medium grained, light brown, slight yellow brown, with varying clay fines content  3.50m decreasing clay fines content  4.00m EXCAVATION TPB46 TERMINATED AT 4.00 m No ground water encountered.	Σ	0		
3ANE LIBRARY - COLOUR.GLB Log TEST PIT ROMA FLOOD LEVEE.GPJ < <drawingfit.< td=""><td>METHONN N E E E BH B B B R R SUPPC T T</td><td>PHOTO NOTES DD latural I ixisting ackhoc ipper DR T imberir</td><td>Exposur Excavat Bucket r Blade</td><td>e ion</td><td>Server 1 Server 2 Server 2 Serve</td><td>TION 5 0 Oct., 7 evel on 1 ater inflo</td><td>[ ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ]</td><td>Its ground water encountered.         Test pit backfilled on completion.         Its ground water encountered.         Its ground water encounter</td><td>ION S) SCRIP on Unit</td><td>/MBOLS TION Tied stem</td><td>s &amp; CCC S &amp; VS S S S S S S S S S S S S S S S S S S</td><td>DNSISTENCY/ ELATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Dense - Very Dense</td></drawingfit.<>	METHONN N E E E BH B B B R R SUPPC T T	PHOTO NOTES DD latural I ixisting ackhoc ipper DR T imberir	Exposur Excavat Bucket r Blade	e ion	Server 1 Server 2 Server 2 Serve	TION 5 0 Oct., 7 evel on 1 ater inflo	[ ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ] ]	Its ground water encountered.         Test pit backfilled on completion.         Its ground water encountered.         Its ground water encounter	ION S) SCRIP on Unit	/MBOLS TION Tied stem	s & CCC S & VS S S S S S S S S S S S S S S S S S S	DNSISTENCY/ ELATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Dense - Very Dense
SMEC BRISE	ee Exp etails c basis	olanato of abbr of des	ory Note eviation criptior	es for ns ns.				SMEC AUSTRALIA				

PROJECT : Roma Flood Mitiga LOCATION : Roma	EXCA	VATION - GEOLOGICAL CLIENT : Ostwald Bros Pty I FEATURE :	Ltd	PIT NO : TPB47 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679171.985, N: EQUIPMENT TYPE : KUBOTA DATE EXCAVATED : 13/1/14	7062926.630 (55 MGA94) Excavator 4T	SURFACE ELEVATI METHOD : Excavat LOGGED BY : CM	ION : 300.372 (AHD) tor	CHECKED BY : CM
DRILLING		MATERI	IAL	
re penetration support support decum water Levels Field Tests ELEVATION	DEPTH (m) GRAPHIC LOG LOG SYMBOL SYMBOL	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONDITION RELATIVE DENSITY	STRUCTURE     & Other Observations     5 101520
	Silty CLA Silty CLA Slightly d Sightly d Silty CLA Slightly d Silty CLA Slightly d Silty CLA Slightly d Slightly d Slightly d Slightly d Slightly d Slightly d Shading 10mm di 1.80m Clavey S medium calcite n Clavey S medium calcite n Claves SC 2.80m Silty CLA Shading 10mm di calcite n Claves SC 2.80m Silty CLA Shading 10mm di calcite n SC 2.80m Silty SAN brown/ye ST SC SC SC SC SC SC SC SC SC SC	VY: very stift to hard, medium to high plasticity, grey t ark, trace of fine grained sand, grass and rootlets to light brown to brown, occasional white powdery calci ameter, varying sand content AND: dense, light brown to brown, slightly grey brow grained sand, medium plasticity fines, occasional wh odules.	Drown, p 0.2m □ H 9 55 ite nodules < □ wn, fine to nite powdery □ 1, light □ 1, lig	
PHOTOGRAPHS NOTES     Y       METHOD     PENE       N     Natural Exposure       BH     Backhoe Bucket       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       SUPPORT     T       T     Timbering	0.0	SAMPLES & FIELD TESTS       0         U50       - Undisturbed Sample         50 mm diameter       0         D       - Disturbed Sample         B       - Bulk Disturbed Sample         MC       - Moisture Content         HP       - Hand Penetrometer (UCS kPa)         VS       - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       - Plate Bearing Test	CLASSIFICATION SYMBOLS SOIL DESCRIPTION Based on Unified Classification System MOISTURE D - Dry M - Moist W - Wet	CONSISTENCY/ RELATIVE DENSITY       VS     - Very Soft       S     - Soft       F     - Firm       St     - Stiff       VSt     - Very Stiff       H     - Hard       VL     - Very Loose       L     - Loose       MD     - Medium Dense       D     - Dense       VD     - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		SMEC

EXCAVATION - GE PROJECT : Roma Flood Mitigation LOCATION : Roma CLIENT FEATURE	OLOGICAL LOG Ostwald Bros Pty Ltd	PIT NO : TPB48 FILE / JOB NO : 30031182 SHEET : 1 OF 1			
POSITION         : E: 679221.757, N: 7062935.324 (55 MGA94)         S           EQUIPMENT TYPE         : KUBOTA Excavator 4T         M	SURFACE ELEVATION : 300.559 (AHD) METHOD : Excavator				
DATE EXCAVATED : 13/1/14	OGGED BY : CM	CHECKED BY : CM			
DRILLING	MATERIAL				
MATERIAL DE MATERIAL DE South of the function of the functio	SCRIPTION or Particle Characteristic or Components	STRUCTURE & Other Observations 5 101520			
Sandy CLAY: very stiff to stiff, media trace of fine grained sand, grass and CI-CH	um to high plasticity, dark grey brown, d rootlets to 0.2m.	ALLUVIUM			
0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.50m 0.	hading lighter Ω				
1.30m B 1.5 Cl	plasticity, dark brown slightly grey				
1.90m B 2.0 2.30m 2.30m 2.30m	prown to brown occasional white				
Sitty SAND: dense fine to medium g brown, varying clay fines content 3.0 - 1 - 1 3.0 - 1 - 1 3.20m	rained, light brown slight yellow				
Gecreasing day times content, shadi	D AT 3.70 m				
4.0 - Solution of the second s					
PHOTOGRAPHS YES NO					
METHOD       PENETRATION       SAMPLES & FIELD T         N       Natural Exposure       Uso       -         E       Existing Excavation       Uso       -       Undisturbed         B       Buldozer Blade       No Resistance       D       -       Disturbed Sa         R       Ripper       VATER       MATER       MC       -       Moisture Control         SUPPORT       T       Timbering       10 Oct., 73 Water       Vane Shear, R-Remoude       Vane Shear, R-Remoude         PBT       Plate Bearing       Vater       PBT       Plate Bearing	SAMPLES & FIELD TESTS       CLASSIFICATION SYMBOLS         U50 -       Undisturbed Sample         50 mm diameter       Based on Unified         D -       Disturbed Sample         B -       Bulk Disturbed Sample         MC -       Moisture Content         HP -       Hand Penetrometer (ICS kPa)         VS -       Vane Shear; P-Peak,         R-Remouded (uncorrected kPa)       W -         PBT -       Plate Bearing Test				
See Explanatory Notes for details of abbreviations & basis of descriptions.	STRALIA	SMEC			

PROJ LOCA	IECT	. : I N : I	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCA	VATION - GE CLIENT : FEATURE :	OLOGICA Ostwald Bros F	LLOG Pty Ltd			<b>PIT</b> FILE / SHEE	<b>NO</b> : JOB N T:1	<b>TPB49</b> O : 30031182 OF 1
POSI EQUI DATE	TION PME E EX(		E: 679 TYPE ATED	9307.623 : KUB( : 13/1/	3, N: 70 DTA Exe 14	62925.0 cavator	097 ( <del>(</del> 4T	55 MGA94)	S M L	URFACE ELEV IETHOD : Exca OGGED BY : C	ATION : 300.7 avator CM	782 (A	AHD)		CHEC	KED BY : CM
EACP	AVA I	DF	RILLIN	IG	5.0.5		DE			MATE	ERIAL					
ENETRATION		SUPPORT	ROUND WATER LEVELS	SAMPLES & IELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	Sc	MATERIAL DES bil Type, Colour, Plasticity o Secondary and Mino	SCRIPTION or Particle Character or Components	ristic	MOISTURE	ONSISTENCY RELATIVE DENSITY	5 1015	20	STRUCTURE & Other Observations
> ш і				0.30m	- 0.0 -		0	Sandy CL brown, tra	AY: very stiff to hard, media ace of fine grained sand, gra	um to high pasticity, ass and rootlets to 0.	dark, grey .2m	0	но		AL	LUVIUM
				B 0.60m 0.70m B 1.00m	0.5 0.5    - 1.0 		SC	0.60m Clayey S/ clay fines 1.20m Silty SAN	AND: dense, brown, fine to	medium grained, me		_	VSt			
					1.5		SM	1.80m	in sand content slightly coar	dense, slignt grey br		D to M				- - - - - - - - - - - - - - 
				2.40m B 2.60m	- - - - - - - - - - - - - - - - - - -		SM SM	2.70m fine to me content.	edium grained, light brown/y	rellow brown, varying			Ω			- - - - - - - - - - - - - - 
2/03/2014 12:59 8.2.900		-					SM	3.40m decrease 3.90m EXCAVA	in clay fines content, shadi	ng lighter						
LOOD LEVEE.GPJ < <drawingrile>&gt;</drawingrile>					4.0 — - - - - - - - - - - - - - - - - - - -			No groun Test pit b	d water encountered. ackfilled on completion.							-
	PH NC	HOTOG	BRAPH:	 `	5.0			NO NO								
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering METHOD PENETRATION Suit II Suit II No Resistance WATER 10 Oct., 73 Water Level on Date shown water inflow water outflow						TION 	o Resi 3 Wate Date sh w low	stance er nown	SAMPLES & FIELD TE U50 - Undisturbed S 50 mm diame D - Disturbed Sa B - Bulk Disturbe MC - Moisture Con HP - Hand Penetro VS - Vane Shear, R-Remouded PBT - Plate Bearing	ESTS Sample eter mple ed Sample itent ometer (UCS kPa) P-Peak, d (uncorrected kPa) g Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unif ation Sy	MBOLS FION ied stem	&	CONSIS RELATI VS S F St VSt H VL L MD D VD	STENCY/ IVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See E details & bas	xpla s of a is of	nator Ibbre desci	y Note viatior ription	es for is s.					SMEC AUS	STRALIA						

PROJECT : Roma	a Flood Mitigation	n	EXCAVATION - GEOLOGICAL LOG		<b>PIT NO</b> : FILE / JOB NO	<b>TPB50</b> D : 30031182 DF 1
POSITION : E: 67	9309.279, N: 70	62844.696 (5	5 MGA94) SURFACE ELEVATION : 300.	342 (AHD)	5EET . T (	'
EQUIPMENT TYPE	: KUBOTA Exe	cavator 4T	METHOD : Excavator			
	0: 14/1/14 =NSIONS · 0.50	) m WIDF	LOGGED BY : CM		CHECI	KED RA : CW
DRILLI	NG		MATERIAL			
PENETRATION SUPPORT ROUND WATER LEVELS	SAMPLES & ELD TESTS ELEVATION (RL) DEPTH (m)	GRAPHIC LOG ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION ONSISTENCY RELATIVE DENSITY	L 20 5 101520	STRUCTURE & Other Observations
<u>Ушкт</u> о		CI	Silty CLAY: very stiff to hard, medium to high plasticity, dark grey brown,	0	5 10 1520	LUVIUM
	0.70m         -           0.70m         -           0.70m         -           1.00m         1.0           1.20m         -           1.20m         -           1.60m         -           2.00m         -           2.80m         -           0.0000         -           2.80m         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.0000         -           0.00000         -		trace of fine grained sand, grass and rootlets to 0.1m <sup>2</sup> Sandy CLAY: very stiff to hard, medium to high plasticity, brown grey slightly dark, fine grained sand 1.00m shading dark, grey brown 1.80m fine grained sand, light brown to brown, occasional white powdery calcite nodules <5mm diameter fine grained sand content, light brown slight yellow brown, occasional white powdery calcite nodules, < 8mm diameter, increase sand content grading clayey sand in parts. 280m Silty SAND: dense, fine to medium grained, yellow brown EXCAVATION TPB50 TERMINATED AT 4.00 m No ground water encountered. Test pit backfilled on completion.	D to M		
	5.0					
PHOTOGRAPH NOTES	S YES		N0			
METHOD N Natural Exposur E Existing Excaval BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	e tion WATER	TION TION S No Resis 0 Oct., 73 Wate evel on Date sh rater inflow rater outflow	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	FION SYMBOLS ESCRIPTION I on Unified ation System	& CONSIS RELATI VS S F St VSt H VL L MD D VD	TENCY/ VE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See Explanatory Not details of abbreviation & basis of description	es for ns is.		SMEC AUSTRALIA			

PRO	JEC	T : ON :	Roma Roma	Flood	Mitigatio	n		EXCA	VATION - C	EOLOGICA : Ostwald Bros F	LLOG Pty Ltd			<b>PIT  </b> FILE / SHEE	NO: JOB NC T:1 C	<b>TPB51</b> 0 : 30031182 0F 1
POS	ITIC	)N :	E: 679	9313.11	9, N: 70	63062.	371 (	55 MGA94)		SURFACE ELEV	ATION : 300.9	921 (/	AHD)			
EQU				: KUB	OTA Ex	cavator	4T			METHOD : Exca	avator					
EXC				NSION	S : 0.5	0 m WI	DE			LOGGED BT . C	7111					
		D	RILLIN	IG	1					MATE	ERIAL				1	
/E		SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	Sc	MATERIAL bil Type, Colour, Plastic Secondary and	DESCRIPTION city or Particle Character Minor Components	ristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	6 5 10152	20	STRUCTURE & Other Observations
Í	Ī		-		- 0.0		SM	Silty SAN arev. with	D: loose to medium der clav fines, grass and r	nse, fine to medium grair	ned, dark brown	0	Щ. Мр		ALL	UVIUM -
					- - - 0.0 - - - -		SM SM	0.20m becoming 0.40m shading d sand cont	ark brown, slight grey t	grading sandy silt			MD			
				<u>1.30m</u> B	- 0.0 - 0.0 - 0.0 - - - - - - - - - - - - - - - - - -		sc	1.00m Clayey S/ clay fines 1.30m shading d	AND: dense, dark brow 	n grey, fine grained, me	dium plasticity — — — — — –	_				- - - - - -
				<u>1.60m</u>			sc					D to M	۵			- - - - - - - - - - - - - - - - - - -
				2.60m B 3.00m	9: 2.5 		СІ	2.60m Sandy CL grained s: 2.90m grading cl 3.10m	AY: very stiff to hard, r and and and layey sand in parts	nedium plasticity, brown,	fine to medium	_	VSt to H			
					- - - - - - - - - - - - - - - - - - -		SM	Silty SAN fines.	D: dense, fine to mediu	m grained, light brown to	b brown, with clay	Σ	۵			-
					0.267 67 9.967 9.967 9.967 9.97 9.97 9.97 9			3.90m EXCAVA No groun Test pit b	TION TPB51 TERMINA d water encountered. ackfilled on completion	TED AT 3.90 m						
					0.96 2.0											
ME BH B R	THO Na Ex Ba Bu Riț PPOI Tir	PHOTOO NOTES D Atural Existing E ckhoe I Ildozer oper RT nbering	sposure xcavati 3ucket Blade	e e e e e e e e e e e e e e e e e e e	YES → YES PENETRA → WATER ↓ ↓ ↓ ↓ WATER	U Oct., 7 0 Oct., 7 evel on l vater inflo	o Resi 3 Wate Date show	NO stance er nown	SAMPLES & FIEL U50 - Undisturt 50 mm d D - Disturbed B - Bulk Dist MC - Moisture HP - Hand Pe VS - Vane Sh R-Remou PBT - Plate Bea	D TESTS bed Sample ameter I Sample urbed Sample Content netrometer (UCS kPa) aar; P-Peak, uded (uncorrected kPa) aring Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ON S SCRIP on Uni ation Sy	/MBOLS TION fied /stem	\$ & ()	CONSIS RELATIV /S 	TENCY/ /E DENSITY - Very Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Dense - Very Dense
See detai & ba	Expl Is of sis c	anator abbre f desc	y Note viation	es for Is s.					SMEC A	JSTRALIA						

PRC	JEC	T:I	Roma	Flood I	<i>l</i> itigatior	ו		EXCA	VATION - GEOLOGICAL CLIENT : Ostwald Bros Pty L	LOG Ltd			FILE / J	IO : TPB52 JOB NO : 30031182	
POS	SITIO	N :	E: 679	268.26	7, N: 70	63070.	392 (	55 MGA94)	SURFACE ELEVATION	ON : 300.9	91 (A	HD)	SHEET		
EQL DAT	JIPM E E>	ENT 1 KCAVA		: KUB	OTA Exc 14	cavator	4T		METHOD : Excavato	or			С	HECKED BY : CM	
EXC	AVA	TION	DIME	NSION	S : 0.50	) m WI	DE						-		
	7	DF		اG س	1_		z		MATERIA	AL		~			
E KE	<ul> <li>PENETRATION</li> <li>H</li> </ul>	SUPPORT	GROUND WATEI LEVELS	SAMPLES & FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIO SYMBOL	s	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	:	MOISTURE	CONSISTENC RELATIVE DENSITY	<u>ලි</u> 5 101520	STRUCTURE & Other Observation:	s
					- 0.0		SM	Silty SAN grey, with	ND: loose to medium dense, fine to medium grained, o th clay fines, grass and rootlets to 0.55	dark brown	٥	to MD		ALLUVIUM	-
							SM	0.25m becomin 0.60m	ng medium dense, occassional rootlets to 0.8m			MD			-
				0.90m B	- -  0 1.0	    /////	SM	0.90m Sandy C grained s	brown grey to dark brown grey CLAY: very stiff to hard, medium plasticity, dark grey br	prown, fine					-
				1.20m			сі 	1.20m increase <10mm o	in moisture content, occasional white powdery calcite diameter	e modules		VSt to H			-
				<u>2.10m</u> B 2.40m	0 2.0 — 0 667 — - - - - - - - - - - - - -		sc	2.00m Clayey S content g 2.50m	SAND: dense, fine to medium grained, brown, varying grading sandy clay/ claying sand in parts.	l sand	D to M				-
					- 588.0		SM	Silty SAN	ND: dense, fine to medium grained light brown, with cl	lay fines.		٥			
					- 3.5 - 362 		SM	3.90m EXCAVA	ATION TPB52 TERMINATED AT 3.90 m						
					- 4.0 			No grour Test pit t	nd water encountered. backfilled on completion.						- - - - -
					- 5.0										
ME N E BH R SU	FTHOE Nat Exi Bad Bul Rip Tim	D tural Ex isting E: ckhoe E lldozer I oper RT	posure ccavati Bucket Blade	on N	→ 0 5.0	TION TION D Oct., 7 evel on I ater inflor ater out	o Res 3 Wat Date s w low	INO NO	SAMPLES & FIELD TESTS       C         U50       Undisturbed Sample         50 mm diameter         D       Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       Plate Bearing Test	CLASSIFICATI SOIL DES Based o Classificat MOISTURE D - Dry M - Moist W - Wet	ON SY SCRIP on Unif	MBOLS ( FION ied stem	S V V V V V V V V V V V V V	CONSISTENCY/ IELATIVE DENSITY S - Very Soft - Soft - Firm t - Stiff St - Very Stiff I - Hard L - Very Loose - Loose ID - Medium Den D - Dense D - Very Dense	ISE
See deta & ba	Expla ils of isis of	anator abbre f desci	/ Note /iation iption:	s for s s.					SMEC AUSTRALIA					SME	C

PF LC	ROJEC	CT ON	Rom	a Flood	Mitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :		PIT N FILE / JU SHEET	O: TPB53 OB NO : 30031182 : 1 OF 1
PC	OSITIC		: E: 6	79181.8′	6, N: 70	63069.(	064 (	5 MGA94) SURFACE ELEVATION : 300.758 (AH	D)		
DA		XCA	VATE	D: 14/1	14	cavalor	41	LOGGED BY : CM		CI	HECKED BY : CM
E>	(CAVA	ATIO	N DIM	ENSION	S : 0.5	0 m Wl	DE				
┝	z			ING ഷഗ	7 0		Z	MATERIAL			
VE	E PENETRATIC		GROUND WATE	SAMPLES	ELEVATIOI (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATIC SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	DENSITY DENSITY	요 101520	STRUCTURE & Other Observations
~				<u>1.30m</u> B	0.0		SM	Silty SAND: loose to medium dense, fine to medium grained, dark grey brown with clay fines content, grass rootlets to 0.3m <ul> <li></li></ul>	LtoMD		ALLUVIUM -
				<u>1.70m</u> <u>2.40m</u> B	1.5		CI — — CL-CI	2.40m becoming low to medium plasticity, increase in sand content, shading light brown.	VStto H		-
				2.80m	3.0 - - - - - - - - - - - - - - - - - - -		SM	2.80m Sitty SAND: dense, fine to medium grained, light brown/yellow brown with varying clay fines.	۵		
		PHOT	OGRAPIS	-15	4.5 - 4.5 - - - - - - - - - - - - - -			I.30m     EXCAVATION TPB53 TERMINATED AT 4.30 m       No ground water encountered.     Test pit backfilled on completion.			
	METHOD       PENETRATION         N       Natural Exposure         E       Existing Excavation         BH       Backhoe Bucket         B       Bulldozer Blade         R       Ripper         SUPPORT       T         T       Timbering					N TION TION S N 0 Oct., 7 evel on I vater inflor vater outf	o Res 3 Wate Date sl w low	tance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	BOLS & DN i em	F St VS F St VS H VL L V V V V V V U	DNSISTENCY/ ELATIVE DENSITY 5 - Very Soft - Soft - Firm - Stiff 5t - Very Stiff - Hard - Very Loose - Loose D - Medium Dense - Dense D - Very Dense
Se de & l	e Exp tails of basis o	lanat f abb of de	ory No reviatio scriptio	tes for ons ns.				SMEC AUSTRALIA			

PROJECT : Roma Flood Miti	igation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :		PIT NO : TPB54 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679891.042,	N: 7062766.769 (	55 MGA94) SURFACE ELEVATION : 304.9	89 (AHD)	
EQUIPMENT TYPE : KUBOT DATE EXCAVATED : 14/1/14	TA Excavator 4T	METHOD : Excavator LOGGED BY : CM		CHECKED BY : CM
EXCAVATION DIMENSIONS	: 0.50 m WIDE			
DRILLING	7	MATERIAL		
VE F PENETRATION H SUPPORT GROUND WATEF LEVELS SAMPLES & SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m) GRAPHIC LOG CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY	A STRUCTURE A Other Observations 5 101520
	-     SM	Silty SAND: loose to medium dense, fine to coarse grained, brown to dark brown, grass and rootlets to 0.2m	te MD D	-
4 MAG A MAG	e 1.0 e 1.5	some sandy clay and claying sand bands 0.70m Sandy CLAY: very stiff to hard, medium plasticity, dark brown grey to black, fine to medium grained sand high organic content, possible organic clay 1.50m	VSt to H MD	
	6 1.5 	Gravelly CLAY: very stiff to hard, medium plasticity, light brown, grey brown, orange brown and brown grey, mottled, fine to coarse subangular to rounded igneous gravels and cobbles to 100mm diameter, high fine to coarse grained sand content 1.90m Clayey GRAVEL: dense to very dense, fine to coarse, light brown, subangular to rounded, igneous, with cobbles to 200mm diameter, high		
ν. Γ	GC 	2.40m Silty SAND: dense, fine to coarse grained, light brown, with clay fines	D to VD	
4 COF	9.2.5 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	3.40m	<u> </u>	
0400 A PAG	0 4.0	4.20m     EXCAVATION TRE64 TEDMINATED AT 4.20 m		
000 2005	- - - - - - - - - 0 000 - - - - - - - -	EXCAVATION IFB34 IERMINATED AT 4.20 m No ground water encountered. Test pit backfilled on completion.		
PHOTOGRAPHS	بن 5.5 – الـــــــــــــــــــــــــــــــــــ	× NO		
NOTES	NETRATION	SAMPLES & FIELD TESTS     CLASSIFICATION       stance     U50 - Undisturbed Sample 50 mm diameter     Based on Classificat       D - Disturbed Sample     Classificat       B - Bulk Disturbed Sample     Classificat       MC - Moisture Content     MOISTURE       HP - Hand Penetrometer (UCS kPa)     D - Dry       VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa)     Wet       PBT - Plate Bearing Test     Wet	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/ RELATIVE DENSITY           VS         - Very Soft           S         - Soft           F         - Firm           St         - Stiff           VSt         - Very Stiff           H         - Hard           VL         - Very Loose           L         - Loose           MD         - Medium Dense           D         - Dense           VD         - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		

PROJECT : Roma Flooo LOCATION : Roma	d Mitigation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	PI FIL SH	IT NO : TPB55 LE / JOB NO : 30031182 HEET : 1 OF 1											
POSITION : E: 679964.3 EQUIPMENT TYPE : KU DATE EXCAVATED : 14/	POSITION         E: 679964.332, N: 7062771.437 (55 MGA94)         SURFACE ELEVATION         : 305.753 (AHD)           EQUIPMENT TYPE         : KUBOTA Excavator 4T         METHOD         : Excavator           DATE EXCAVATED         : 14/1/14         LOGGED BY         : CM           EXCAVATION DIMENSIONS         : 0.50 m WIDE         : 0.50 m WIDE         : 0.50 m WIDE														
EXCAVATION DIMENSIO DRILLING	NS : 0.50 m WIDE	MATERIAL													
VE E PENETRATION H SUPPORT GROUND WATER GROUND WATER EVELS	ELEVATION (RL) DEPTH (m) GRAPHIC LOG CLOG	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY DENSITY	STRUCTURE & Other Observations											
	4. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	Silty SAND: loose to medium dense, fine to coarse grained, brown to dark brown, grass and rootlets to 0.2m with clay fines content 0.30m varying bands/ lenses of silty sand, clayey sand and sandy clay, gravelly in parts, trace of cobbles to 100mm diameter.	L to MD	ALLUVIUM											
	0.5 - 1	1.90m XW MUDSTONE/SILTSTONE: extremely low strength, extremely weathered, pale grey, grey brown and yelow/orange brown, breaks down to gravelly clay. 2.20m becoming VLS, HW breaks down to clayey gravel	D to M MD	WEATHERED ROCK											
PHOTOGRAPHS NOTES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering		4.20m       EXCAVATION TPB55 TERMINATED AT 4.20 m No ground water encountered. Test pit backfilled on completion.         Test pit backfilled on completion.       Test pit backfilled on completion.         Image: Sample seistance       U50 - Undisturbed Sample 50 mm diameter       CLASSIFICATI SOIL DES Based Classifica         U50 - Undisturbed Sample B       D - Disturbed Sample B       MOISTURE D - Disturbed Sample B         MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test       MOISTURE	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Stiff H - Hard VL - Very Stiff H - Hard VL - Very Sose L - Loose MD - Medium Dense D - Dense VD - Very Dense											
See Explanatory Notes for details of abbreviations & basis of descriptions.	1 1	SMEC AUSTRALIA													
PR( LOC	DJECT CATIO	- : I N : I	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCA	V	CLIENT : Ostwald Bros Pty Ltd FEATURE :	ì		<b>PIT</b> FILE SHEI	NO / Joi et :	: <b>TPB56</b> 3 NO : 30031182 1 OF 1
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PO		N :	E: 678	3569.26	4, N: 70	61469.	252 ( 4т	55 MGA94)		SURFACE ELEVATION : 2	98.517 (	AHD)			
DA	TE EX	CAV	ATED	: 25/1/	14	cavaloi	41			LOGGED BY : CM				СН	ECKED BY : CM
EXC	CAVA			NSION:	S : 0.50	0 m WI	DE			ΜΔΤΕΡΙΔΙ					
	ENETRATION	UPPORT	DUND WATER	AMPLES &	EVATION (RL) EPTH (m)	sRAPHIC LOG	SSIFICATION SYMBOL	S	Soil 1	MATERIAL DESCRIPTION Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	OISTURE	NSISTENCY RELATIVE DENSITY	DCP		STRUCTURE & Other Observations
۳ u		s	GRO	S III	ם <u>ت</u> 		CLA	Sandy C		Y very stiff to hard medium plasticity, dark oney brown, fine	≥ c	<u>0</u> <u>-</u>	5 1015	20	ALLUVIUM
					- 298		СІ	0.30m	n grai	ined sand; grass and rootlets to 0.2m.		VSt to H			
				0.50m	-		СІ	becomin	ng ha	nard, shading grey brown, slow digging					
				B 0.70m	- 0.5 - 508:0		sc	Clayey S plasticity 0.70m	SAN y cla	ND: dense, dark grey, brown, fine to medium grained, mediu ay fines.	n				-
				В	-		sc	shading 5mm dia	g ligh amet	ht brown to brown; occasional white powdery calcite module eter, slight increase in sand content	s <				
				1.00m B	- 1.0 			1.00m shading	g ligh	ht brown, slow digging (single tyne ripper used)					
				1.30m	- 50		sc								
					- 0 <sup>.</sup> 26			1.50m occasion	 nal p	pale grey and light grey brown, finely mottled		т			-
					-										
					- 2.0 —		sc				Þ				
					- 50			2.30m			D to				
				2.50m	2.5-			Sandy C grey to g	CLA) grey	<ul> <li>Y: hard, grey brown, medium plasticity, light brown and pale y, mottled</li> </ul>					-
				⊿ 2.70m	296.(										
					-		CI					т			
					- 5.2										-
					-		сı	3.30m increase	e in s	sand content, grading fine to coarse grained					
					- <sup>3.5</sup>			Silty SAN with clay	ND: y fine	: dense, fine to coarse grained, light grey brown/light brown nes					
						1 1	SM								
		-			- 534 - 534 - 534			4.00m EXCAVA No grour Test pit b	ATIC and w	ON TPB56 TERMINATED AT 4.00 m water encountered. xhilled on completion.					
5					- - -	-									
					- 294.	-									
					5.0										
	PI	HOTOG	RAPHS		] YES		[	NO NO				VMPOU	<u> </u>	<u></u>	
MI N E B R SU	METHOD     PENETRATION     SAMPLES & FIELD TESTS     CLASSIFIC       No Natural Exposure     Sample     Soll     Soll       Existing Excavation     No Resistance     Uso - Undisturbed Sample     Base       Backhoe Bucket     Sullozer Blade     No Resistance     D - Disturbed Sample     Base       R Ripper     MATER     In Oct., 73 Water     WATER     D - Dr.       Impering     In Oct., 73 Water     VS - Vane Shear, P-Peak, R-Remouded (uncorrected kPa)     M - Mo						DESCRIP Sed on Un sification S RE y pist et	YMBOL: PTION ified ystem	5 &	REL VS S F St VSt H VL L MD	INISTENCY ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense Dense				
	Tim	nator	v Note	as for		vater inflo vater outf	low			PBT - Plate Bearing Test				VD	- Dense - Very Dense
deta	ails of a asis of	desci	viation	IS S.						SMEC AUSTRALIA					SMEC

PF LC	ROJ ICA	ECT	- :   N :	Roma Roma	I Flood N	<i>l</i> itigatio	n		EXCA	VATION - GEOLOGICAL LOC CLIENT : Ostwald Bros Pty Ltd FEATURE :	3		<b>P</b> I FII S⊦	IT NO Le / Jo Heet	O: TPB57 DB NO: 30031182 : 1 OF 1
P(	) SUI		NT :	E: 67	8641.66	6, N: 70 ЭТА Бх	61486. cavator	153 ( 4т	55 MGA94)	SURFACE ELEVATION : 2 METHOD : Excavator	98.765 (	AHD)			
DA	ATE	EEX	CAV	ATED	: 25/1/	14	avator	-		LOGGED BY : CM				Cł	HECKED BY : CM
E۷	(CA	VAT			INSION:	S : 0.5	0 m Wll I	DE		ΜΔΤΕΡΙΔΙ					
	NO			Ĕ	_s S	Z G	0	NOI			шz	∑			
VE	E PENETRATI	L I	SUPPOR	GROUND WA	SAMPLES FIELD TES	ELEVATIO	GRAPHIC LOG	CLASSIFICAT SYMBOL	So	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTUR	CONSISTEN RELATIVE DENSITY	5 10	3 1520	STRUCTURE & Other Observations
						- 0.0		SM	Silty SAN slight gre	ND: Loose to medium dense, fine to medium grained, brown, ey brown, grass and rootlets to 0.2m,	٥				ALLUVIUM
						-			0.20m with clay		-+	L to MC			-
					0.40m	298.5		SM	0.40m						-
					В	0.5		sc	Clayey S clay fines	SAND: slight grey brown, dense, fine grained, medium plasticity s, light brown to brown,	/				
					0.60m		(  ]-]- []-]-]-		0.60m shading l	light brown to brown and orange brown					-
					B			SC							-
						- 296									-
					1.00m	1.0		1	1.00m Silty SAN	ND: dense, fine to medium grained, light brown/yellow brown, v	with				-
						-		SM	clay fines	s					-
						97.5			increase	in silt content with clay fines, grading sand in parts,					-
						- <sup>10</sup>		SM	4.50		D to M				-
						1.5 —			decrease						_
						-									-
						297.0		SM							-
						-									-
						2.0 -									
						-			2.20mshading l	light brown/light grey brown slight vellow brown					-
						296.5		SM	2.40m						-
						2.5-	   	SM	decrease						-
			-			_			2.60m EXCAVA	ATION TPB57 TERMINATED AT 2.60 m					
00						-			No groun Test pit b	nd water encountered. backfilled on completion.					-
9 8.2.9						296.									-
114 12:5						3.0 —									-
2/03/20						-									-
ie>> 1						- 5.5									-
awingF						29(									-
Ū≫ ſo						3.5 —									-
VEE.GF						-	1								-
DOD LE						95.0									-
MAFLC						-									-
ЭЧ Ц		PH N/		RAPH	s			]	NO NO						1
	ИFT								*	SAMPLES & FIELD TESTS CLASSIFI	CATION S	MBOL	8&	cc	DNSISTENCY/
	N	Natu	ural Ex	posure				o Res	istance	U50 - Undisturbed Sample Ba	L DESCRIP ased on Uni	fied		VS S	- Very Soft - Soft
OUR.G	= BH	Exis Bacl	ting E khoe E	kcavat Bucket	ion					D - Disturbed Sample	RE	ysterri		F St	- Firm - Stiff
- COL	2	Bullo Ripp	lozer er	Biade	V	NATER	0.0-+ -	0 \A/-·	or	MC - Moisture Content HP - Hand Penetrometer (UCS kPa)	ry loist			H VL	- very stim - Hard - Very Loose
BRAR	SUP	POR	Г				u Oct., 7 evel on [ ater info	3 Wat Date s	er hown	VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)	ioist /et				- Loose D - Medium Dense
BANE L		ı ımt	bering				ater outf	low		PBT - Plate Bearing Test				VD	- Dense ) - Very Dense
SMEC BRIS de &	e E tails bas	xpla s of a is of	nator ibbre desc	y Note viatior ription	es for ns s.					SMEC AUSTRALIA					

EXCAVATION - GEOLOGICAL LOG           PROJECT : Roma Flood Mitigation         CLIENT : Ostwald Bros Pty Ltd           LOCATION : Roma         FEATURE :	PIT NO : TPB58 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION         E: 678917.392, N: 7061849.195 (55 MGA94)         SURFACE ELEVATION         : 298.797 (AHD)           EQUIPMENT TYPE         KUBOTA Excavator 4T         METHOD         : Excavator           DATE EXCAVATED         : 26/1/14         LOGGED BY         : CM	CHECKED BY : CM
EXCAVATION DIMENSIONS : 0.50 m WIDE       DRILLING       MATERIAL	
MATERIAL DESCRIPTION NOILUION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components Secondary and Minor Components	STRUCTURE & Other Observations 5 101520
Sandy CLAY: very stiff to hard, medium to high plasticity, light brown grey, fine to medium grained sand, grass and rootlets to 0.15	ALLUVIUM
shading grey brown with white powdery calcite nodules,	
2.80m     2.80m     becoming medium to high pasticity, decrease in sand content, silty clay in parts       3.10m     3.0     0       3.10m     3.5     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0       0     0	
000000000000000000000000000000000000	
METHOD       PHOTOGRAPHS NOTES       YES       NO         METHOD       VES       NO         N       Natural Exposure E Existing Excavation BH Backhoe Bucket B Buildozer Blade R Ripper       PENETRATION WATER       SAMPLES & FIELD TESTS US0 - Undisturbed Sample Somm diameter       CLASSIFICATION SYMBOLS SOIL DESCRIPTION Based on Unified Classification System         SUPPORT T       Timbering       Verter U Oct., 73 Water Level on Date shown water outflow       No Resistance D       US0 - Undisturbed Sample Somm diameter       MOISTURE D - Dry No Resistance D         Buildozer Blade R       No Ct., 73 Water Level on Date shown water outflow       Van e Shear; P-Peak, R-Remouded (uncorrected kPa)       D - Dry M - Moist W - Wet	S & CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.	SMEC.

PROJEC	:T : F ON : F	Roma Roma	Flood I	<i>l</i> itigatio	n		EXC	AV	ATION - GEOLOGICA CLIENT : Ostwald Bros Pr FEATURE :	L LOG			<b>PI</b> File She	<b>f no</b> E / Joi	0 : <b>TPB59</b> B NO : 30031182 1 OF 1
POSITIC	)N : E	: 678	8917.46	8, N: 70	061887.	564 (	55 MGA94)	)	SURFACE ELEVA	TION : 298.7	780 (4	AHD)	511		
			: CAT	Excava	tor 36T					vator M				CU	
EXCAVA			NSION	S : 0.5	0 m WI	DE								On	
	DR		G	1		-			MATE	RIAL		L			
VE E PENETRATION H	SUPPORT	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	ELEVATION (RL) DFPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL		Soil	MATERIAL DESCRIPTION I Type, Colour, Plasticity or Particle Characteris Secondary and Minor Components	stic	MOISTURE	CONSISTENCY RELATIVE DENSITY	ං වි 5 101	520	STRUCTURE & Other Observations
			0.20m B 0.30m	- 0.0 -		сі-сн	Sandy brown	y CLA n, with	AY: very stiff to hard, medium to hard plasticity, h fine grained sand, grass and rootlets to 0.15m	dark grey 1	٥	_			ALLUVIUM -
			0.40m B 0.70m 1.00m B	0.5 - 0.5 - 0.86 0.86 0.5 -		сі-сн	<u>0.80m</u> shadii 10mm	ing gre ing ligi n dian	ht brown to brown, some white powdery calcite meter	nodules to	_				
			1.30m 1.70m	- 5. - 5. - 1.5 -		сі-сн	1.60m shadii increa	ing ligi	ht brown, some pale grey brown, mottling calcit moisture content.	e nodules,	Σ	VSt to H			- - - - -
			2.00m 2.10m B 2.40m	0,20- 2.0- 5,967		сі-сн	<u>2.</u> 10m becon mottle	 ming r ed, fin	medium plaisticity, light brown, pale grey and so ne to medium grained sand	me grey brown,					- - - - - - - - - - - -
				50000		SP	2.60m SAND conter	D: den	nse, fine to medium grained, light grey brown, va creasing with depth	arying silt		۵			
				9.967 3.5 -			EXCA No gra Test p	AVATI round pit bao	ION 1PB59 TERMINATED AT 3.20 m water encountered. ckfilled on completion.						- - - - - - - - - - - - - - 
	РНОТОС	RAPHS		4.0-	1										
METHOI N Na E Exi BH Ba B Bu R Rip SUPPOF T Tin	PHOTOGRAPHS NOTES YES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering WATER (10 Oct., 73 Wat Level on Date s water inflow water outflow					Io Resi 3 Wate Date sl ow flow	NO istance er nown		SAMPLES & FIELD TESTS         U50       Undisturbed Sample         50 mm diameter         D       Disturbed Sample         B       Bulk Disturbed Sample         MC       Moisture Content         HP       Hand Penetrometer (UCS kPa)         VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       Plate Bearing Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unit ation Sy	MBOLS TION ied /stem	÷&	CON REL VS S F St VSt H VL L MD D VD	ISISTENCY/ ATIVE DENSITY - Very Soft - Firm - Stiff - Hard - Very Losse - Loose - Medium Dense - Dense - Very Dense
See Expl details of & basis o	anatory abbrev f descr	Note viation	s for s s.						SMEC AUSTRALIA						SMEC

PROJE LOCAT	CT ION	: Ror : Ror	na Flood I na	Mitigatio	n		EXCA	VATION - GEOLOGICAL CLIENT : Ostwald Bros Pty FEATURE :	LOG			<b>PIT</b> FILE SHE	' <b>NC</b> / JO ET :	D : <b>TPB60</b> B NO : 30031182 1 OF 1
POSITI EQUIPI DATE E EXCAV	ION MEN <sup>®</sup> EXCA /ATIC	: E: ( f typ .vate N din	78959.92 E : CAT D : 26/1/ /ENSION	25, N: 70 Excavat 14 S : 0.5	61893.: tor 36T 0 m WI	249 (: DE	55 MGA94)	SURFACE ELEVAT METHOD : Excava LOGGED BY : CM	ΓΙΟΝ : 298.94 ator 1	4 (A	(HD)		СН	ECKED BY : CM
		DRILI	ING					MATER	RIAL					
/E E PENETRATION	T	GROUND WATER	SAMPLES &	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	S	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristi Secondary and Minor Components	ic	MOISTURE	CONSISTENCY RELATIVE DENSITY	0 5 1015	520	STRUCTURE & Other Observations
			0.20m B	- 0.0		CL	Sandy Cl grained s	CLAY: very stiff to hard, low plasticity, dark brown gro sand, grass and rootlets to 0.2m	rey, fine	D				ALLUVIUM
			0.40m 0.50m B 0.80m 0.90m B			  	0.40mshading g 0.80m shading l 10mm dia	grey brown to brown, fine to medium sand content	cite nodules to		VSt to H			-
			1.70m B 2.00m			CL	1.60m shading I slight inc 2.00m SAND: di silt conte	light brown to brown slight grey brown, decrease in crease in moisture content. dense, fine to medium grained, light brown/yellow bro ent decrease with depth	sand content,	D to M				-
82.900				- 		- SM	2.80m EXCAVA	ATION TPB60 TERMINATED AT 2.80 m			٩			
<ul> <li>LiawingFile&gt;&gt; 12/03/2014 12:59</li> </ul>				- 0.6 296.0	-		Test pit b	backfilled on completion.						-
ROMA FLOOD LEVEE.GFJ <				- 5.5 - 56  50 50 50 50 										
	METHOD NOTES       YES       NO         METHOD N       YES       NO         METHOD N       PENETRATION Support       Samples & FIELD TESTS Solid Description       Samples Solid Description         B       Buildozer Blade R       No Resistance       US0       Undisturbed Sample S0 mm diameter       D       Disturbed Sample B       Builk Disturbed Sample B       Builk Disturbed Sample B       D       D       D       D       D       Based on Unified Classification System         Support T       T Timbering       Water utflow water outflow       Water Level on Date shown water outflow       PBT - Plate Bearing Test       D       D - Dry M       Moist							\$&	COT REL VS S F St VSt H VL L MD D VD	NSISTENCY/ ATIVE DENSITY - Very Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense				
See Exp details of & basis	plana of abb of de	ory N reviat scripti	otes for ons ons.					SMEC AUSTRALIA						

PR LO	OJEC	:T : DN :	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / J SHEET	O : <b>TPB61</b> OB NO : 30031182 : 1 OF 1
PO FQ	SITIO UIPM	N :	E: 678	3970.06 • CAT	2, N: 70 Excavat	61831. or 36T	483 (	55 MGA94) SURFACE ELEVATION : 299.0 METHOD : Excavator	039 (A	AHD)		
DA	TE EX	XCAV.	ATED	: 26/1/	14			LOGGED BY : CM			С	HECKED BY : CM
EX	CAVA			NSIONS	S : 0.50	) m WI	DE	MATERIAI				
	PENETRATION	SUPPORT	ROUND WATER	SAMPLES &	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	ONSISTENCY RELATIVE DENSITY	L O O 5 101520	STRUCTURE & Other Observations
<u> </u>		:	U	, 이 문	0.0	]////	ŭ	Sandy CLAY: very stiff to hard, medium plasticity, dark brown grey, fine to		Ō		ALLUVIUM
				0.40m	- 53 		Сі	0.40m	0	_		
				0.70m 0.90m B	0.5		CI	0.80m	_	VSt to H		
				1.20m	0.867 -         		CI		D to M			-
				2.00m	- 60		SM	1.80m Silty SAND: dense, fine to medium grained, pale grey to grey 1.95m Sandy CLAY: very stiff to hard, medium plasticity, dark grey brown to	0	0		
				B 2.30m	0.265		СІ	brown, fine to medium grained, some white powdery calcite modules <10mm diameter	) to M	VSt to H		_
006738 827.00					- 506.5 - -	       	SM	2.60m Silty SAND: dense, fine to medium grained, light brown/yellow brown, varying clay fines content 3.00m	_	۵		
ייטייטידי אקאוואנוואא					- 586.0			EXCAVATION TPB61 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				
.000 נבעבב.סרט איני					- 3.5 - 32: - 56 							-
					4.0-							
1	F	PHOTO	GRAPHS		] YES		[	NO				
	NOTES       YES       NO         METHOD       PENETRATION       SAMPLES & FIELD TESTS       Soll DESCRIPTION         N       Natural Exposure       Summaria       Soll Description         E       Existing Excavation       Soll Description       Based on Unified         BH       Backhoe Bucket       Soll Description       Based on Unified         B       Bulldozer Blade       WATER       D       D       Disturbed Sample         MATER       Image: Dot on the shown water inflow       Image: Dot on the shown water inflow       Image: Dot on the shown water inflow       No       No         PBT       Plate Bearing Test       PBT       Plate Bearing Test       Moist       W								S& CC Ri VS S F St VS H VL L MI D VI	DNSISTENCY/ ELATIVE DENSITY 5 - Very Soft - Soft - Firm - Stiff 5t - Very Stiff - Hard - Very Loose - Loose D - Medium Dense - Dense D - Very Dense		
MEC RKIN	e Expla ails of asis o	anator abbre f desc	y Note viatior ription	es for is s.				SMEC AUSTRALIA			·	SMEC

PR LO	OJEC <sup>.</sup> CATIC	T : N :	Roma Roma	Flood N	litigatio	n		EXCA	VATION - GEOLOGICAL LO CLIENT : Ostwald Bros Pty Ltd FEATURE :	G		<b>PIT</b> FILE SHE	<b>NO</b> / JOI	D : <b>TPB62</b> B NO : 30031182 1 OF 1
PO		N :	E: 679	9235.97	5, N: 70	62244.	498 (	55 MGA94)	SURFACE ELEVATION :	299.269	(AHD)			
DA	TE EX	CAV.	ATED	: 26/1/1	 14	01 001			LOGGED BY : CM				СН	ECKED BY : CM
EX	CAVA			INSIONS	S : 0.50	0 m WI I	DE		ΜΑΤΕΡΙΛΙ					
-	NO			a ST ∞ S	Z (	0	NOI		WATERIAL	ша				
VE	E PENETRATI F H	SUPPOR	GROUND WAT LEVELS	SAMPLES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	S	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE		5 1015	520	STRUCTURE & Other Observations
					- 0.0	[ [	SM	Silty SAN and rooth	ND: loose to medium dense, fine to medium grained, brown, y lets to 0.3m	grass	L to MD			ALLUVIUM
				0.40m	299.0	l L	SM	with some	le clay fines		MD			
				B	0.5			Clayey S medium p	SAND: dense, light brown/orange brown, fine to medium grain plasticity clay fines	ied,				
				0.70m	بن 		SC							
				0.90m	298			0.90m						
				В	1.0-	( 		Silty SAN some var depth.	ND: dense, fine to medium grained, light brown/yellow brown rying clay fines content in parts, decreasing fines content wit	with 1				
				1.20m		i i								
					- 1.5	1	SM			to M				
					-	l L					٥			
					297.5	t F								
				2.00m B	2.0-			2.05m pale grey	y to white sandy clay band, approximately 150mm thick					
				2.30m				2.20m SAND: D	Dense, fine to medium grained, light brown/yellow brown.					
					2.5-		SM							
					-									
					296.5			2.80m EXCAVA No groun Test pit b	TION TPB62 TERMINATED AT 2.80 m d water encountered. packfilled on completion.					
					3.0									
					- 596.0									
					3.5 —	-								
					-	-								
					295.5	-								
	P		i Graphs	ـــــــــــــــــــــــــــــــــــــ	⊥ <sub>4.0</sub> ] YES	ļ	1	NO NO						
N		)	0000		PENETRA				SAMPLES & FIELD TESTS CLASSI	FICATION S	SYMBOL PTION	S &	CON REL VS	NSISTENCY/ ATIVE DENSITY - Very Soft
E B R	E     Existing Excavation       BH     Backhoe Bucket       B     Bulldozer Blade       R     Ripper       WATER     Moisture Content       Classificatio       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -       D     -							URE	System		S F St VSt H	- Soft - Firm - Stiff - Very Stiff - Hard		
S T	UPPOR Tim	Ibering				0 Oct., 7 evel on l vater inflo vater out	3 Wate Date sl ow low	er nown	HP       Hand Penetrometer (UCS kPa)       M         VS       Vane Shear; P-Peak,       W         R-Remouded (uncorrected kPa)       PBT       Plate Bearing Test	Moist Wet			VL L MD D VD	- Very Loose - Loose - Medium Dense - Dense - Very Dense
See det & b	e Expla ails of asis of	anator abbre f desc	y Note viatior ription	es for is s.	. 1				SMEC AUSTRALIA			I		

PF	ROJE	ECT	:   	Roma	Flood N	<i>l</i> itigatio	n		EXC	CAV	ATION - G	EOLOGICA : Ostwald Bros F	AL LOG Pty Ltd			<b>PIT</b> FILE	NC / JC	<b>D</b> : <b>TPB63</b> B NO : 30031182
PC	DSIT		N .     :	E: 679	9314.094	4, N: 70	62260.	951 (	55 MGA94	4)	FEATURE	SURFACE ELEV	ATION : 299.4	152 ( <i>A</i>	AHD)	SHE	1	
EC DA		PME EX(	NT 1 CAV	YPE ATED	: CAT : 26/1/	Excavat 14	or 36T					METHOD : Exca LOGGED BY : (	avator CM				CH	IECKED BY : CM
E۷	(CA)	VAT	ION	DIME	NSION	S : 0.50	) m WI	DE										
-	N		DF		ାG ୁ≪ ମ	z o		Z				MAT	ERIAL		<u>≻</u>			
VE	E F F	н	SUPPORT	GROUND WATI LEVELS	SAMPLES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI		Soil	MATERIAL I Type, Colour, Plastic Secondary and I	DESCRIPTION ty or Particle Characte <i>I</i> inor Components	ristic	MOISTURE	CONSISTENC RELATIVE DENSITY	වි 5 101:	520	STRUCTURE & Other Observations
						- 0.0	l I	SM	Silty and 0.20m	y SAND: rootlets	loose to medium den to 0.3m	se, fine to medium grain	ned, brown, grass	D	L to MD			ALLUVIUM
						-	 	SM		n some c					QW			-
						- 0.5 — - - -		CI	San	ndy CLA wn, fine	Y: very stiff to hard, m to medium grained sa	edium plasticity, light br nd	rown/orange		VSt to H			
						- \$9 86 7 - -			Silty Som dept	y SAND: ne varyir th.	dense, fine to mediu ng clay fines content in	n grained, light brown/y parts, decreasing fine:	rellow brown with s content with	-				- - - - -
						- 86 1.5 — - -		SM						D to M				
						- 19:2.0 — 70 -		SM	2.05m pale 2.20m SAN	e grey to	white sandy clay ban	d, approximately 150mi ained, light brown/yellov		_	Ω			
8						- 0. 2.5 — - -		SP	2.80m									
IIe>> 12/03/2014 12:59 8.2.9						- 96 3.0 - -			EXC No <u>c</u> Test	CAVATIO ground v t pit bac	ON TPB63 TERMINA <sup>*</sup> water encountered. kfilled on completion.	'ED AT 2.80 m						-
JD LEVEE.GPJ <<⊔rawingr						- 0.967 - - -												-
MAFLOU						-												
5 L		PH		I BRAPHS	; ;	⊥;; 4.0	ļ											1
	PHOTOGRAPHS VE NOTES PERIT METHOD PENET N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering			PENETRA > □ □ □ □ VATER ↓ □ □ VATER	TION	lo Res 3 Wat Date s ow	istance er nown		SAMPLES & FIELD 50 mm dia D - Disturbed B - Bulk Distu MC - Moisture ( HP - Hand Per VS - Vane She R-Remou PBT - Plate Bea	o TESTS ed Sample mmeter Sample rbed Sample Content etrometer (UCS kPa) ar; P-Peak, ded (uncorrected kPa) ring Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unif ation Sy	TION TION ied rstem	5&	CORE VS SF St St H VL MD VD	NSISTENCY/ _ Very Soft - Soft - Firm - Stiff - Very Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense		
MEC BRISE de & I	e Ex tails basis	plar of a of of of	nator bbre desc	y Note viation	s for s.						SMEC AL	JSTRALIA				<b>I</b>		SMEC

PRO LOC	JEC <sup>-</sup> ATIO	T : N :	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT NO</b> FILE / JOI SHEET :	D : <b>TPB64</b> B NO : 30031182 1 OF 1
POS		N :	E: 679	184.52	2, N: 70	62229.	417 (	55 MGA94) SURFACE ELEVATION : 300.2	69 (/	AHD)		
DAT	E EX	EN I (CAV)	ATED	: CAT : 26/1/	Excava 14	tor 36 i		LOGGED BY : CM			СН	ECKED BY : CM
EXC	AVA	TION	DIME	NSION	S : 0.5	0 m WI	DE					
<u> </u>	7	D	RILLIN	IG " Ø	1_		z	MATERIAL		>		
LE E	F PENEIKALIO	SUPPORT	GROUND WATE LEVELS	SAMPLES &	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	පු 5 101520	STRUCTURE & Other Observations
						ĺ.	SM	Silty SAND: loose to medium dense, fine to medium grained, brown, grass and rootlets to 0.3m,	٥	to MD		_
					300.0		SM	0.20m with some clay fines		Q		-
				0.60m B	0.5 -			0.50m Sandy CLAY: very stiff to hard, medium plasticity, light brown/orange brown, fine to medium grained sand, occasional white powdery calcite nodules				
				0.90m	539.5		CI	1.00m				-
				1.20m B				shading light brown/light grey brown with white powdery calcite nodules, decrease in sand content		St to H		-
				1.50m	- 1.5 -		CI		×	\$		-
					298.5			1.80m	D to			-
					2.0 -	1	CI	2.00m Silty SAND: dense, fine to medium grained, light brown/light grey brown vary clay fines content				-
					298.0		SM					-
					2.5			2.60m decreasing fines content				-
							SM	3.00m EXCAVATION TPB64 TERMINATED AT 3.00 m				-
					0.76	-		No ground water encountered. Test pit backfilled on completion.				-
					3.5 -							-
					296.5	-						-
	P	нотос	RAPHS		4.0-		<u>                                     </u>					
ME	N THOD	OTES						SAMPLES & FIELD TESTS	ON S	(MBOLS	& CO	
N E BH R SU	METHOD     PENETRATION       N     Natural Exposure       E     Existing Excavation       BH     Backhoe Bucket       B     Buildozer Blade       R     Ripper       SUPPORT     In Oct., 73 V					10 Oct., 7 .evel on I	o Res 3 Wat Date s	er hown VS - Vane Shart P-Peak, VS - Vane Shart, P-Peak, VS - Vane Shart, P-Peak, VS - Vane Shart, P-Peak, VS - Vane Shart, P-Peak, VS - Vate Shart P-Peak,	SCRIP on Unition Sy	TION fied /stem	KEL VS S F St VSt H VL L MD	ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense
T	Tim Expla	anator	y Note	s for		vater inflo vater outf	w low	R-Remouded (uncorrected kPa) PBT - Plate Bearing Test			D VD	- Dense - Very Dense
& ba	sis of	desc	riptions	5 6.				SMEC AUSTRALIA				SMEC

PR		T: N:	Roma Roma	Flood N	<i>l</i> itigatio	ı		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT N</b> FILE / J SHEET	O: <b>TPB65</b> IOB NO : 30031182 : 1 OF 1
PO	SITIO	N :	E: 679	9231.74	6, N: 70	62310. <sup>-</sup>	146 (	5 MGA94) SURFACE ELEVATION : 30	).554 (	AHD)		
	TE E	CAV	ATED	: 26/1/	Excavat 14	01 30 1		LOGGED BY : CM			С	HECKED BY : CM
EX	CAVA	TION	DIME	NSION	S : 0.50	) m Wll	DE					
-	z			s αΩ	z o		S	MATERIAL		λ		
, ve	e penetratio H	SUPPORT	GROUND WATI LEVELS	SAMPLES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	요 5 101520	STRUCTURE & Other Observations
					0.02		SM	Silty SAND: loose to medium dense, fine to medium grained, brown, grast and rooflets to $0.3\mbox{m}$	s			ALLUVIUM
					۳ -			0.20m				
					- 0.5 0:00 - - - -		SM SM	0.60m		L to MD		
					1.0 — 1.5 — - 1.5 — 0:		SM	1.00m shading lighter grey brown	D to M			
					- 56		СІ	Silty CLAY: very stiff to hard, medium plasticity, lighter brown/orange brown, fine to medium grained sand		St to H		
					2.0 — 5:867 — -			2.20m Silty SAND: dense, fine to medium grained, light brown, with varying clay content				-
:59 8.2.900					- 2.5 — 0. 867 — - -		SM SM	2.80m		٩		
21 4102/01/21 CC9					- <u></u> 			3.00m EXCAVATION TPB65 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				
A FLOOD LEVEE.GPJ < <ul> <li>Surawingri</li> </ul>					3.5 — 0.267 — - -							
м У	L L L	НОТО		L	4.0-		<u> </u>					
	PHOTOGRAPHS NOTES       YES       NO         METHOD       PENETRATION       SAMPLES & FIELD TESTS       CLA         N       Natural Exposure E       Existing Excavation BH Backhoe Bucket B       PENETRATION       U50 - Undisturbed Sample 50 mm diameter       U50 - Undisturbed Sample B       MC         WATER       WATER       10 Oct., 73 Water Level on Date shown water inflow       MC       Moisture Content HP       MC         BUIPPORT T       Timbering       10 Oct., 73 Water Level on Date shown water outflow       VS       Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W						NO       SAMPLES & FIELD TESTS       CLASSIFIC. SOIL I         stance       U50 - Undisturbed Sample 50 mm diameter       CLASSIFIC. SOIL I         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content       MOISTURI D - Dry M - Moi         r       Own       HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       M - Moi W - We	ATION S DESCRIP ed on Uni ication S	YMBOLS TION fied ystem	S& CC Rt V: S F S V: H VI L M D VI	ONSISTENCY/ ELATIVE DENSITY S - Very Soft - Firm t - Stiff St - Very Siff - Hard L - Very Loose - Loose D - Medium Dense D - Medium Dense D - Very Dense	
See deta & b	e Expla ails of asis of	anator abbre f desc	y Note viatior ription	es for is s.				SMEC AUSTRALIA			<b>I</b>	

PROJE LOCAT	CT : ION :	Roma Roma	Flood N	<i>l</i> itigatio	n		EXCA	VATION - C CLIENT FEATURE	EOLOGICA : Ostwald Bros F	<b>LLOG</b> Pty Ltd			<b>PIT</b> FILE / SHEE	NO / Job et :	: <b>TPB66</b> NO : 30031182 1 OF 1
POSITI	ION :	E: 679	841.28	9, N: 70	61714.	238 (!	55 MGA94)		SURFACE ELEV	ATION : 302.4	427 (A	AHD)			
DATE E	EXCAV	ATED	: KUB : 11/2/	JTA EX 14	cavator	41			LOGGED BY : 0	avator CM				CHE	ECKED BY : CM
EXCAV	/ATION	I DIMEI	NSION	S : 0.5	0 m WI	DE									
z		RILLIN	G ແມ	7 0		Z			MATI	ERIAL		<u>≻</u>			
VE E PENETRATIO F	H SUPPORT	GROUND WATE LEVELS	SAMPLES 8 FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC SYMBOL	So	MATERIAL bil Type, Colour, Plasti Secondary and	DESCRIPTION city or Particle Characte Minor Components	ristic	MOISTURE	CONSISTENC RELATIVE DENSITY	요 5 1015	20	STRUCTURE & Other Observations
				-		сі-сн	Sandy Cl grained s	_AY: very stiff, medium and, grass and rootlets	to high plasticity, dark gr to 0.2m	ey brown, fine	Q				ALLUVIUM - -
				- 0;03 - 0.0 305 - -		СІ-СН — —	0.60m shading l nodules,	ight brown to brown, so < 10mm diameter	me white powdery and c	rystalline calcite					-
				۔ - 1.0 ج		сі-сн	0.90m	ight brown and dark broine angular gravels, inc	own grey with calcite nod rease in moisture conten	ules as above	D to M				-  -
				- 010 1.5 - -		CI-CH	Silty CLA powdery gravel	Y: very stiff, medium to crystalline calcite nodu	high plasticity, light brow les with fine to medium s	/n, some white and and trace of		VSt			
				- 900 2.0 - - - - - - - - - - - - - - - - - - -		CI-CH	<u>1.90m</u> shading I trace of fi	ight brown/yellow brow ne grained sand.	n becoming moist, withou	t calcite nodules,	×				- - - - - - - - - - - - - - - - - - -
						2.90m EXCAVA No groun Test pit b	TION TPB66 TERMINA d water encountered. ackfilled on completion	.TED AT 2.90 m							
	РНОТО	GRAPHS		<u>⊣</u> ജ് <sub>4.0</sub> –	L	r									
METHON N N E E BH E B E R F SUPPO T T	PHOTOGRAPHS NOTES VES METHOD ↓ Natural Exposure Existing Excavation BH Backhoe Bucket B Buildozer Blade R Ripper SUPPORT Timbering				73 Wate Date sh Dw flow	× NO stance er nown	SAMPLES & FIEL U50 - Undistur 50 mm d D - Disturbe B - Bulk Dist MC - Moisture HP - Hand Pe VS - Vane Sh R-Remo PBT - Plate Be	D TESTS bed Sample iameter d Sample urbed Sample Content netrometer (UCS kPa) ear; P-Peak, uded (uncorrected kPa) aring Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	TION SY SCRIP on Unit ation Sy	<b>MBOLS</b> TION ied rstem	&	CON REL/ VS S F St VSt H VL L MD D VD	SISTENCY/ ATIVE DENSITY - Very Soft - Firm - Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense	
See Exp details o & basis	planato of abbre of desc	ry Notes eviations criptions	s for s					SMEC A	USTRALIA						

PROJECT : Roma Floo LOCATION : Roma	d Mitigation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd EATURE :		PIT NO : TPB67 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679896. EQUIPMENT TYPE : KU DATE EXCAVATED : 11	057, N: 7061718.663 (55 JBOTA Excavator 4T /2/14	MGA94) SURFACE ELEVATION : 303.11 METHOD : Excavator LOGGED BY : CM	57 (AHD)	CHECKED BY : CM
	DNS : 0.50 m WIDE	ΜΔΤΕΡΙΔΙ		
ENETRATION SUPPORT OUND WATER	ELD TESTS LEVATION (RL) EPTH (m) BRAPHIC LOG LOG XSSIFICATION XSSIFICATION	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION SONDITION INSISTENCY RELATIVE DENSITY	C STRUCTURE C & Other Observations
		Gravelly CLAY: very stiff to hard, medium plasticity, light brown, brown and brown grey variable, fine to coarse angular to subrounded gravels, with fine to coarse sand in parts, grass and rootlets to 0.2m, 30m		101520 ALLUVIUM
	0.5	Sandy CLAY: very stiff to hard, medium to high plasticity, dark grey brown, fine to medium grained sand.	^St	-
	1.0 - CI-CH	shading brown, becoming very stin, <u>90m</u> shading light brown and dark grey brown, with white powdery and crystalline calcite nodules, trace of fine angular gravels, fissured in parts.		
	020 1.5	20m	D to M	
	2.0 - CI-CH	Som Silty CLAY: very stiff, medium to high pasticity, light brown, some white powdery and crystaline calcite modules, with fine to medium grained sand and fine gravels.	VSt	
	2.5	50mshading light brown vellow brown maist without cabite podules trace of		
		fine grained sand	×	
	3.0 - - - - 3.5 -	EXCAVATION TPB67 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.		
	4.0			
PHOTOGRAPHS NOTES	YES	NO		
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	PENETRATION Wur I WATER 10 Oct., 73 Water Level on Date show water inflow water outflow	SAMPLES & FIELD TESTS     CLASSIFICATIN SOIL DES Based or Classificat       und     -     Undisturbed Sample 50 mm diameter     Based or Classificat       D     -     Disturbed Sample       B     -     Bulk Disturbed Sample       MC     -     Moisture Content       HP     -     Hand Penetrometer (UCS kPa)       VS     -     Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       PBT     -     Plate Bearing Test	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/ RELATIVE DENSITY         VS       - Very Soft         S       - Soft         F       - Firm         St       - Stiff         VSt       - Very Stiff         H       - Hard         VL       - Very Loose         L       - Loose         MD       - Medium Dense         D       - Dense         VD       - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		SMEC

PROJECT	:Roma Flo I:Roma	od Mitigatior	1	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			PIT NO FILE / JOI SHEET :	0 : <b>TPB68</b> B NO : 30031182 1 OF 1
POSITION EQUIPMEN DATE EXC	: E: 67988 NT TYPE : K CAVATED : 1	5.762, N: 706 (UBOTA Exc 1/2/14	61686.003 (5 avator 4T	5 MGA94) SURFACE ELEVATION : 302.1 METHOD : Excavator LOGGED BY : CM	712 (/	AHD)	СН	ECKED BY : CM
EXCAVAT		IONS : 0.50	) m WIDE	ΜΑΤΕΡΙΔΙ				
PENETRATION	SUPPORT ROUND WATER LEVELS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG ASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	ONSISTENCY RELATIVE DENSITY	00 5 101520	STRUCTURE & Other Observations
<u>&gt; u</u>		0.0	ci	Gravelly CLAY: very stiff to hard, medium plasticity, light brown and brown, variable, fine to coarse gravels, with fine to medium grained sand, grass, rootlets to 0.15m		0		ALLUVIUM -
		302.5	СІ	Sandy CLAY: very stiff to hard, medium plasticity, dark grey brown, fine to medium grained sand, occasional rootlets to 0.3m	- 0	рH		-
		0.5	СІ-СН	0.40m		^St i		
			сі-сн	1.10m occasional white powdery calcite nodules,	_			-
		ё – 1.5 — -	CI-CH	shading brown and dark brown grey, with white powdery and crystalline calcite nodules, < 10mm diameter trace of fine, angular gravels in parts, 1.60m Sitty CLAY: very stiff, medium to high pasticity, light brown, with some	×			- - - -
		0.10 0.10 0.0 - - - - - - - - - - - - -	CI-CH	while powdery and crystalline calcite hodules, with line to medium grained sand and fine gravels	D to	VSt		- - - - -
		00 - 2.5 - 0.000 - -	СІ-СН	2.30m				-
		<del>3.0</del> 	<i>811      </i>	3.00m EXCAVATION TPB68 TERMINATED AT 3.00 m No ground water encountered. Test pit backfilled on completion.				- - - - - - -
		- 500						-
METHOD N Natur E Existi BH Backi R Rippe SUPPORT T Timbe	OTOGRAPHS TES ral Exposure ing Excavation hoe Bucket ozer Blade er ering		D Oct., 73 Wate PO Oct., 73 Wate Po Oct., 73 Wate Po D Date st ater inflow ater outflow	NO       SAMPLES & FIELD TESTS       CLASSIFICAT         stance       U50 - Undisturbed Sample 50 mm diameter       Based Classifica         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content       MOISTURE         MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       D - Dry M - Moist W - Wet         PBT - Plate Bearing Test       PBT	TON S SCRIP on Uni ation S	YMBOLS ( TION fied ystem	& CON REL VS F St VSt H L MD D VD	ISISTENCY/ ATIVE DENSITY - Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
See Explan details of at & basis of d	atory Notes fo obreviations lescriptions.	r		SMEC AUSTRALIA				SMEC

PROJECT : Roma Flood Mitig	EXCAN	ATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd		PIT NO : TPB69 FILE / JOB NO : 30031182 SHEET : 1 OE 1
POSITION : E: 679877.674, N EQUIPMENT TYPE : KUBOTA	: 7061648.173 (55 MGA94) Excavator 4T	SURFACE ELEVATION : 301.6 METHOD : Excavator	14 (AHD)	
EXCAVATION DIMENSIONS :	0.50 m WIDE			
DRILLING	7	MATERIAL		
VE F PENETRATION H SUPPORT GROUND WATEF GROUND WATEF LEVELS FIELD TESTS ELEVATION	DEPTH (m) DEPTH (m) CRAPHIC LOG SYMBOL SYMBOL	MATERIAL DESCRIPTION il Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY	C STRUCTURE & Other Observations
301.0 301.5	0.0 Silty SANE brown, sor CI Sandy CL CL 0.25m medium gr CI 0.40m becoming 0.5 CI 0.40m becoming with white	D: medium dense, fine to medium grained, light brown and me clay content, some fine to medium gravels AY: very stiff to hard, medium plasticity, dark grey brown, fine to rained sand, occasional rootlets to 0.3m I moisture content, Very stiff, brown, I moisture content, I moisture cont	VSt VSt MD	ALLUVIUM
300.5	1.0	rown and dark brown grey, with calcite nodules as above, trace gular gravels, fissured in parts. very stiff, medium to high plasticity, light brown, with some dery and crystalline calcite nodules with fine to medium grained e of fine gravels	D to M	
2965	- <u>1.90m</u>	ght brown/yelow brown, without white nodules, trace of fine	M VSt	
298.0 298.5 299.0	3.0	ION TPB69 TERMINATED AT 3.10 m water encountered. cckfilled on completion.		
	4.0			-
PHOTOGRAPHS NOTES	res No ETRATION u ⊥ T → No Resistance ER 10 Oct., 73 Water Level on Date shown water inflow water outflow	SAMPLES & FIELD TESTS       CLASSIFICAT SOIL DE: Soll DE: Based Classifica         U50 - Undisturbed Sample 50 mm diameter       Based Classifica         D - Disturbed Sample       MOISTURE         B - Bulk Disturbed Sample       MOISTURE         MC - Moisture Content       D - Dry         HP - Hand Penetrometer (UCS kPa)       M - Moist         VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       W - Wet         PBT - Plate Bearing Test       Vane Shear; Patrice Alexandre Alexan	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/         RELATIVE DENSITY         VS       - Very Soft         S       - Soft         F       - Firm         St       - Stiff         VSt       - Very Stiff         H       - Hard         VL       - Very Loose         L       - Loose         MD       - Medium Dense         D       - Dense         VD       - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		SMEC

PROJECT : Roma Flood Mitigation LOCATION : Roma CLIENT : Ostwald Bros Pty Ltd FEATURE :		PIT NO         TPB70           FILE / JOB NO         : 30031182           SHEET : 1 OF 1
POSITION         E: 679252.688, N: 7059313.809 (55 MGA94)         SURFACE ELEVATION         299           FOUNDATION         FOUNDATION         FOUNDATION         FOUNDATION         299	5.972 (AHD)	
EQUIPMENT TYPE : KUBOTA Excavator 4T     METHOD : Excavator       DATE EXCAVATED : 12/2/14     LOGGED BY · CM		CHECKED BY · CM
EXCAVATION DIMENSIONS : 0.50 m WIDE		
DRILLING MATERIAL		
MATERIAL DESCRIPTION MATERIAL DESCRIPTION Solid Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations 5 101520
0.0 Silty SAND: loose to medium dense, fine to medium grained, dark brown,		ALLUVIUM
0.40m       Gravelly CLAY: very stiff to hard, medium pasticity, brown and orange brown, fine to coarse gravels trace of cobbles to 60mm         0.40m       0.40m         B       0.40m         0.60m       0.40m         0.70m       0.60m         B       0.60m         0.70m       0.60m         CI-CH       Shading brown to dark grey brown,		
1.00m       0       1.0         1.20m       1.20m         Sandy CLAY: very stiff to hard, medium to high pasticity, light brown to brown, some white powdery calcite modules, < 20mm diameter, fine to medium grained sand,	D to M VSt to H	
2.20m B 2.40m 12 2.40m 12 2.5 - 2.00m Shading brown and grey mottled, partly fissured, some dark grey veneer of staining along fissures	7 -	
2.60m shading pale grey, light brown and brown, mottled.		
PHOTOGRAPHS NOTES       98 VES       NO         METHOD NoTES       PENETRATION VES       SAMPLES & FIELD TESTS US0 - Undisturbed Sample 50 mm diameter       CLASSIFIC. SOLL I Base Classifi D         METHOD BH Backhoe Bucket B Bulldozer Blade R Ripper       PENETRATION WATER       U50 - Undisturbed Sample 50 mm diameter       CLASSIFIC. SOLL I Base Classifi D         MULL SUPPORT T Timbering       VMLER I Corrected KPay Water unflow       Image: Classific MOISTURE D       D	ATION SYMBOLS & DESCRIPTION ed on Unified fication System E st t	CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions. SMEC AUSTRALIA		

PROJE LOCAT	CT : 10N :	Roma Roma	Flood M	litigation	n	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT NC</b> FILE / JC SHEET	D : <b>TPB71</b> DB NO : 30031182 : 1 OF 1_
POSITI	ION :	E: 679	283.227	7, N: 70	59258.564 (	55 MGA94) SURFACE ELEVATION : 295	.799 (/	AHD)		
		ATED	. KUBC : 12/2/1	14 EX	Lavator 41	LOGGED BY : CM			CH	ECKED BY : CM
EXCAV	ATION		NSIONS	6 : 0.50	) m WIDE					
NO	<u>ь</u>		sts TS	N (F	U N		шZ	, Z		
-VE E PENETRAT	H SUPPOR	GROUND WA LEVELS	SAMPLES FIELD TES	ELEVATIO	GRAPHI LOG CLASSIFICAT SYMBOI	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTUR CONDITIC	CONSISTEN RELATIVI DENSITY	요 5 101520	STRUCTURE & Other Observations
				- 0.0 - - - - - - - - - - - - - - - - - - -	CI-Cł	Silty CLAY: very stiff to hard, medium to high plasticity, dark grey brown, with some fine to medium grained sand, grass rootlets to 0.2m				ALLUVIUM -
				0.5 — - - - - - - - - - - - - - - - - - - -	1 ICI-CH	0.90m Sandy CLAY: very stiff to hard, medium to high plasticity, brown, fine to medium grained sand	D to M			
				- 1.5	CI-Cł		×	VStto H		-
				0.462 - 2.0 - - - -	ci-cł	2.10m becoming medium plasticity, shading slight grey brown, increase in sand content		-		- - - - - - -
2032014 13:00 8.2.900				67 - 2.5 - - 0 67 - 3.0 - - 3.0 -	CI-CH	2.40m	D to M			-
LOOD LEVEE.GPJ < <drawingfile> 1</drawingfile>						EXCAVATION TPB71 TERMINATED AT 3.20 m No ground water encountered. Test pit backfilled on completion.				-
SOMAF				4.0						
METH	PHOTO NOTES	GRAPHS	P	YES	TION	NO SAMPLES & FIELD TESTS CLASSIFICA SOIL	TION S	YMBOLS	& CO RE	NSISTENCY/ LATIVE DENSITY
BANE LIBRARY - CULUUR SILE LO	Natural E: Existing E Backhoe I Bulldozer Ripper <b>DRT</b> Timbering	kposure xcavatio Bucket Blade	on		No Res 0 Oct., 73 Wa evel on Date s ater inflow ater outflow	istance U50 - Undisturbed Sample Base 50 mm diameter Classifi D - Disturbed Sample Classifi D - Disturbed Sample MOISTURE MC - Moisture Content D - Dry HP - Hand Penetrometer (UCS kPa) Nown VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	d on Uni cation S	fied ystem	VS F SS H VL L D VD VD	- Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See Exp details of & basis	planator of abbre of desc	y Note viation riptions	s for s s.			SMEC AUSTRALIA				

PRO LOC	JECT ATION	: I N : I	Roma Roma	Flood I	Vitigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			<b>PIT</b> FILE SHE	<b>' NO</b> : / Joi :et :	D : <b>TPB72</b> B NO : 30031182 1 OF 1
POS		:   NT T	E: 679	260.67	9, N: 70	59216.	999 (	55 MGA94) SURFACE ELEVATION : 296.97	14 (A	AHD)			
DAT	E EXC			: 12/2/	14	cavator	41	LOGGED BY : CM				СН	ECKED BY : CM
EXC	AVAT	ION	DIME	NSION	S : 0.5	) m WI	DE						
ā	ŝ		HELIN	sΩ∞	Z G		N	MATERIAL		5			
VE E DENETROTIN	H H	SUPPOR <sup>-</sup>	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	년 5 1015	520	STRUCTURE & Other Observations
					- 0.0 - - - - - - - - - - - - - - - - - -		СІ-СН	Silty CLAY: very stiff to hard, medium to high pasticity, dark grey brown, with some fine to medium grained sand, grass rootlets to 150mm	۵	-			ALLUVIUM - - - -
					- - - - - - - - - -		сі-сн	1.10m Sandy CLAY: verv stiff to hard, medium to high plasticity, brown slight	D to M				
				1.20m B 1.40m			сі-сн	grey brown, fine to medium grained sand					-
				<u>1.50m</u>	9:96 - 1.5 -			becoming medium plasticity, light brown, occasional white powdery calcite nodules, increase in fine to medium grained sand content, moist		VSt to H			
				<u>1.80m</u>			СІ	<u>2.30m</u>	Σ	_			- - - - -
			-	2.40m B 2.80m	- <u>1</u> 1967 2.5 - -		СІ	shading light grey brown and some orange brown mottling, increase in sand content, slightly lower plasticity.	D to M				
					- 0.8 594.0			3.10m EXCAVATION TPB72 TERMINATED AT 3.10 m					
					- - - - - - - - - - - - - - - - - - -			rvo grouna water encountered. Test pit backfilled on completion.					
					-								
	PH		RAPHS		] YES		[	NO NO					
ME N BH B R SUI T	THOD Natu Exist Back Bulld Rippo PPORT Timb	ral Ex ing Ex hoe E lozer I er er	posure ccavatio ucket Blade	n		0 Oct., 7 evel on vater inflo	lo Resi 3 Wate Date sl ow flow	stance U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	ON SY CRIP In Unifi ion Sy	<b>MBOL</b> : TION ied /stem	S &	CON REL VS S F St VSt H VL L MD D VD	ASISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
See detai & ba	Explar Is of al sis of c	hator bbrev desci	/ Note /iation: iptions	s for s s.	_	_	_	SMEC AUSTRALIA	_	_	_	_	

PROJEC LOCATIO	CT : ON :	Roma Roma	Flood N	litigatio	n		EXCAVATION - GEOLOGICAL LO CLIENT : Ostwald Bros Pty Ltd FEATURE :	G		<b>PIT NO</b> FILE / JO SHEET	<b>D</b> : <b>TPB73</b> DB NO : 30031182 : 1 OF 1
POSITIC	ON :	E: 679	9261.146	6, N: 70	59185.	803 (5	55 MGA94) SURFACE ELEVATION :	296.529 (/	AHD)		
EQUIPM		TYPE	: KUBC		cavator	4T	METHOD : Excavator				
			: 12/2/1	14 3 · 0 5/	) m \//	DE	LOGGED BY : CM			CH	IECKED BY : CM
EACAVA	אטוזר וח		ING	J. U.5			ΜΑΤΕΡΙΔΙ				
z		E E	a SI	Z O	0	S			5		
VE E PENETRATIO H	SUPPORT	GROUND WAT LEVELS	SAMPLES FIELD TES	ELEVATIO (RL) DEPTH (m	GRAPHIC LOG	CLASSIFICATI SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTEN RELATIVE DENSITY	ි 5 101520	STRUCTURE & Other Observations
				- 596.5			Sandy CLAY: very stiff to hard, medium to high plasticity, dark grey brown, with some fine to medium grained sand, grass rootlets to 0.2rr	n _			ALLUVIUM
			0.50m B 1.00m 1.80m B 2.20m	- 386 - 386 386 386 386 386 386 386 386 		сі-сн сі-сн	1.00m         1.00m         shading slight grey, brown to dark grey brown         1.60m         becoming medium to high plasticity, brown, fine to medium grained set         2.50m         andth figured shading slight grey brown with dark oray	n	VSt to H		
				- 53 53 		СІ-СН	3.10m EXCAVATION TPB73 TERMINATED AT 3.10 m No ground water encountered. Test pit backfilled on completion.				-
	PHOTOC NOTES D atural Existing E	GRAPHS xposure xcavati	3 P oon	4.0	TION	o Resi	SAMPLES & FIELD TESTS CLASSII stance U50 - Undisturbed Sample E 50 mm diameter Cla	FICATION S DIL DESCRIP Based on Uni assification S	YMBOLS TION fied ystem	S& CO RE VS S F	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm
BH Ba B Bu R Rij SUPPOI T Tir See Expl details of	ackhoe I ulldozer pper RT mbering lanator f abbre	Blade Blade y Note viation	es for		0 Oct., 7 evel on I vater inflo	3 Wate Date sh w low	B - Bulk Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test SMEC AUSTRALIA	URE Dry Moist Wet		St VS H L D VD	- Stiff - Very Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense - Very Dense
& basis o	of desc	ription	S.								

PROJECT : Roma Flood Mitigati	on	ATION - GEOLOGICA CLIENT : Ostwald Bros Pr FEATURE :	<b>L LOG</b> ty Ltd	PIT NO : TPB74 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679810.652, N: 7 EQUIPMENT TYPE : KUBOTA E	062123.589 (55 MGA94) xcavator 4T	SURFACE ELEVA METHOD : Exca	ATION : 305.628 (AHD) vator	
DATE EXCAVATED : 12/2/14 EXCAVATION DIMENSIONS : 0.	50 m WIDE	LOGGED BY : CI	Μ	CHECKED BY : CM
DRILLING	7	MATE	RIAL	
VE E PENETRATION H SUPPORT GROUND WATEF GROUND WATEF ELEVELS FIELD TESTS ELEVATION	DEPTH (m) GRAPHIC LOG SYMBOL SYMBOL SYMBOL	MATERIAL DESCRIPTION il Type, Colour, Plasticity or Particle Characteri: Secondary and Minor Components	A CONDITION CONSISTENCY RELATIVE DENSITY	STRUCTURE C & Other Observations 5 101520
	CI CI CI CI CI CI CI CI CI CI CI CI CI C	AY: very stiff to hard, medium pasticity, light grey ge brown mottling, trace of fine angular gravels 	y brown and	ALLUVIUM
1.5 900 000 000 000 000 000 000 000 000 00	- EXCAVAT No ground Test pit ba	ION TPB74 TERMINATED AT 1.20 m I water encountered. ickfilled on completion.		
PHOTOGRAPHS NOTES YES METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Bulldozer Blade R Ripper SUPPORT T Timbering	No Resistance	SAMPLES & FIELD TESTS         U50       -         D       -         Disturbed Sample         B       -         Bulk Disturbed Sample         MC       -         Moisture Content         HP       -         Hand Penetrometer (UCS kPa)         VS       -         Vane Shear; P-Peak, R-Remouded (uncorrected kPa)         PBT       -         Plate Bearing Test	CLASSIFICATION SYMBOLS SOIL DESCRIPTION Based on Unified Classification System MOISTURE D - Dry M - Moist W - Wet	S & CONSISTENCY/ RELATIVE DENSITY VS - Very Soft S - Soft F - Firm St - Stiff VSt - Very Stiff H - Hard VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		

PROJECT : Roma Flood Mitig LOCATION : Roma	gation	EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :	F	PIT NO : TPB75 FILE / JOB NO : 30031182 SHEET : 1 OF 1
POSITION : E: 679817.657, N	V: 7062179.472 (55 M A Excavator 4T	IGA94) SURFACE ELEVATION : 306.8	67 (AHD)	
DATE EXCAVATED : 12/2/14		LOGGED BY : CM		CHECKED BY : CM
EXCAVATION DIMENSIONS : DRILLING	0.50 m WIDE	MATERIA		
UPPORT UPPORT UND WATER LEVELS LLD TESTS EVATION	(RL) EPTH (m) sRAPHIC LOG SSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	INDITION ONDITION NSISTENCY RELATIVE DENSITY	STRUCTURE     & Other Observations
		Sandy CLAY: very stiff to hard, medium plasticity, light grey brown and	<sup>≥</sup> 0 0 5 1	01520
ROMA FLOOD LEVEE GPJ < <drawingfiles> 12032014 13:00 8.2.900</drawingfiles>	CI CI 0.5 0.5 0.5 0.5 0.50m 0.50m 0.90m 1.0 0.90m 1.30m 1.30m 1.30m 1.80m 2.0 0.5 0.50m 0.90m 1.0 0.90m 1.0 0.90m 1.0 0.90m 1.0 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.90m 0.	SANDSTONE: ELS-VLS, EW-HW, fine to coarse grained, remoulds to sity sand, non-plastic  SANDSTONE: ELS-VLS, EW-HW, fine to coarse grained, remoulds to sity sand, non-plastic  MUDSTONE/SILTSTONE: ELS-VLS, HW, interbedded bands 30-100mm thick, fine to coarse grained orange brown sandstone and pale grey orange brown and light brown mudstone.  MUDSTONE/SILTSTONE: ELS - VLS, HW, light brown/yellow/orange brown  EXCAVATION TPB75 TERMINATED AT 1.80 m No ground water encountered. Test pit backfilled on completion.	DtoM VStbH	WEATHERED ROCK
METHOD N Natural Exposure E Existing Excavation BH Backhoe Bucket B Buildozer Blade R Ripper VWAT SUPPORT T Timbering	ER 10 Oct., 73 Water Level on Date shown water inflow water outflow	NO     SAMPLES & FIELD TESTS     CLASSIFICATI SOIL DES       U50     - Undisturbed Sample 50 mm diameter     Based of Classification       D     - Disturbed Sample     Classification       B     - Bulk Disturbed Sample     Based of Classification       MC     - Moisture Content     D       HP     - Hand Penetrometer (UCS kPa) VS     Vane Shear; P-Peak, R-Remouded (uncorrected kPa)     Moist W       PBT     - Plate Bearing Test     Wet	ON SYMBOLS & SCRIPTION on Unified tion System	CONSISTENCY/ RELATIVE DENSITY           VS         - Very Soft           S         - Soft           F         - Firm           St         - Stiff           VSt         - Very Siff           H         - Hard           VL         - Very Loose           L         - Loose           MD         - Medium Dense           D         - Dense           VD         - Very Dense
See Explanatory Notes for details of abbreviations & basis of descriptions.		SMEC AUSTRALIA		SMEC

F	PRC	JEC	T : N :	Roma Roma	I Flood N	<i>l</i> itigatio	n		E	XCA	/ATION - GEOL CLIENT : Os FEATURE :	.OGICA twald Bros F	AL LOG Pty Ltd				<b>PIT</b> FILE SHEI	' <b>NC</b> / JO ET :	<b>)</b> : <b>TPX01</b> B NO : 30031182 : 1 OF 1
F	205		N :	E: 67	7483.89	7, N: 70	63387.	895 (	55 MC	GA94)	SURI	ACE ELEV	ATION : 302.7	769 (Å	AHD)				
	EQU				: KUBO	DTA Ex	cavator	• 4T			METI		avator					СЦ	
H			TION		NSIONS	S : 0.5	0 m WI	DE			LUG	. 10 01. (	ואוכ					СП	
			D	RILLIN	١G		T					MAT	ERIAL						
		NON CON	E	TER	s & sTS	NO (R	ο	NOI						щZ	ŠШZ				
	E VE	F PENEIKAI H	SUPPOR	GROUND WA	SAMPLES FIELD TES	ELEVATIO (RL) DEPTH (r	GRAPHIC	CLASSIFICAT SYMBOL		Sc	MATERIAL DESCRI il Type, Colour, Plasticity or Pa Secondary and Minor Co	'TION ticle Character mponents	ristic	MOISTUR	CONSISTEN RELATIVI DENSITY	5 '	원 1015	520	STRUCTURE & Other Observations
						0.0-	000	0	0.02m	Bituminou	s Spray Seal: 20mm thick.		/	-					ROAD SURFACE
						-	40000 b000			Cement T angular to	reated Base (CTB) course: grey subrounded gravel, fine grained	to dark grey a sands and so	and brown, ome fines		9				-
					0.22m		b 0 0 0	1	0.22m	Orrestal				Ξ					
					0.30m	02.5	00 X////	SP	0.30m	gravels, s	ome fines content, grading sand	y gravel in part	ts						
						°.		СН		Silty CLA to mediur	f: stiff to very stiff, high plasticity grained sand	, dark grey to b	olack, some fine						
					0.45m	-		-	0.45m	Sandy Cl	AV: stiff to yory stiff modium pla		own, como fino to	_					
						0.5 —		СІ		medium g	rained sand	sticity, grey bit	own, some nne to						-
					0.60m B				0.60m	bocoming			/ slight dark grov	-	to VS				-
										Decoming	meanum prasticity to high plastic	ity, brown grey	, siigint uark yrey		St				_
								сі-сн	I										
						302.(										H			-
					0.90m				0.90m	hecomin-				-	<u> </u>				-
						1.0-				content g	ading silty clay, some sand impa	acts shading, g	rey brown						_
						· ·													-
						· ·								Σ					-
						1.5		сі-сн							VSt				_
						30													
						-													-
						1.5 —													-
																			_
								CLCH	1.67m										
						-				Bituminou Silty CLA	s Spray Seal: old road surfacing /: verv stiff to hard, medium plas	of about 30mr	m thick	-					FILL
						301.0		T		brown, so	me white powdery calcide nodul	es >15mm diar	meter		<sub>-</sub>				-
						ĺ.		CI							St to F				-
															>>				
。						2.0-													-
3.2.90	$\left  \right $	++	-	<u> </u>		-			2.10m	EXCAVA	TION TPX01 TERMINATED AT 2	2.10 m		-	-	<b>I</b>	+	+	
3:00 8							-			No groun Test pit b	d water encountered. ackfilled on completion.								-
014 1.						).5	J												
/03/21						300	]												-
>> 12						-	1												-
gFile:						2.5	-												-
Drawir																			
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E GP						-	1												-
LEVE						0.00	-												-
000						ro -													_
MA FL																			
T RO	1	F	нотос	GRAPHS	s	⊥ 3.0		r	$\sim$	NO						<u> </u>			
STPI		N	IOTES			J TES		l		UNU			01.000-00-0-	1011 -					NOIOTENOX
og TE	ME	THO	)		F						SAMPLES & FIELD TESTS		CLASSIFICAT SOIL DE	ION SY SCRIP	'MBOL TION	5&		COI REL	NSISTENCY/ LATIVE DENSITY
ĽB Ľ	N	Na	ural Ex	posure	e	> Ш ш І	<u> </u>	lo Res	istance	1	U50 - Undisturbed Sam	le	Based	on Unif	ied stem			VS S	- Very Soft - Soft
UR.G	E BH	Exi Ba	sting E ckhoe I	xcavati Bucket	ion						D - Disturbed Sample				56511		4	F St	- Firm - Stiff
COLOC	B	Bul	ldozer	Blade							B - Bulk Disturbed Sa MC - Moisture Content	mple	MOISTURE					VSt H	- Very Stiff - Hard
RY - C	ĸ	κıρ	hei		'		0 Oct., 7	'3 Wat	er		HP - Hand Penetromet	er (UCS kPa)	M - Moist					VL L	- Very Loose - Loose
IBRA	SU T		T				evel on	Date s	hown		vs - Vane Shear; P-Pe R-Remouded (und	ак, corrected kPa)	W - Wet					MD D	- Medium Dense
ANE L	I	III	weiing				vater out	flow			PBT - Plate Bearing Tes	i.						VD	- Very Dense
SMEC BRISB.	See leta & ba	Expla ils of sis of	anator abbre f desc	y Note viatior ription	es for is s.						SMEC AUST	RALIA	·						SMEC

F	RO		- :   N ·	Roma	I Flood N	<i>l</i> itigatio	on		EXCAVATION - GEOLOGICAL LOG			PI FIL	T NC .e / JC	D : <b>TPX02</b> DB NO : 30031182 : 1 OF 1
F	POSI	TIOI	۷ :	E: 67	7557.02	0, N: 70	063047	.447 (	55 MGA94) SURFACE ELEVATION : 30	3.242 ( <i>i</i>	AHD)		1	
E	:QU )ATE	IPME E EX	:NT T CAV/	YPE ATED	: KUB	ОТА Ex 13	cavato	r 4T	METHOD : Excavator LOGGED BY : CM				CH	ECKED BY : JSM
E	XC/	AVA <sup>-</sup>	FION	DIME	NSION	S : 0.5	60 m W	IDE						
┢	Ž		DF	<b>NLLIN</b> ا	NG ∞∽	z -		NC N	MATERIAL		2			
	E PENETRATIC	<u>u I</u>	SUPPORT	GROUND WATE LEVELS	SAMPLES	ELEVATIOI	GRAPHIC LOG	CLASSIFICATIC	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENC RELATIVE DENSITY	5 10	1520	STRUCTURE & Other Observations
					B 0.20m 0.33m B	303.0		GM	0.03m Asphalt: 30mm thick     Sandy GRAVEL: very dense, some fine grained sand, red brown,     rounded to angular gravels, trace of cobbles to 75mm, some fines conter     0.33m     Sandy CLAY: hard, medium plasticity, grey brown, with firm to medium     grained sand	t w				ROAD SURFACE BASECOURSE - - FILL
					0.55m B	0.5 -	- - - - - - - - - - - - - - - - - - -		0.55m Silty SAND: dense, light brown with slight grey brown with some white powdery calcide nodules >25mm diameter		ΔΛ			-
			_		0.80m	- <sup>1</sup> 0	- 11 - 11 - 11 - 11 - 11 - 11 - 11 - 1	I I I I	becoming light grey brown/ yellow brown, increase in fine to medium grained sand content, fewer white powdery calcide nodules	Σ				-
PIT ROMA FLOOD LEVEE.GPJ < <drawingfile>&gt; 12/03/2014 13:00 8.2.900</drawingfile>		PI	HOTOG	RAPHS	5	1.0- 0200 1.5- 2.0- 0100 2.5- 2.5- 2.5- 2.5- 2.5-			EXCAVATION TPX02 TERMINATED AT 1.00 m No ground water encountered. Test pit discontinued at 1.0m. Test pit backfilled on completion, pavement surface reinstated with 150mm thick compacted asphalt concrete					
BANE LIBRARY - COLOUR.GLB Log TEST P	MET N BH R SUF T	Natu Exis Bac Bull Ripp POR	ural Ex ting E khoe E dozer ber T bering	posure xcavat Bucket Blade	e ion V	VATER	ATION T T 10 Oct., 7 Level on water inflw water out	No Res 73 Wat Date s ow flow	stance SAMPLES & FIELD TESTS SOLL Base 50 mm diameter D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test	t	YMBOLS TION fied ystem	\$ & 	CO RE VS F St S F VS H VL D D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
MEC BRIS	ee E etail bas	Expla s of a sis of	nator abbre desc	y Note viatior ription	es for ns s.				SMEC AUSTRALIA					

F	PRC	) JEC	СТ : ОМ :	Roma Roma	a Flood N	<i>l</i> itigatio	n		EXCA	VATION - GEOLOGIC CLIENT : Ostwald Bro FEATURE :	CAL LOG			F	<b>PIT</b> FILE	NC / JO =T :	D : <b>TPX04</b> B NO : 30031182 1 OF 1
F	205	SITIC	ON :	E: 67	9176.35	3, N: 70	62624.	455 (	55 MGA94)	SURFACE EL	EVATION : 301	.109 (A	AHD)				
H		JIPN			: KUB(	OTA Ex	cavator	· 4T		METHOD : E	Excavator					<u></u>	
H	JA I EXC				ENSION	S : 0.5	0 m WI	DE		LUGGED BY						υН	LONED BT . JOW
ľ			D	RILLII	NG					M	ATERIAL						
	ш	PENETRATION	SUPPORT	ROUND WATER LEVELS	SAMPLES &	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	LASSIFICATION SYMBOL	So	MATERIAL DESCRIPTION oil Type, Colour, Plasticity or Particle Chara Secondary and Minor Components	acteristic	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 1	015	20	STRUCTURE & Other Observations
	<u> </u>		=	0	в	0.0 -	0,000	0	0.0207 Bituminou	us Spray Seal: 15mm thick			0			1	ROAD SURFACE
					0.22m	301.0		GM	0.22m	RAVEL: very dense, red brown angular to su rading, with some fines content	ubrounded, fine to						FILL
						-	1 1 1	SM SM	0.30m clay fines shading li	ight brown to brown		D to M	Q				
					<u>0.50m</u>	0.5	1     	  -  -	0.50m decreasir	ng fines content		_					-
						- 30	   	SM   	0.80m								
						1.0	-		No groun Test pit b	ad water encountered. backfilled on completion.							-
						300.0	-										
						-	_										
						1.5 —	-										-
						- 500.5	-										
0						2.0	-										-
014 13:00 8.2.90						- 599.0	-										
gFile>> 12/03/2						2.5-											-
EVEE.GPJ < <drawin< td=""><td></td><td></td><td></td><td></td><td></td><td>298.5</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></drawin<>						298.5											
ROMA FLUUD LI						3.0											
			PHOTO NOTES	GRAPH	s	] YES			NO NO								
3ANE LIBRARY - COLOUR.GLB Log TES	ME B⊢ R SU T	THO Na Ex Ba Bu Ri <b>PPO</b>	D atural E ackhoe Illdozer pper RT mbering	xposur Excaval Blade	e tion :		0 Oct., 7 .evel on vater inflo	lo Res 73 Wat Date s ow flow	iistance er hown	SAMPLES & FIELD TESTS         U50       - Undisturbed Sample         50 mm diameter         D       - Disturbed Sample         B       - Bulk Disturbed Sample         MC       - Moisture Content         HP       - Hand Penetrometer (UCS kF         VS       - Vane Shear; P-Peak,         R-Remouded (uncorrected k         PBT       - Plate Bearing Test	Pa) CLASSIFICA SOIL D Based Classific MOISTURE D - Dry M - Moist W - Wet	TION SY ESCRIP d on Unif cation Sy	(MBOL: TION fied /stem	S &		COREL VS S F St VSt H VL L MD D VD	NSISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Hard - Very Loose - Loose - Medium Dense - Dense - Very Dense
MEC BRISE	See leta & ba	Exp ils of isis c	lanato f abbre of desc	ry Note viation criptior	es for ns ns.					SMEC AUSTRALIA	4						

PI L(	ROJE( <u>)CAT</u> I	CT : ON :	Roma <u>Rom</u> a	Flood N	<i>l</i> itigatio	n		EXCAVATION - GEOLOGICAL LOG CLIENT : Ostwald Bros Pty Ltd FEATURE :			P FI SI	<b>IT N(</b> LE / J( <u>HEET</u>	<b>D</b> : <b>TPX06</b> DB NO : 30031182 : 1 OF 1
P	OSITI		E: 67	9179.38	2, N: 70	60810.	796 (	55 MGA94) SURFACE ELEVATION : 297.80	63 (A	HD)			
E D	QUIPN ATE E	XCAV	ATED	: KUBC	JTA EX /13	cavator	·41	LOGGED BY : CM				Cł	HECKED BY : JSM
E	XCAV		DIME	NSION	S : 0.5	0 m WI	DE						
$\vdash$	z	D		NG ഷഗ	z ~		N	MATERIAL		X			
VF	E PENETRATIO	H SUPPORT	GROUND WATE LEVELS	SAMPLES {	ELEVATION ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	5 10	2 1520	STRUCTURE & Other Observations
				B 0.18m	-		GM	Asphalt: 30mm thick     Sandy GRAVEL: very dense, fine to coarse, ligth brown, angular to     subrounded with low plasticity fines, dry to moist 0.18m					BASECOURSE
				B 0.40m B 0.50m B			CL SC	Gravelly SILT: very stiff to hard, low plasticity, light brown and grey brown, variable layers < 50mm thick, fine to coarse angular to subrounded gravels with fine to coarse grading sands. 0.40m ClayeySAND: very dense, low to medium plasticity, fine to coarse grading, red brown, trace of subangular and rounded gravels and cobbles to 50mm. Silty CLAY: stiff, medium to biob plasticity, dark brown orey, trace of fine	D to M	٨D			FILL -
				0.80m	297.0		CI-CH	grained sand.	Σ	St			-
T ROMA FLOOD LEVEE.GPJ < <drawingfile>&gt; 12/03/2014 13:00 8.2.900</drawingfile>		PHOTO						1.00m EXCAVATION TPX06 TERMINATED AT 1.00 m No ground water encountered: Test pit backfilled on completion.					
SANE LIBRARY - COLOUR.GLB Log TEST PI	METHO N N E E: BH B B B R R R R SUPPO T TI	NOTES DD atural E xisting E ackhoe ulldozer ipper DRT mbering	kposure xcavat Bucket Blade	e ion V		$\frac{1}{2} \xrightarrow{1}{2} N$	lo Res 3 Wate Date s ow flow	istance SAMPLES & FIELD TESTS U50 - Undisturbed Sample 50 mm diameter D - Disturbed Sample MC - Moisture Content HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa) PBT - Plate Bearing Test CLASSIFICATION SOLD DES Based o Classificat MOISTURE D - Dry M - Moist W - Wet	ON SY SCRIPT on Unifition Sy	MBOLS FION ied stem	S &	CC RE VS S F St VS H VL L D VD	DNSISTENCY/ LATIVE DENSITY - Soft - Soft - Firm - Stiff - Hard - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
SMEC BRIS	ee Exp etails o basis	lanator f abbre of desc	y Note viatior ription	es for ns s.				SMEC AUSTRALIA					

EXCAVATION - GEOLOGICAL LOG         PIT NO :         PI												D: <b>TPX08</b> B NO : 30031182 1 OF 1	
PC EC D/	POSITION         E: 679275.378, N: 7060809.143 (55 MGA94)         SURFACE ELEVATION         : 298.195 (AHD)           EQUIPMENT TYPE         KUBOTA Excavator 4T         METHOD         : Excavator           DATE EXCAVATED         17/12/13         LOGGED BY         : CM         CHECKED BY         JSM												
Ε>	EXCAVATION DIMENSIONS : 0.50 m WIDE       DRILLING       MATERIAL												
Æ	E PENETRATION SUPPORT SUPPORT ELEVELS SAMPLES & SAMPLES							MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	5 10	1520	STRUCTURE & Other Observations
				B 0.20m B 0.30m	- 0.0 -		GM	Asphalt: 30m thick     Silty Sandy GRAVEL: very dense, fine to coarse, grey and ligth brown,     fine to coarse grained sands, low plasticity fines, dry to moist.     O.20m     Sandy CLAY: very stiff, medium plasticity, ligth brown to brown, fine to     medium grained sand, occasional rootlets, moist	D to M	d d			ROAD SURFACE BASECOURSE FILL
				B 0.60m			сі сі	0.60m	- 2	VSt to H			
					1.0		СІ	1.00m Silty CLAY: hard, medium to high plasticity, dark brown grey, trace of fine to grained sand	D to M	т			ALLUVIUM -
EE.GPJ < <drawingfile>&gt; 12/03/2014 13:00 8.2.800</drawingfile>					2.0			EXCAVATION TPX08 TERMINATED AT 1.20 m No ground water encountered. Test pit backfilled on completion.					-
RISBANE LIBRARY - COLOUR GLB LOG TEST PIT ROMA FLOOD LEVE	METHO N N E E BH B B3 B R R SUPPO T T	PHOTO NOTES DD atural E xisting E ackhoe ulldozer pper DRT imbering	xposuri xxposuri xxcavat Blade	s F e ion v	× 3.0 − 3.0 − YES VATER	0 Oct., 7 evel on I vater inflo	lo Res 3 Wat Date s w low	NO       SAMPLES & FIELD TESTS       CLASSIFICAT         istance       U50 - Undisturbed Sample 50 mm diameter       Based Classific         D - Disturbed Sample B - Bulk Disturbed Sample MC - Moisture Content       MOISTURE D - Dry M - Moist         er hown       HP - Hand Penetrometer (UCS kPa) VS - Vane Shear; P-Peak, R-Remouded (uncorrected kPa)       M - Moist W - Wet	FION S' ESCRIP I on Uni ation S	YMBOL: TION fied ystem	S &	CO REI VS S F St t VL L D VD	NSISTENCY/ ATIVE DENSITY - Very Soft - Soft - Firm - Stiff - Very Stiff - Hard - Very Loose - Loose - Loose - Medium Dense - Dense - Very Dense
MEC BRI %	tails o basis	of abbre of desc	y Note viation ription	25 10F 15 15.				SMEC AUSTRALIA					SMEC

PROJ LOCA	EXCAVATION - GEOLOGICAL LOG       PIT NO :       TPX09         PROJECT : Roma Flood Mitigation       CLIENT : Ostwald Bros Pty Ltd       FILE / JOB NO : 30031182         LOCATION : Roma       FEATURE :       SHEET : 1 OF 1															
POSI	POSITION         E: 679040.312, N: 7060807.014 (55 MGA94)         SURFACE ELEVATION         : 297.833 (AHD)           EQUIPMENT TYPE         : KUBOTA Excavator 4T         METHOD         : Excavator															
DATE	DATE EXCAVATED : 17/12/13 LOGGED BY : CM CHECKED BY : JSM															
EXCA	EXCAVATION DIMENSIONS : 0.50 m WIDE DRILLING MATERIAL															
EPENETRATION	EPRETRATION SUPPORT SUPPORT ROUNDWATER ROUNDWATER LEVELS GRAPHIC LOG ASSFICATION SAMBOL LOG ASSFICATION SAMBOL							Soi	MATERIAL I il Type, Colour, Plastic Secondary and I	DESCRIPTION ity or Particle Character Vinor Components	ristic	MOISTURE	ONSISTENCY RELATIVE DENSITY	5 10.	1520	STRUCTURE & Other Observations
<u>&gt;ш</u>	<u> </u>		0	В	0.0 -		GM	0.0207 Bituminous Sandy GR slight red b	s SpraySeal: 15mm thi AVEL: very dense, fine prown, fine to coarse g	ck e to medium grained, ora rained, sand content, hig	ange/ligth brown, gh fines content					ROAD SURFACE BASECOURSE
				0.18m B 0.50m			СІ-СН	0.18m Silty CLAY fine graine	': hard, medium to high d sand, occasional roc	plasticity, dark brown, g tiets.	grey, trace of	D to M				ALLUVIUM
		-		0.70m B 0.90m			СІ-СН	0.70m shading lig occasional 1.00m	gth brown, slight ligth g I white powdery calcite	rey brown with clay fines nodules less that 15mm	s in parts, 1 diameter.	≥	_			
awingFile>> 12.032.014 13:00 8.2.900								EXCAVAT No ground Test pit ba	ION TPX09 TERMINA I water encountered. ickfilled on completion.	TED AT 1.00 m						
A State LIBRARY - COLOURGEB LOG TESI PITI KOMA FLOOU LEVEEGPU <cut< td=""><td>PH-NC HOD Natu Exist Back Bullc Ripp POR1 Timb</td><td>aral Ex DTES aral Ex those E dozer I er</td><td>posure ccavati Jucket Blade</td><td>S F Son V</td><td></td><td>0 Oct., 7 evel on vater inflo vater out</td><td>[ 3 Wat Date s Dw flow</td><td>NO istance er nown</td><td>SAMPLES &amp; FIELD U50 - Undisturb 50 mm di D - Disturbed B - Bulk Distu MC - Moisture HP - Hand Per VS - Vane She R-Remou PBT - Plate Bea</td><td>D TESTS ed Sample ameter Sample trobed Sample Content tetrometer (UCS kPa) var; P-Peak, ded (uncorrected kPa) ring Test</td><td>CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet</td><td>ION SY SCRIP on Unit</td><td>(MBOL: TION fied sstem</td><td>S&amp;</td><td>CORE VSS F VL L MD D VD</td><td>NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Dense - Very Dense</td></cut<>	PH-NC HOD Natu Exist Back Bullc Ripp POR1 Timb	aral Ex DTES aral Ex those E dozer I er	posure ccavati Jucket Blade	S F Son V		0 Oct., 7 evel on vater inflo vater out	[ 3 Wat Date s Dw flow	NO istance er nown	SAMPLES & FIELD U50 - Undisturb 50 mm di D - Disturbed B - Bulk Distu MC - Moisture HP - Hand Per VS - Vane She R-Remou PBT - Plate Bea	D TESTS ed Sample ameter Sample trobed Sample Content tetrometer (UCS kPa) var; P-Peak, ded (uncorrected kPa) ring Test	CLASSIFICAT SOIL DE Based Classifica MOISTURE D - Dry M - Moist W - Wet	ION SY SCRIP on Unit	(MBOL: TION fied sstem	S&	CORE VSS F VL L MD D VD	NSISTENCY/ LATIVE DENSITY - Very Soft - Soft - Firm - Stiff t - Very Stiff - Hard - Very Loose - Loose - Loose - Dense - Very Dense
details & basi	s of a is of	bbre	viation	IS S.					SMEC AL	JSTRALIA						SMEC

PF	ROJECT	T:F N·F	Roma	Flood M	litigatio	n <b>N</b>	ION	I-CORE DRILL HOLE - GEOLOGICAL	LOG	6	HOLE NO : BH14 FILE / JOB NO : 30031182 SHEET : 1 OF 1
P		N : E	: 677	7931.047	', N: 70	62952.3	878 (5	5 MGA94) SURFACE ELEVATION : 301.490 (AHD)	ANC	GLE FI	ROM HORIZONTAL : 90°
D/	ATE ST	ARTE	ydrap D:2	ower Sc 2/10/13	DAT	E COM	G : PLET	ED : 2/10/13 DATE LOGGED : 9/10/13 LOGGED I	3Y : (		CHECKED BY : JSM
		DB		IG				ΜΑΤΕΡΙΔΙ			
PR									щZ	У Ш Х	
DRILLING	& CASING WATER	DRILLING PENETRAT	GROUND WA	SAMPLES FIELD TES	ELEVATIC (RL) DEPTH (m	GRAPHI LOG	CLASSIFICA SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTUR	CONSISTEI RELATIV DENSIT	STRUCTURE & Other Observations
					301.0		CI	0.20m Silty CLAY: stiff, medium plasticity, dark brown, some fine sand, dry, some rootlets			
				1.00m SPT 2, 8, 8 N=16 1.45m	1.0 1.0		СІ			St to VSt	
				2.00m BPT 8, 10, 11 N=21 2.45m	- 2.0-			2.20m Sandy CLAY: very stiff, medium plasticity, orange brown, dry to moist, fine grained sand			
				3.00m SPT 9, 9, 10 N=19 3.45m	- 3.0-		CI	3.50m			
				4.00m 9PT 7, 8, 10 N=18	0		ML	4.20m Sandy CLAY: very stiff, medium plasticity, dark brown, dry to moist			
900 — AD/T —				4.45m	0.792 297.0 297.0		CI	fine grained sand	D to M	VSt	
>> 12/03/2014 13:44 8.2.				5.50m SPT 8, 9, 12 N=21 <u>5.99m</u>	- 0.0 - 738 - 0.0 - 0.0 - 0.0		CI	5.50m becoming moist, with pale grey interbedding	-		
:VEE.GPJ < <drawingfile< td=""><td></td><td></td><td></td><td>7.00m SPT 6, 8, 9 N=17 7,45m</td><td>- 0. 260 70 - 7.0 - 7.0</td><td></td><td> CI </td><td>6.70m sand is becoming fine to medium grained 7.20m 7.30m becoming wet</td><td>M to W</td><td>-</td><td></td></drawingfile<>				7.00m SPT 6, 8, 9 N=17 7,45m	- 0. 260 70 - 7.0 - 7.0		 CI 	6.70m sand is becoming fine to medium grained 7.20m 7.30m becoming wet	M to W	-	
IOLE ROMA FLOOD L			Juring auger drilling		534.0		СІ	8.30m			7.93: standing water level measured on 03/10/13 before conducting insitu permeability testing
ION-CORE DRILL H			ter level estimated o	8.50m 9PT 6, 9, 17 N=26 8.95m	- 5330	00 00 00	; SW	Gravelly SAND: medium dense well graded, yellow brown, white and grey, fine gravels, wet 8.95m XW Mudstone: Dark grey and orange brown mottling, some white interchedding, you law strongth, most	3	QW	
UR.GLB Log SMEC N			02/10/13, Wat	10.00m	- 0.0		-	Therbodding, very row strength, moist			
ANE LIBRARY - COLC			<u> </u>	30/140mm N=R 10.14m	291.0			BOREHOLE BH14 TERMINATED AT 10.14 m Target depth Piezometer pipe installed on completion of the borehole			
SMEC BRISB de &	e Expla tails of a basis of	natory abbrev descri	Note iation ptions	es for Is S.	<u>→</u> 11.0			SMEC AUSTRALIA	-		SMEC.





File: 30031182 BH15 Page 1 OF 1





File: 30031182 BH16 Page 1 OF 1







		IFOT	·	lomo		itiantion	N	101		00	)	
ļ		ATIO	N : F	Roma		itigation	1		FEATURE :			SHEET : 1 OF 1
H	POS		I:Е :∙ц	E: 677	2526.818	, N: 706	63351.6	68 (!	55 MGA94) SURFACE ELEVATION : 302.420 (AHD)	ANG		ROM HORIZONTAL : 90°
H	DAT	E ST/	ARTE	D:6	/11/13	DATE	E COM	PLET	ED : 7/11/13 DATE LOGGED : 12/10/13 LOGGED BY	1:0	CM	CHECKED BY : JSM
PROGRESS Z H w Ø Z												
	& CASING	WATER	DRILLING PENETRATIO	GROUND WATE LEVELS	SAMPLES 8 FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIC SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENC RELATIVE DENSITY	STRUCTURE & Other Observations
						- 0.0		СІ	Sandy CLAY: firm, low to medium plasticity, dark brown, grey, firm to medium grained sand, dry, grass and rootlets to 0.15m depth		ш	ALLUVIUM
					1.00m BPT 2, 3, 4 N=7 1.45m			— — сі сі	becoming medium plasticity, grey brown, slightly grey brown, some white     powdery calcite nodules and glass like medium to coarse sand sized     shiny crystals     1.00m		F to St	
					2.00m	0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:		SC SM	1.60m Clayey SAND: medium dense, fine medium grained, light brown to <u>1.90m</u> brown, varying clay content, grading clayey sand/sandy clay, dry to moist clay fines reducing		DM	
					3.00m			CL-CI	2.30m Sandy CLAY: very stiff, low to medium plasticity, light brown and light grey brown and pale grey, fine to medium grained sand		VSt	
					2, 12, 17 N=29 <b>3.45m</b> 4.00m	- - - - - - - - - - - - - - - - - - -		CL-CI	3.20m becoming very stiff to hard, varying fine to medium grained sand content	D to M	VSt to H	
					<u></u> в5, 17, 16 N=33 <b>4.45m</b>	- - - - - - - - - - - - - - - - - - -		sc	Clayey SAND: medium dense to dense, fine to medium grained, light brown, pale grey and light grey brown, high clay content in bands, grading, sandy clay/clayey sand     4.70m			
006	D/T —							sc	grading fine to coarse grained, some fine to coarse angular to subrounded gravels, less clay content			
3/2014 13:45 8.2.	A				5.50m SPT 9, 13, 15 N=28 5.95m	- 587.0		· · SP ·	SAND: medium dense to dense, fine to medium grained, brown and grey, some fine to medium angular to subrounded gravels, trace of fines, dry	۵	MD to D	
< <drawingfile>&gt; 12/0</drawingfile>						296.0		SP	6.20m increase in clay fines content, grading clayey sand in bands < 100mm, some fine to coarse gravels and cobbles to 50mm diameter (slow augering)			
HOLE ROMA FLOOD LEVEE.GPJ					7.50m SPT 6, 30/140mm N>30 7.80m			SC	Clayey SAND: dense to very dense, fine to coarse angular to subrounded gravels and cobbles to 80mm, dry to moist.	D to M		
ILB LOG SMEC NON-CORE DRILL !						293.0 294.0 		sc	8.50m	Σ	D to VD	
E LIBRARY - COLOUR.G	<b>•</b>				10.40m SPT 17, 30/100mm N>30	- 10.0 - - - - - - - - - - - - - - - - - -			10.45m <u>10.65m</u> XW Mudstone: Extremely low strength, grey and dark grey, so yellow brown mottles, dry to moist, laminated, remoulds to high plasticity silty	D to M		WEATHERED ROCK
BANE					10.65m	11.0			BOREHOLE BH18 TERMINATED AT 10.65 m			-
SMEC BRIS	See Explanatory Notes for details of abbreviations     Piežometer pipe installed on completion of the borehole SMEC AUSTRALIA       & basis of descriptions.     SMEC AUSTRALIA											


	NON-CORE DRILL HOLE - GEOLOGICAL LOG HOLE NO : BH19																
PROJECT : Roma Flood Mitigation CLIENT : Ostwald Bros Pty Ltd FILE / JOB NO : 30031182   LOCATION : Roma FEATURE : SHEET : 1 OF 1																	
POSITION : E: 677560.166, N: 7063357.903 (55 MGA94) SURFACE ELEVATION : 301.476 (AHD) ANGLE FROM HORIZONTAL : 90°																	
DAT	DATE STARTED : 7/11/13 DATE COMPLETED : 7/11/13 DATE LOGGED : 12/10/13 LOGGED BY : CM CHECKED BY : JSM																
	DRILLING MATERIAI																
PROG																	
DRILLING & CASING	WATER	DRILLING	GROUND WA	SAMPLES FIELD TES	ELEVATIO	DEPTH (m	GRAPHI LOG	CLASSIFICAT SYMBOL		Soil Type, Co Secu	MATERIAL DE blour, Plasticity ondary and Mir	SCRIPTION or Particle Characte or Components	eristic	MOISTUR	CONSISTEN RELATIVI DENSITY	STRUCTURE & Other Observations	
						0.0 - -		CL-CI	0.50m	Sandy CLAY: firm, lo medium grained san	ow to medium p id, grass rootlet	lasticity, dark brown s to 0.15m	grey, fine to	٥	ш	ALLUVIUM	
				1.00m	301.0	1		CL-CI	1.00m	becoming stiff, light	brown, fine grai	 ned sand			ŭ		
				BPT 6, 10, 10 N=20		1.0			<u>1.00m</u>	becoming very stiff r and grey brown, son	medium plasticit ne white powde	y, orange/yellow bro ry calcite nodules	own, pale grey	-			
					300.0			CI							VSt		
				2.00m	-	2.0			2.00m	becoming very stiff t							
				N=35 2.45m				CL-CI						D to M	st to H		
				2.1011	299.0	-		CL-CI	2.50m 2.70m	increasing in sand c	ontent, decreas	ing in moisture cont			<pre>N</pre>	-	
				3.00m		-				Silty SAND: medium and light grey brown	dense, fine to	medium grained, yel	llow, light brown	1			
				BPT 8, 9, 11 N=20 3.45m		3.0	Frida 1 - T Frida	SC	0.50								
					298.0			sc	3.50m	increase in clay cont	tent, grading cla	wey sand/sandy cla		1			
				4.00m		-		sc	3.80m	traces of fine to med	 dium gravels					-	
	c –			<b>ჭ,¤</b> 1¢m β), 12, 13 N=25 <b>4.45m</b>	-	4.0			4.10m	becoming fine to coa	arse grained, gr	ading of sand varyir	 ng in bands less		-		
						-		sc		than 50mm thick.			-	0		-	
						297.0		1		4.70m						W	-
0						- 5.0		SC		medium dense, vary	um dense, fine ving clay fines g	to coarse grained, lig rading towards sand	ght brown, ly clay in bands	5			
- WB/1									5.30m					Dto			
				5.50m	96.0			SC	5.60m	decreasing clay fine	s content					-	
<u>i</u>				5.95m	5	-		SP		SAND: medium dens brown and pale grey bands	se, fine to coars /, trace of angul	ed grained, light gre ar fine gravels, trace	ey brown yellow e of fines, varying				
				0.00111		6.0		SD	<u>6.00m</u>	trace of fine to medi	um grained ang	ular gravel				-	
						-		55	6.40m			0					
					295.0	1				XW Mudstone: extre dark grey some orar	ngee yellow streng	oth, extremely weath wn mottles, dry to m	lered, grey and loist,				
				7.00m SPT	_	7.0-			7.00m	visibilv laminated, ex				-		-	
				27, 30/80mm N>30 7,23m						,		<u>.</u>					
				7.2011	294.0											-	
																-	
2						8.0											
				8.50m					8.50m							-	
				SPT 30/140mm N=R	293.	-				shading dark grey, p medium grained, ext	ale grey and ye tremely low stre	ellow brown, occasio ngth, higher moistur	nal brown e content			-	
				8.64m	4	- 9.0										-	
5					32.0												
					26	-										-	
				10.00m SPT 21.	1	0.0			10.05								
				30/95mm N>30 10.25m					10.25m	BOREHOLE BH19 T		T 10.25 m					
					291.0					Borehole grouted to	surface on com	pletion.					
						10											
See detai	See Explanatory Notes for details of abbreviations SMEC AUSTRALIA																

File: 30031182 BH19 Page 1 OF 1

NON-CORE DRILL HOLE - GEOLOGICAL LOG   HOLE NO : BH20     PROJECT : Roma Flood Mitigation   CLIENT : Ostwald Bros Pty Ltd   FILE / JOB NO : 30031182     LOCATION : Roma   FEATURE :   SHEET : 1 OF 1													
POSITION     E: 677590.388, N: 7063360.726 (55 MGA94)     SURFACE ELEVATION: 301.207 (AHD)     ANGLE FROM HORIZONTAL: 90°													
RIG I YPE : Hydrapower Scout MOUNTING : Truck CONTRACTOR : GEODRILL DRILLER :   DATE STARTED : 7/11/13 DATE COMPLETED : 7/11/13 DATE LOGGED : 12/10/13 LOGGED BY : CM CHECKED BY : JSM													
	DRILLING MATERIAL												
& CASING	WATER	DRILLING	GROUND WATEI LEVELS	SAMPLES &	ELEVATION (RL)	GRAPHIC LOG	SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	STRUCTURE & Other Observations		
				4.00	301.0		CI CI	Sandy CLAY: Firm to stiff, medium plasticity, dark brown, slightly dark grey brown, fine to medium grained sand grass and rootlets to 0.1m 		St F to St	ALLUVIUM		
				BPT 8, 11, 14 N=25 1.45m	0.000 0.000		1 CI 1	10m     becoming very stiff, medim plasticity, light and yellow brown, pale grey and light grey brown, mottled     50m     some white powdery calcite nodules <10 mm diameter generally shading lighter browns	-				
				2.00m BPT 6, 11, 14 N=25 2.45m	0.66 0.662		CI 2 ;L-CI	50m becoming low to medium plasticity, increasing fine to medium grained .80msand	_	VSt			
				3.00m SPT 9, 12, 13 N=25 3.45m	- 3.0		sc	higher silt content 10m Clayey SAND: medium dense, fine to medium grained, some coarse grains, yellow orange brown, pale grey and ligth grey brown.	DtoM	DM			
				4.00m 9PT 9, 14, 30/140mm N>44 <i>4</i> :4411	- 4.0 562 0		sc	.80m	_	o VD			
WB/1				5.50m SPT 21, 30/120mm N>30 5.77m	5.0 739(0) 6.0		SC g	.50m	-	G	WEATHERED ROCK		
				7.00m SPT 18, 30/110mm N>30 7.26m	294.0 295.		7	.80m	_				
				8.50m SPT 12, 24, 26 N=50 8.95m	292.0 293.0 2.6 293.0 2.6		£	50m	_				
				9.80m SPT 20/5mm HB N=R 9.81m	10.0		5	.81m BOREHOLE BH20 TERMINATED AT 9.81 m Target depth Borehole grouted to surface on completion.					
See detai & ba	See Explanatory Notes for details of abbreviations SMEC AUSTRALIA												



File: 30031182 BH21 Page 1 OF 1



PRC	NON-CORE DRILL HOLE - GEOLOGICAL LOG   HOLE NO : BH22     PROJECT : Roma Flood Mitigation   CLIENT : Ostwald Bros Pty Ltd   FILE / JOB NO : 30031182     OCATION : Roma   FATURE :   SHEET : 2 OF 2												
POS	LOCATION     Roma     FEATURE     SHEET: 2 OF 2       POSITION     E: 679105.613, N: 7062556.552 (55 MGA94)     SURFACE ELEVATION: 300.476 (AHD)     ANGLE FROM HORIZONTAL: 90°												
RIG	RIG TYPE : Hydrapower Scout MOUNTING : Truck CONTRACTOR : GEODRILL DRILLER : TP												
DAT	DATE STARTED : 1/10/13 DATE COMPLETED : 1/10/13 DATE LOGGED : 9/10/13 LOGGED BY : OC CHECKED BY : JSM												
		DF	RILLIN	IG (0	1		z	MATERIAL					
& CASING	WATER	DRILLING	GROUND WATEI LEVELS	SAMPLES & FIELD TEST	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENC RELATIVE DENSITY	STRUCTURE & Other Observations		
0					- 11.0			XW Mudstone: dark grey and orange brown with iron staining throughout, some guartz laminations, very low strengt, moist (continued)			WEATHERED ROCK		
WB/T				11.50m	-			11.50m	_		-		
<b>V</b>	-			SPT 15,	289.0			as above, quartz laminations discontinued					
				30/120mm N>30 11.77m				11.95m			-		
				·	12.0			BOREHOLE BH22 TERMINATED AT 11.95 m Target depth					
								Piezometer pipe installed on completion of the borehole			-		
					- 288.0						-		
					-						-		
					13.0								
											-		
					87.0								
											-		
					14.0								
					-						-		
					86.0						-		
					<sup>6</sup> –						-		
					15.0								
					-						-		
											-		
					- 28						-		
					16.0						-		
8					-						-		
8.2.9											-		
13:45					284						-		
/2014					17.0						-		
12/03					-						-		
											-		
awingf					283.1						-		
Ŭ V											-		
GPJ											-		
LEVE											-		
00					282.C								
MAFL					-						-		
E RC					19.0						-		
LHOL											-		
DRIL					281.0								
CORE											-		
NON					20.0						-		
SMEC											-		
, Go					- 280.0								
.GLB													
LOUR					21.0								
CO V											-		
RARY					79.0						-		
E L B					- 5						-		
	Expla	natory	Note	s for	⊥ 22.0						_		
deta & ba	details of abbreviations SMEC AUSTRALIA												





PRC	NON-CORE DRILL HOLE - GEOLOGICAL LOG     HOLE NO : BH23       PROJECT : Roma Flood Mitigation     CLIENT : Ostwald Bros Pty Ltd     FILE / JOB NO : 30031182       COCATION : Roma     CLIENT : Ostwald Bros Pty Ltd     FILE / JOB NO : 30031182												
POS	LOCATION: Roma     FEATURE:     SHEET: 2 OF 2       POSITION: E: 679111.967, N: 7062507.173 (55 MGA94)     SURFACE ELEVATION: 300.431 (AHD)     ANGLE FROM HORIZONTAL: 90°												
RIG	RIG TYPE : Hydrapower Scout MOUNTING : Truck     CONTRACTOR : GEODRILL     DRILLER : TP												
DAT	DATE STARTED : 1/10/13 DATE COMPLETED : 1/10/13 DATE LOGGED : 9/10/13 LOGGED BY : OC CHECKED BY : JSM												
		DF		IG (0			z	MATERIAL					
& CASING	MATER	DRILLING	GROUND WATEF	SAMPLES & FIELD TESTS	ELEVATION (RL) DEPTH (m)	GRAPHIC LOG	CLASSIFICATIO SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations		
WB/TC				11 50m	-			XW Mudstone: dark grey and orange brown, with some iron staining nodules and quartz laminations, very low strength, moist ( <i>continued</i> )	Σ		WEATHERED ROCK		
¥.	-			SPT 30/140mm	289.(			11.64m becoming dark grey, homogenous throughout					
				N=R 11.64m	1 =			BOREHOLE BH23 TERMINATED AT 11.64 m Target depth Proceeding sources and accompletion			-		
					12.0			Borenole grouted to surface on completion					
					_						-		
					288.0								
					-						-		
					13.0								
					-						-		
					287.								
											-		
					-						-		
					0.						-		
					- 286						-		
					15.0						-		
											-		
					5.0						-		
					- 58						-		
					16.0								
8					-						-		
8.2.9					4.0						-		
13:45					- 28						-		
3/2014					17.0						-		
<ul><li>12/0</li></ul>											-		
gFile>;					33.0						-		
Drawin					- 5						-		
] ₹					18.0								
/EE.G					-						-		
DD LEI					282.0								
AFLOC											-		
ROM					19.0						-		
HOLE											-		
ORILL					281.0								
ORE L													
0-NON					20.0								
SMEC											-		
Log					280.0								
3.GLB											-		
JLOUF					21.0								
,-cč											-		
IBRAF					279.0								
ANEL											-		
See deta & ba	See Explanatory Notes for details of abbreviations & basis of descriptions.												

### **APPENDIX D TEST PIT AND SPT SAMPLE PHOTOS**

# **Test Pit Photos**

## Test Pit TP3











Test Pit TP11









Test Pit TP15





Test Pit TP17





Test Pit TP20









Test Pit TP25









# **Test Pits Borrow Areas Photos**



Test Pit TPB2
































































Test Pit TPB36

































































Test Pit TPB73







# Pavement Test Pits Test Pit TPX01



Test Pit TPX02





Test Pit TPX6







#### Bore Holes SPT Samples Photographs:

BH14 – SPT 1.0-1.45:



## BH14 – SPT 2.0-2.45:



BH14 – SPT 3.0-3.45:



#### BH14 – SPT 4.0-4.45:





BH14 – SPT 7.0-7.45:





#### BH14 – SPT 10.0-10.45:



#### BH15 - SPT1.0-1.45:



BH15 – SPT 2.0-2.45:



BH15 - SPT 3.0-3.45:



BH15 – SPT 4.0-4.45:







## BH15 – SPT 10.0-10.45:



#### BH16 – SPT 1.0-1.45:



## BH16 – SPT 2.0-2.45:



## BH16 – SPT 3.0-3.45:



## BH16 – SPT 4.0-4.45:



#### BH16 – SPT 5.5-5.95:



## BH16 – SPT 7.0-7.45:



BH16 - SPT 8.5-8.95:



#### BH16 – SPT 9.5-9.95:

SOO SI 0 83 ROMA FLOD LEVEE SHEC SHILS 30/09/13 SPT: 9.5+9.95 11,30/130 N=R \*\* \*\* 200 10 20 30 40 50 50 50 50 50 90 300 16 20 30 40 50 40 10 40 10 40 10 20 40 50 80 76 80 80 ----

BH17 – SPT 1.0-1.45:



BH17 – SPT 2.0-2.45:



## BH17 – SPT 3.0-3.45:



BH17 – SPT 4.0-4.45:



# BH17 – SPT 5.5-5.95:


# BH17 – SPT 7.0-7.45:



# BH17 – SPT 8.0-8.45:



BH18 – SPT 1.0-1.45:



BH18 – SPT 2.5-2.45:



#### BH18 – SPT 3.0-3.45:



#### BH18 - SPT 3.0-3.45:



BH18 - SPT 3.0-3.45:



BH18 - SPT 4.0-4.45:



BH18 – SPT 5.50-5.95:



BH18 – SPT 7.0-7.45:



BH18 – SPT 7.5-7.79:

30031083 ROMA FLOOD LEVEE SPT 7.50-7.79 BH18 6,30/140 N=>30. 6/11/13 NIL LOVE AMERICAN

BH18 – SPT 8.5-8.95:

1.1 30031083 ROAD RODA LEVEE BH 18 8.50- 8.95 6/11/13 222 N=4 ? 的这一时,而一时分时

#### BH18 – SPT 10.5-10.65:



BH19 – SPT 1.0- 1.45:



### BH19 – SPT 2.0-2.45:



BH19 – SPT 3.00-3.45:



BH19 – SPT 4.0-4.45:



BH19 – SPT 5.5-5.95:



#### BH19 – SPT 7.0-7.23:



BH19 - SPT 8.5-8.64:



BH19 – SPT 8.5-8.64:



# BH19 - SPT 10.0-10.245:



BH20 - SPT 1.0-1.45:



BH20 – SPT 2.0-2.45:



BH20 – SPT 2.0-2.45:



BH20 – SPT 3.0-3.45:



BH20 – SPT 4.0-4.45:



BH20 – SPT 5.5-5.77:



BH20 – SPT 7.0-7.26:



BH20 – SPT 8.5-8.95:



BH21 – SPT 1.0-1.45:



BH21 – SPT 2.0-2.45:





BH21 – SPT 4.0-4.45:



BH21 - SPT 3.0-3.45:



BH21 – SPT 7.0-7.45:





#### BH21 – SPT 10.0-10.15:



BH22 – SPT 1.0-1.45:



#### BH22 – SPT 2.0-2.45:



BH22 – SPT 3.0-3.45:



## BH22 – SPT 4.0-4.45:



BH22 – SPT 5.5-5.95:



## BH22 – SPT 7.0-7.45:



BH22 - SPT 8.5-8.95:



### BH22 - SPT 10.0-10.45:



BH22 – SPT 11.5-11.95: 3003/023 ROMA FLOOD LEVEE SMEC BH 22 01/10/13 SPT: 19.5-11.95 15,30/120 N=RNE

### BH23 – SPT 1.0-1.45:



BH23 – SPT 2.0-2.45: 30031083 ROMA FLOD LEVER SMEC 11013 BH 23 SPT 2.0-2.45 2,7,6;N=B 

### BH23 – SPT 3.0-3.45:





### BH23 – SPT 5.5-5.95:





### BH23 – SPT 8.5-8.95:



# BH23 – SPT 10.0-10.45:

